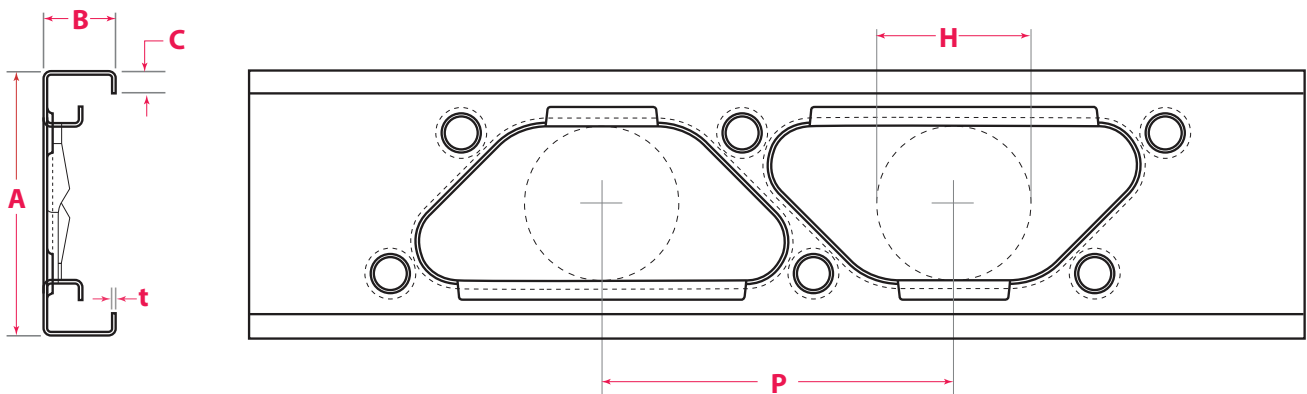


DeltaStud - Lightweight Steel Framing



Load Tables for Wind Bearing and Combined Wind
& Axial Load Bearing Condition

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Commentary

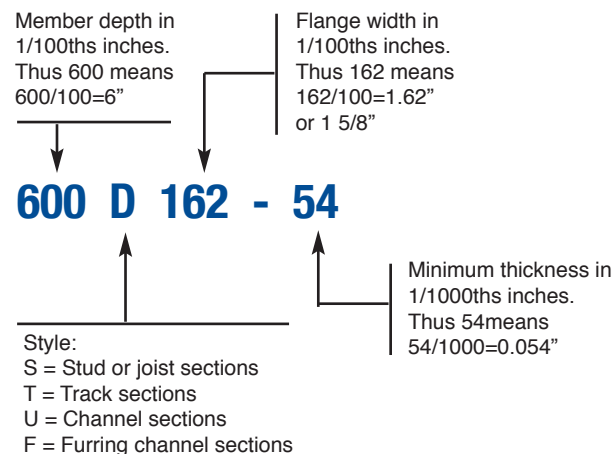
1. INTRODUCTION

The technical data contained herein is intended as an aid to the design professional and should not be used to replace the judgment of a qualified Engineer or Architect.

2. PRODUCT IDENTIFICATION

The cold-formed steel framing manufacturers use a universal designator system for their products. The designator is a four part code which identifies depth, flange width, member type and material thickness.

Example: 600D162-54



Notes:

1. Minimum thickness exclusive of coatings and represents 95% of the design thickness. See CAN/CSA-S136-12 Section A2.4.
2. The yield strength used in design, if greater than 33 ksi, needs to be identified. For example, a 1-5/8" x 6" DeltaStud with a design thickness of 0.0566" and a design yield strength of 50 ksi would be designated as : 600D162-54 (50 ksi). Note that if the (50 ksi) is omitted then 33 ksi is assumed.

3. For track, "T", sections, depth is a nominal inside to inside dimension. Other dimensions are out to out.

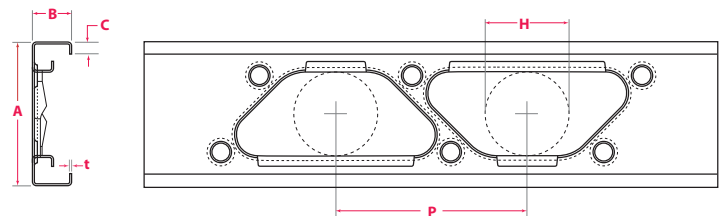
3. SECTION GEOMETRIES

- 3.1 Section geometries are identified by the product designation as defined in the previous section.

- 3.2 Stud lip lengths are listed in the Section Properties table.

- 3.3 Stud, and Track Inside Bend Radii

For stud and track, the inside radius equals the maximum of $(3/32" - t/2)$ or $1.5t$ where t = thickness exclusive of coating in inches.



SECTION DEPTH	FLANGE	LIP	HOLE DIA.	PITCH
A	B	C	H	P
3.625"	1.625"/2.000"/2.500"/3.000"	0.500"/0.625"/0.625"/0.625"	2.125"	4.000"
4.000"	1.625"/2.000"/2.500"/3.000"	0.500"/0.625"/0.625"/0.625"	2.125"	4.000"
6.000"	1.625"/2.000"/2.500"/3.000"	0.500"/0.625"/0.625"/0.625"	3.500"	8.000"
8.000"	1.625"/2.000"/2.500"/3.000"	0.500"/0.625"/0.625"/0.625"	3.500"	8.000"

4. STUD SECTION PROPERTY TABLES

4.1 Structural properties are computed in accordance with CSA Standard CAN/CSA-S136-12, North American Specification for the Design of Cold-Formed Steel Structural Members.

4.2 Steel shall meet the requirements of CAN/CSA-S136-12 with a minimum yield strength of 33 ksi for design thicknesses less than or equal to 0.0451" and 50 ksi for design thicknesses greater than or equal to 0.0566".

4.3 Section properties are computed on the basis of the design thicknesses shown in the tables. Design thicknesses are exclusive of coating.

4.4 Perforations are shown on the part drawings. The distance from the centreline of the last perforation to the end of a wall stud is assumed to be 12" minimum.

4.5 The fully braced factored moment resistances, M_{rx} and M_{ry} are derived using effective section properties. The increase in yield from the cold work of forming has been utilized where applicable.

4.6 The maximum unbraced length, L_u , which precludes lateral buckling in beams is calculated from the formulae in the Commentary on North American Specification for the Design of Cold-Formed Steel Structural Members, 2012 Edition, published by the American Iron and Steel Institute (Formulae CC3.1.2.1-11, C-C3.1.2.1-12 & C-C3.1.2.1-14). K_y , K_t and C_b are set equal to one

4.7 Factored resistances include the following phi factors:

Moment $\Phi_b = 0.90$

Shear $\Phi_v = 0.80$

Web Crippling See Item 4.9

4.8 The deflection moment of inertia, I_{xd} , includes the effects of local buckling at the stress level resulting from specified live loads (approximated by $0.6 \times F_y$). This inertia is only appropriate for checking serviceability limit states.

4.9 Web crippling

No specific provisions are currently included in CAN/CSA-S136-12 for the design of steel stud flexural members with stud-to-track connections susceptible to web crippling. However, web crippling provisions are provided in the North American Standard for Cold-Formed Steel Framing - Wall Stud Design (AISI S211-12), published by the American Iron and Steel Institute and have been adopted herein where they apply.

The AISI S211 web crippling equation coefficients are as follows:

$$C = 3.7$$

$$C_R = 0.19$$

$$C_N = 0.74$$

$$C_h = 0.019$$

$$\Phi_w = 0.75$$

The limits of applicability are as follows:

- i) Stud design thickness 0.0346" to 0.0770"
- ii) Stud design yield strength 33 ksi to 50 ksi
- iii) Stud nominal depth 3.50" to 6"
- iv) Track thickness equal to or greater than the stud thickness
- v) Both flanges of the stud attached to the track
- vi) Track nominal flange width 1.25" to 3.375"
- vi) Studs not adjacent to wall openings

For studs with design thicknesses greater than 0.0770" or depths greater than 6", the web crippling provision for CAN/CSA-S136-12 are assumed to apply. The end one-flange loading fastened to support condition (Table C3.4.1-2) is used with a 0.75 resistance factor, Φ_w .

For both approaches to web crippling, an unperforated section with 1.0" bearing length is assumed.

4.10 Distortional Buckling

Distortional buckling properties and factored resistance are based on an unperforated section. Neither S136-12, Sections A – G, nor do these tables include provisions for the weak axis distortional buckling of studs (lips in compression). Where weak axis distortional buckling is a design concern, additional calculation is required.

5. Track Section Property Tables

5.1 The previous Commentary Items 4.1 - 4.3 apply.

5.2 The factored moment resistance, M_{rx} , is derived using effective section properties with the cold work of forming conservatively neglected. Factored shear and moment resistances, V_r and M_{rx} , include a 0.8 and 0.9 resistance factor respectively.

5.3 The deflection moment of inertia, I_{xd} , includes the effects of local buckling at the stress level resulting from specified live loads (approximated by $0.6 \times F_y$). This inertia is only appropriate for checking serviceability limit states.

6. Wind Bearing Stud Allowable Height Tables

6.1 The allowable heights are computed in accordance with the requirements of the National Building Code of Canada 2010 and CAN/CSA S136-12, North American Specification for the Design of Cold-Formed Steel Structural Members.

6.2 Stud material, geometry and properties conform to the Wall Stud Section Property Tables and Commentary Item 4.

6.3 Strength allowable heights are limited by web crippling or midspan moment at factored loads. Sheathing providing full lateral support on both sides of the studs is assumed. The sheathings are to have adequate durability, strength and rigidity to prevent the studs from buckling laterally and to resist the torsional component of loads not applied through the shear centre. Loads are assumed to be uniformly distributed.

In addition to the sheathing requirements outlined above, it is recommended that bridging be provided at 5'-0" o.c. or less in order to achieve alignment of the members and to provide the necessary structural integrity during construction. Design the bridging to prevent stud rotation and translation about the minor axis. Provide periodic anchorage and/or blocking-in for the bridging as required structurally.

6.4 The deflection allowable heights ($L/360$, $L/600$ and $L/720$) are calculated for the specified wind loads shown without imposing any strength limit states. In no case shall the deflection allowable height exceed the strength allowable height.

Allowable heights for deflection limits not shown can be calculated by multiplying the L/360 allowable heights by the following factors:

Required Deflection Limit	Factor
L/1000	0.711
L/720	0.794
L/600	0.843
L/360	1.000
L/240	1.145
L/180	1.260

6.5 Web crippling allowable heights are limited by stud web crippling in the top or bottom track at factored loads.

6.6 Shaded values indicate heights where the factored end reaction exceeds the factored web crippling resistance, P_r . Use the allowable height value provided for web crippling or design end connections that are not susceptible to web crippling.

7. Combined Wind and Axial Load Bearing DeltaStud Tables

7.1 SHEATHED AND UNSHEATHED

7.1.1 The factored loads are computed in accordance with the requirements of the National Building Code of Canada 2010 and CAN/CSA S136-12, North American Specification for the Design of Cold-Formed Steel Structural Members.

7.1.2 Stud material, geometry and properties conform to the Stud Section Properties Table and Commentary Item 4.

7.1.3 Wind loads are assumed to be uniformly distributed. For deflection check refer to Wind Bearing Stud Allowable Height Tables.

7.1.4 Studs subject to web crippling have not been flagged in the tables. Refer to the Wind Bearing Stud Tables for limiting stud heights for web crippling.

7.1.5 Where dead, live and/or wind loads are combined, the appropriate load combination factors must be applied before using the tables.

7.2 SHEATHED TABLES

7.2.1 The factored loads are limited by the interaction of axial load and major axis bending due to wind. End shear due to wind alone is checked.

7.2.2 For factored axial resistance, $\Phi_c = 0.8$.

7.2.3 Sheathing providing full lateral support on both sides of the studs is assumed. The sheathings are to have adequate durability, strength and rigidity to prevent the studs from buckling laterally and to resist the torsional component of loads not applied through the shear centre. (Some wallboard and sheathing materials provide partial support only. Refer to CAN/CSA S136-12 Clause D4.1 or use the unsheathed tables.)

7.2.4 Axial loads are assumed to be concentrically applied to studs with respect to the X and Y axes. (Some end connection details can introduce significant eccentricities which will reduce the stud capacities given in the tables.)

7.2.5 Provide bridging at 4'-0" o.c. or less in order to align members and to provide the necessary structural integrity during construction and in the completed structure. Design the bridging to prevent stud rotation and translation about the minor axis. Provide periodic anchorage for the bridging as required structurally.

7.2.6 Effective lengths are calculated as follows (only major axis buckling is considered):

- $K_x = 1$
- L_x = the overall length of the stud

7.2.7 Studs are treated as compressive members in frames that are braced against joint translation. Provide the necessary bracing to adequately control the sidesway of the overall structure either due to wind, seismic loads or P-delta effects.

7.3 UNSHEATHED TABLES

7.3.1 The factored loads are limited by the interaction of axial load and major axis flexural bending due to wind. End shear due to wind alone is checked. The factored resistance for moment includes the effects of lateral instability assuming an unsupported length equal to the maximum permitted bridging spacing. The effects of warping torsion due to loads not applied through the shear centre are not included in the tables.

Studs subject to web crippling have not been flagged in the tables. Refer to the Wind Bearing Stud Tables for limiting stud heights in situations where web crippling applies. Where web crippling is critical, bearing stiffeners at the top and bottom track may be required. Refer to CAN/CSA-S136-12.

7.3.2 For factored axial resistance, $\Phi_c = 0.8$

7.3.3 Sheathing is not relied on to restrain the studs. Periodic lateral and torsional support is assumed to be provided by bridging spaced at a maximum of 4' - 0" o.c. The bridging need not be spaced equally over the height of the stud provided that the 4' - 0" spacing limit between lines of bridging and between the last line of bridging and the end of the stud is adhered to. The ends of the studs are also assumed to be laterally and torsionally restrained. Design bridging for the accumulated torsion between bridging lines in combination with 2% of the factored compressive force in each stud. Refer to CAN/CSA S136-12. Provide periodic anchorage for the bridging as required structurally.

7.3.4 Axial loads are assumed to be concentrically applied to studs with respect to the X and Y axes. (Some end connection details can introduce significant eccentricities which will reduce the stud capacities given in the tables.)

7.3.5 Effective lengths are calculated as follows (major axis, minor axis and torsional-flexural buckling is considered):

- K_x, K_y and $K_t = 1$
- L_x = the overall length of the stud
- L_y, L_t = maximum distance between lines of bridging

7.3.6 Studs are treated as compressive members in frames that are braced against joint translation. Provide the necessary bracing to adequately control the sidesway of the overall structure either due to wind, seismic loads or P-delta effects.

8. Symbols

A	= out to out depth of stud (in.) = nominal depth of track (in.)
Area	= fully effective (unreduced for local buckling) area (in. ²)
B	= out to out width of flange (in.)
C	= out to out depth of lip stiffener (in.)
C_w	= warping torsional constant (in. ⁶)
F_y	= minimum yield strength (ksi)
H	= maximum diameter of round pipe that can fit through web hole
I_x	= fully effective (unreduced for local buckling) moment of inertia about the major axis (in. ⁴)
I_{xd}	= effective moment of inertia about the major axis for checking deflections with specified (unfactored) loads (in. ⁴)
I_y	= fully effective (unreduced for local buckling) moment of inertia about the minor axis (in. ⁴)
J	= St. Venant torsional constant (in. ⁴)
j	= torsional-flexural buckling parameter for singly symmetric beam-columns (in.)
m	= distance from centreline of web to the shear centre (in.)
M_{rx}	= fully braced factored moment resistance about the major axis (in.kips)
M_{ry}	= fully braced factored moment resistance about the minor axis with the web in compression or with the lips in compression (in.kips)
L_u	= maximum unbraced length of flexural members which precludes lateral buckling (in.)
P_r	= factored web crippling resistance (kips)
r	= inside bend radius (in.)
r_x	= fully effective (unreduced for local buckling) radius of gyration about the major axis (in.)
r_y	= fully effective (unreduced for local buckling) radius of gyration about the minor axis (in.)
S_f	= fully effective (unreduced for local buckling) section modulus (in. ³)
t	= design steel thickness exclusive of coating (in.)
V_r	= factored shear resistance (kips)
Weight	= weight per foot based on uncoated, unperforated steel (lbs/ft)
x_{cg}	= distance to centroid from back of web for the fully effective section (unreduced for local buckling) (in.)
x_o	= distance from shear centre to centroid (in.)

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The load tables and technical information contained in this catalogue were prepared by Dr. Roger A. LaBoube, Ph.D, P.E. Professor LaBoube received his engineering degrees from the University of Missouri-Rolla. He has approximately 14 years of industry experience, with ten of those years with Butler Manufacturing Company in Research and Development.

Since 1978, Dr. LaBoube has held faculty positions at Iowa State University, the University of Kansas, and the Missouri University of Science & Technology (formerly University of Missouri-Rolla). Dr. Laboube is Curator's Teaching Professor Emeritus of Civil Engineering and Director of the Center for Cold-Formed Steel Structures at Missouri University of Science & Technology.

Dr. LaBoube is active professionally in the following activities:

- A member of the AISI Committee on Specifications for the Design of Cold-Formed Steel Structural Members.
- Currently serves as Chairman of the Education Subcommittee of the Committee on Specifications.
- A member of the AISI Committee on Framing Standards and chairs the Design Methods Subcommittee.
- Co-author with Dr. Wei-Wen Yu, Cold-Formed Steel Design, 4th edition, John Wiley & Sons.

- Has authored or co-authored the following AISI design guides:

The Design Guide for Cold-Formed Steel Trusses

Design Guide for Beams with Web Openings

A Design Guide for Designing with Standing Seam Roof Panels (co-author)

- Is actively involved in cold-formed steel research.
- Has served as a consultant to manufacturers and consulting engineers on numerous topics related to cold-formed steel members and connections.

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3 5/8" Track Section Properties

Flange = 1.25" and 2.00"											
Section Identification	Dimension			Weight (lb/ft)	Yield F_y (ksi)	Area (in. ²)	x_{cg} (in.)	x_o (in.)	C_w (in. ⁶)	J (in. ⁴)	j (in.)
	Thickness t (in.)	Depth A (in.)	Flange B (in.)								
362T125-33	0.0346	3.701	1.25	0.721	33	0.212	0.266	0.658	0.0756	0.0000845	2.09
362T125-43	0.0451	3.696	1.25	0.939	33	0.276	0.269	0.654	0.0978	0.0001870	2.09
362T125-54	0.0566	3.710	1.25	1.180	50	0.346	0.273	0.648	0.1230	0.0003690	2.11
362T125-68	0.0713	3.732	1.25	1.480	50	0.436	0.277	0.641	0.1560	0.0007380	2.13
362T125-97	0.1017	3.778	1.25	2.110	50	0.621	0.287	0.626	0.2260	0.0021400	2.17
362T200-33	0.0346	3.701	2.00	0.897	33	0.264	0.533	1.270	0.2690	0.0001050	2.09
362T200-43	0.0451	3.696	2.00	1.170	33	0.343	0.536	1.270	0.3500	0.0002330	2.09
362T200-54	0.0566	3.710	2.00	1.470	50	0.431	0.539	1.260	0.4420	0.0004600	2.11
362T200-68	0.0713	3.732	2.00	1.850	50	0.543	0.543	1.250	0.5640	0.0009190	2.13
362T200-97	0.1017	3.778	2.00	2.630	50	0.773	0.551	1.230	0.8250	0.0026700	2.17

Flange = 1.25" and 2.00"									
Section Identification	r_x (in.)	r_y (in.)	I_x (in. ⁴)	I_y (in. ⁴)	S_r (in. ³)	M_{rx} (in-kips)	L_u (in.)	Shear V_r (kips)	I_x defl. (in. ⁴)
362T125-33	1.44	0.377	0.438	0.0301	0.232	5.17	25.7	1.31	0.381
362T125-43	1.44	0.375	0.571	0.0389	0.302	7.27	25.8	2.22	0.525
362T125-54	1.45	0.373	0.723	0.0482	0.378	14.00	20.9	4.31	0.671
362T125-68	1.45	0.370	0.921	0.0598	0.475	19.20	21.0	6.02	0.901
362T125-97	1.47	0.365	1.340	0.0825	0.675	30.40	21.4	8.48	1.340
362T200-33	1.53	0.645	0.619	0.1100	0.328	5.65	41.0	1.31	0.458
362T200-43	1.53	0.643	0.808	0.1420	0.427	8.03	41.1	2.22	0.640
362T200-54	1.54	0.641	1.020	0.1770	0.536	15.50	33.4	4.31	0.819
362T200-68	1.55	0.638	1.310	0.2210	0.675	21.60	33.6	6.02	1.120
362T200-97	1.57	0.632	1.920	0.3090	0.963	36.20	34.3	8.48	1.820

4" Track Section Properties

Flange = 1.25" and 2.00"											
Section Identification	Dimension			Weight (lb/ft)	Yield F_y (ksi)	Area (in. ²)	x_{cg} (in.)	x_o (in.)	C_w (in. ⁶)	J (in. ⁴)	j (in.)
	Thickness t (in.)	Depth A (in.)	Flange B (in.)								
400T125-33	0.0346	4.076	1.25	0.765	33	0.225	0.251	0.630	0.0946	0.0000897	2.35
400T125-43	0.0451	4.071	1.25	0.996	33	0.293	0.255	0.626	0.1220	0.0001980	2.35
400T125-54	0.0566	4.085	1.25	1.250	50	0.367	0.259	0.621	0.1540	0.0003920	2.37
400T125-68	0.0713	4.107	1.25	1.570	50	0.462	0.264	0.614	0.1940	0.0007830	2.39
400T125-97	0.1017	4.153	1.25	2.240	50	0.659	0.274	0.600	0.2800	0.0022700	2.44
400T200-33	0.0346	4.076	2.00	0.941	33	0.277	0.509	1.230	0.3360	0.0001100	2.37
400T200-43	0.0451	4.071	2.00	1.230	33	0.360	0.512	1.220	0.4360	0.0002440	2.37
400T200-54	0.0566	4.085	2.00	1.540	50	0.452	0.515	1.220	0.5510	0.0004830	2.38
400T200-68	0.0713	4.107	2.00	1.940	50	0.569	0.519	1.210	0.7020	0.0009650	2.39
400T200-97	0.1017	4.153	2.00	2.760	50	0.811	0.528	1.190	1.0200	0.0028000	2.40

Flange = 1.25" and 2.00"									
Section Identification	r_x (in.)	r_y (in.)	I_x (in. ⁴)	I_y (in. ⁴)	S_r (in. ³)	M_{rx} (in-kips)	L_u (in.)	Shear V_r (kips)	I_x defl. (in. ⁴)
400T125-33	1.56	0.371	0.549	0.0309	0.265	5.97	25.6	1.20	0.480
400T125-43	1.56	0.369	0.716	0.0398	0.344	8.37	25.6	2.22	0.660
400T125-54	1.57	0.367	0.904	0.0494	0.431	16.10	20.8	4.31	0.842
400T125-68	1.58	0.364	1.150	0.0612	0.541	22.00	20.9	6.66	1.130
400T125-97	1.59	0.358	1.670	0.0846	0.768	34.60	21.1	9.39	1.670
400T200-33	1.67	0.639	0.768	0.1130	0.371	6.53	41.0	1.20	0.574
400T200-43	1.67	0.637	1.000	0.1460	0.482	9.23	41.1	2.22	0.799
400T200-54	1.67	0.635	1.270	0.1820	0.604	17.80	33.4	4.31	1.020
400T200-68	1.69	0.632	1.620	0.2270	0.761	24.70	33.6	6.66	1.390
400T200-97	1.71	0.626	2.360	0.3180	1.090	41.00	34.1	9.39	2.240

6" Track Section Properties

Flange = 1.25" and 2.00"											
Section Identification	Dimension			Weight (lb/ft)	Yield F_y (ksi)	Area (in. ²)	x_{cg} (in.)	x_o (in.)	C_w (in. ⁶)	J (in. ⁴)	j (in.)
	Thickness t (in.)	Depth A (in.)	Flange B (in.)								
600T125-33	0.0346	6.076	1.25	1.000	33	0.294	0.196	0.516	0.238	0.000117	4.22
600T125-43	0.0451	6.071	1.25	1.300	33	0.383	0.200	0.513	0.307	0.000260	4.24
600T125-54	0.0566	6.085	1.25	1.630	50	0.480	0.204	0.508	0.384	0.000513	4.27
600T125-68	0.0713	6.107	1.25	2.060	50	0.605	0.210	0.503	0.483	0.001030	4.32
600T125-97	0.1017	6.153	1.25	2.930	50	0.862	0.221	0.491	0.685	0.002970	4.41
600T200-33	0.0346	6.076	2.00	1.180	33	0.346	0.411	1.050	0.847	0.000138	3.44
600T200-43	0.0451	6.071	2.00	1.530	33	0.451	0.414	1.040	1.100	0.000305	3.45
600T200-54	0.0566	6.085	2.00	1.920	50	0.565	0.418	1.040	1.380	0.000604	3.46
600T200-68	0.0713	6.107	2.00	2.420	50	0.712	0.422	1.030	1.750	0.001210	3.48
600T200-97	0.1017	6.153	2.00	3.450	50	1.010	0.432	1.020	2.510	0.003500	3.52

Flange = 1.25" and 2.00"									
Section Identification	r_x (in.)	r_y (in.)	I_x (in. ⁴)	I_y (in. ⁴)	S_r (in. ³)	M_{rx} (in-kips)	L_u (in.)	Shear V_r (kips)	I_x defl. (in. ⁴)
600T125-33	2.20	0.339	1.430	0.0338	0.465	8.81	24.8	0.795	1.20
600T125-43	2.20	0.337	1.860	0.0436	0.604	13.70	24.8	1.760	1.72
600T125-54	2.21	0.335	2.340	0.0540	0.756	26.60	20.1	3.480	2.19
600T125-68	2.22	0.333	2.970	0.0670	0.950	38.60	20.1	6.840	2.92
600T125-97	2.23	0.327	4.280	0.0925	1.350	60.60	20.1	13.900	4.28
600T200-33	2.35	0.604	1.910	0.1260	0.622	9.89	40.9	0.795	1.50
600T200-43	2.35	0.602	2.490	0.1630	0.809	16.80	40.9	1.760	2.06
600T200-54	2.36	0.600	3.140	0.2040	1.010	32.30	33.2	3.480	2.62
600T200-68	2.37	0.597	3.990	0.2540	1.280	43.80	33.3	6.840	3.51
600T200-97	2.39	0.592	5.770	0.3550	1.820	70.50	33.5	13.900	5.51

8" Track Section Properties

Flange = 1.25" and 2.00"											
Section Identification	Dimension			Weight (lb/ft)	Yield F_y (ksi)	Area (in. ²)	x_{cg} (in.)	x_o (in.)	C_w (in. ⁶)	J (in. ⁴)	j (in.)
	Thickness t (in.)	Depth A (in.)	Flange B (in.)								
800T125-43	0.0451	8.071	1.25	1.61	33	0.473	0.166	0.436	0.589	0.000321	6.95
800T125-54	0.0566	8.085	1.25	2.02	50	0.594	0.171	0.432	0.735	0.000634	7.01
800T125-68	0.0713	8.107	1.25	2.54	50	0.748	0.177	0.427	0.920	0.001270	7.08
800T125-97	0.1017	8.153	1.25	3.63	50	1.070	0.189	0.417	1.300	0.003670	7.21
800T200-43	0.0451	8.071	2.00	1.84	33	0.541	0.349	0.913	2.120	0.000367	5.04
600T200-54	0.0566	8.085	2.00	2.31	50	0.679	0.353	0.908	2.660	0.000725	5.07
800T200-68	0.0713	8.107	2.00	2.91	50	0.854	0.358	0.902	3.360	0.001450	5.10
800T200-97	0.1017	8.153	2.00	4.15	50	1.220	0.369	0.889	4.790	0.004200	5.17

Flange = 1.25" and 2.00"									
Section Identification	r_x (in.)	r_y (in.)	I_x (in. ⁴)	I_y (in. ⁴)	S_r (in. ³)	M_{rx} (in-kips)	L_u (in.)	Shear V_r (kips)	I_x defl. (in. ⁴)
800T125-43	2.82	0.311	3.770	0.0459	0.924	19.00	23.8	1.320	3.34
800T125-54	2.83	0.310	4.750	0.0569	1.160	37.00	19.3	2.600	4.26
800T125-68	2.83	0.307	6.000	0.0705	1.450	54.70	19.2	5.220	5.83
800T125-97	2.84	0.302	8.610	0.0974	2.060	92.80	19.2	13.900	8.61
800T200-43	3.01	0.569	4.890	0.1750	1.200	20.10	40.3	1.320	3.82
600T200-54	3.01	0.567	6.150	0.2180	1.500	39.20	32.7	2.600	4.88
800T200-68	3.02	0.564	7.790	0.2720	1.890	58.90	32.7	5.220	6.81
800T200-97	3.03	0.558	11.200	0.3800	2.680	106.00	32.7	13.900	10.80

3 5/8" DeltaStud Section Properties

Flange = 1.625"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
362D162-33	0.0346	3.625	1.625	0.500	0.892	33	0.262	0.537	0.787	-1.307	0.284	0.000105	2.073	1.450	0.616
362D162-43	0.0451	3.625	1.625	0.500	1.156	33	0.340	0.537	0.780	-1.295	0.360	0.000230	2.064	1.445	0.611
362D162-54	0.0566	3.625	1.625	0.500	1.436	50	0.422	0.536	0.772	-1.280	0.433	0.000451	2.055	1.438	0.605
362D162-68	0.0713	3.625	1.625	0.500	1.782	50	0.524	0.535	0.761	-1.260	0.514	0.000887	2.043	1.429	0.596
362D162-97	0.1017	3.625	1.625	0.500	2.464	50	0.724	0.532	0.737	-1.217	0.644	0.002496	2.019	1.409	0.578

Flange = 1.625"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
362D162-33	8.682	42.1	2.56	3.07	1.310	0.318	0.551	0.099	0.304
362D162-43	12.750	42.1	3.38	4.02	2.226	0.531	0.710	0.127	0.392
362D162-54	23.019	33.9	6.24	7.45	4.316	1.197	0.873	0.154	0.482
362D162-68	29.905	33.8	7.72	9.29	5.593	1.779	1.069	0.186	0.590
362D162-97	42.263	33.6	12.83	12.83	7.607	2.398	1.437	0.242	0.793

Flange = 2"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
362D200-33	0.0346	3.625	2.000	0.625	1.010	33	0.297	0.728	1.030	-1.741	0.558	0.000118	2.337	1.478	0.772
362D200-43	0.0451	3.625	2.000	0.625	1.310	33	0.385	0.728	1.023	-1.729	0.709	0.000261	2.326	1.474	0.768
362D200-54	0.0566	3.625	2.000	0.625	1.628	50	0.479	0.727	1.015	-1.714	0.858	0.000511	2.315	1.467	0.761
362D200-68	0.0713	3.625	2.000	0.625	2.025	50	0.595	0.726	1.004	-1.694	1.029	0.001008	2.300	1.459	0.753
362D200-97	0.1017	3.625	2.000	0.625	2.810	50	0.826	0.722	0.981	-1.653	1.320	0.002847	2.269	1.440	0.736

Flange = 2"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
362D200-33	9.538	53.1	3.85	3.97	1.310	0.318	0.643	0.177	0.358
362D200-43	13.342	53.1	5.11	5.98	2.226	0.531	0.836	0.227	0.461
362D200-54	23.422	42.9	9.50	11.15	4.316	1.197	1.030	0.277	0.568
362D200-68	33.692	42.8	11.85	13.94	5.593	1.779	1.266	0.338	0.698
362D200-97	49.014	42.8	19.45	19.45	7.607	2.398	1.713	0.447	0.945

3 5/8" DeltaStud Section Properties

Flange = 2.50"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
362D250-33	0.0346	3.625	2.500	0.625	1.127	33	0.331	0.944	1.284	-2.211	0.936	0.000132	2.736	1.514	0.951
362D250-43	0.0451	3.625	2.500	0.625	1.463	33	0.430	0.944	1.277	-2.198	1.192	0.000292	2.725	1.510	0.946
362D250-54	0.0566	3.625	2.500	0.625	1.821	50	0.535	0.943	1.268	-2.183	1.448	0.000571	2.712	1.504	0.940
362D250-68	0.0713	3.625	2.500	0.625	2.267	50	0.666	0.941	1.257	-2.163	1.746	0.001129	2.695	1.496	0.931
362D250-97	0.1017	3.625	2.500	0.625	3.156	50	0.927	0.938	1.234	-2.121	2.267	0.003197	2.659	1.479	0.914

Flange = 2.50"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
362D250-33	10.266	63.7	5.29	5.52	1.310	0.318	0.726	0.299	0.419
362D250-43	14.160	63.6	7.04	8.30	2.226	0.531	0.980	0.385	0.541
362D250-54	24.745	51.5	13.16	15.53	4.316	1.197	1.200	0.473	0.668
362D250-68	32.346	51.4	16.52	19.52	5.593	1.779	1.491	0.578	0.823
362D250-97	54.037	51.5	27.53	27.53	7.607	2.398	2.029	0.774	1.119

Flange = 3"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
362D300-33	0.0346	3.625	3.000	0.625	1.245	33	0.366	1.167	1.536	-2.686	1.436	0.000146	3.162	1.543	1.125
362D300-43	0.0451	3.625	3.000	0.625	1.617	33	0.475	1.167	1.529	-2.673	1.834	0.000322	3.150	1.539	1.120
362D300-54	0.0566	3.625	3.000	0.625	2.013	50	0.592	1.166	1.521	-2.658	2.234	0.000632	3.136	1.533	1.114
362D300-68	0.0713	3.625	3.000	0.625	2.510	50	0.738	1.164	1.510	-2.638	2.704	0.001250	3.118	1.525	1.105
362D300-97	0.1017	3.625	3.000	0.625	3.502	50	1.029	1.160	1.486	-2.595	3.541	0.003548	3.079	1.509	1.086

Flange = 3"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
362D300-33	10.745	73.8	6.90	7.20	1.310	0.318	0.796	0.463	0.481
362D300-43	14.606	73.8	9.21	10.90	2.226	0.531	1.091	0.596	0.621
362D300-54	25.651	59.7	17.26	20.46	4.316	1.197	1.298	0.734	0.767
362D300-68	33.785	59.7	21.78	25.81	5.593	1.779	1.682	0.901	0.947
362D300-97	53.328	59.9	36.69	36.69	7.607	2.398	2.322	1.215	1.294

4" DeltaStud Section Properties

Flange = 1.625"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
400D162-33	0.0346	4.000	1.625	0.500	0.936	33	0.275	0.512	0.766	-1.261	0.347	0.000110	2.203	1.586	0.611
400D162-43	0.0451	4.000	1.625	0.500	1.214	33	0.357	0.513	0.759	-1.249	0.440	0.000242	2.194	1.581	0.606
400D162-54	0.0566	4.000	1.625	0.500	1.508	50	0.443	0.512	0.751	-1.235	0.530	0.000473	2.187	1.574	0.600
400D162-68	0.0713	4.000	1.625	0.500	1.873	50	0.550	0.511	0.740	-1.215	0.630	0.000933	2.178	1.564	0.592
400D162-97	0.1017	4.000	1.625	0.500	2.593	50	0.762	0.508	0.716	-1.173	0.793	0.002628	2.159	1.543	0.573

Flange = 1.625"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
400D162-33	9.890	41.8	2.57	3.10	1.249	0.314	0.692	0.103	0.346
400D162-43	14.515	41.7	3.39	4.06	2.226	0.525	0.892	0.131	0.446
400D162-54	26.232	33.6	6.27	7.54	4.316	1.186	1.098	0.160	0.549
400D162-68	34.116	33.4	7.74	9.40	6.235	1.764	1.347	0.193	0.673
400D162-97	48.373	33.2	12.99	12.99	8.522	2.380	1.815	0.251	0.907

Flange = 2"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
400D200-33	0.0346	4.000	2.000	0.625	1.054	33	0.310	0.698	1.006	-1.687	0.673	0.000124	2.425	1.619	0.769
400D200-43	0.0451	4.000	2.000	0.625	1.367	33	0.402	0.698	0.999	-1.675	0.856	0.000272	2.415	1.615	0.765
400D200-54	0.0566	4.000	2.000	0.625	1.700	50	0.500	0.697	0.991	-1.660	1.038	0.000534	2.404	1.608	0.758
400D200-68	0.0713	4.000	2.000	0.625	2.116	50	0.622	0.696	0.980	-1.641	1.246	0.001054	2.391	1.599	0.750
400D200-97	0.1017	4.000	2.000	0.625	2.939	50	0.864	0.693	0.957	-1.599	1.603	0.002978	2.363	1.580	0.733

Flange = 2"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
400D200-33	10.855	52.7	3.86	4.02	1.249	0.314	0.805	0.183	0.406
400D200-43	15.161	52.6	5.13	6.05	2.226	0.525	1.048	0.235	0.524
400D200-54	26.629	42.5	9.54	11.28	4.316	1.186	1.292	0.287	0.646
400D200-68	38.307	42.3	11.92	14.12	6.235	1.764	1.590	0.350	0.795
400D200-97	55.922	42.2	19.71	19.71	8.522	2.380	2.157	0.464	1.079

4" DeltaStud Section Properties

Flange = 2.50"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
400D250-33	0.0346	4.000	2.500	0.625	1.172	33	0.344	0.909	1.258	-2.150	1.129	0.000137	2.797	1.659	0.949
400D250-43	0.0451	4.000	2.500	0.625	1.521	33	0.447	0.909	1.251	-2.138	1.440	0.000303	2.786	1.655	0.945
400D250-54	0.0566	4.000	2.500	0.625	1.893	50	0.556	0.908	1.243	-2.123	1.752	0.000594	2.774	1.649	0.938
400D250-68	0.0713	4.000	2.500	0.625	2.358	50	0.693	0.906	1.232	-2.103	2.115	0.001174	2.758	1.640	0.930
400D250-97	0.1017	4.000	2.500	0.625	3.286	50	0.966	0.903	1.208	-2.060	2.753	0.003329	2.724	1.623	0.912

Flange = 2.50"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
400D250-33	11.667	63.3	5.30	5.59	1.249	0.314	0.906	0.310	0.474
400D250-43	16.059	63.2	7.07	8.41	2.226	0.525	1.224	0.399	0.612
400D250-54	28.107	51.1	13.21	15.74	4.316	1.186	1.498	0.490	0.756
400D250-68	36.695	51.0	16.63	19.79	6.235	1.764	1.865	0.599	0.933
400D250-97	61.330	50.9	27.93	27.93	8.522	2.380	2.543	0.803	1.272

Flange = 3"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
400D300-33	0.0346	4.000	3.000	0.625	1.289	33	0.379	1.128	1.510	-2.620	1.735	0.000151	3.204	1.692	1.125
400D300-43	0.0451	4.000	3.000	0.625	1.674	33	0.492	1.128	1.502	-2.607	2.216	0.000334	3.192	1.687	1.120
400D300-54	0.0566	4.000	3.000	0.625	2.086	50	0.613	1.126	1.494	-2.592	2.703	0.000655	3.178	1.681	1.114
400D300-68	0.0713	4.000	3.000	0.625	2.601	50	0.764	1.124	1.483	-2.572	3.275	0.001295	3.161	1.673	1.105
400D300-97	0.1017	4.000	3.000	0.625	3.632	50	1.067	1.120	1.459	-2.529	4.301	0.003679	3.123	1.657	1.087

Flange = 3"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
400D300-33	12.205	73.4	6.92	7.26	1.249	0.314	0.991	0.479	0.542
400D300-43	16.557	73.4	9.25	11.06	2.226	0.525	1.357	0.618	0.700
400D300-54	29.128	59.4	17.33	20.76	4.316	1.186	1.616	0.760	0.866
400D300-68	38.316	59.3	21.92	26.19	6.235	1.764	2.095	0.933	1.070
400D300-97	60.351	59.3	37.25	37.25	8.522	2.380	2.899	1.260	1.465

6" DeltaStud Section Properties

Flange = 1.625"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
600D162-33	0.0346	6.000	1.625	0.500	1.172	33	0.344	0.413	0.673	-1.068	0.827	0.000137	3.236	2.282	0.581
600D162-43	0.0451	6.000	1.625	0.500	1.521	33	0.447	0.414	0.666	-1.058	1.051	0.000303	3.235	2.277	0.576
600D162-54	0.0566	6.000	1.625	0.500	1.893	50	0.556	0.414	0.658	-1.043	1.270	0.000594	3.240	2.268	0.570
600D162-68	0.0713	6.000	1.625	0.500	2.358	50	0.693	0.413	0.647	-1.025	1.519	0.001174	3.248	2.255	0.561
600D162-97	0.1017	6.000	1.625	0.500	3.286	50	0.966	0.411	0.624	-0.985	1.936	0.003329	3.266	2.230	0.543

Flange = 1.625"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
600D162-33	17.162	40.7	2.59	3.22	0.817	0.295	1.793	0.116	0.598
600D162-43	25.107	40.5	3.43	4.22	1.812	0.499	2.316	0.148	0.772
600D162-54	45.634	32.6	6.36	7.84	3.613	1.134	2.861	0.181	0.954
600D162-68	59.476	32.3	7.89	9.79	6.848	1.696	3.526	0.218	1.175
600D162-97	85.308	31.7	11.76	13.57	13.404	2.300	4.800	0.284	1.600

Flange = 2"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
600D200-33	0.0346	6.000	2.000	0.625	1.289	33	0.379	0.574	0.898	-1.454	1.542	0.000151	3.178	2.340	0.743
600D200-43	0.0451	6.000	2.000	0.625	1.674	33	0.492	0.574	0.891	-1.443	1.967	0.000334	3.173	2.335	0.739
600D200-54	0.0566	6.000	2.000	0.625	2.086	50	0.613	0.574	0.883	-1.429	2.392	0.000655	3.170	2.327	0.732
600D200-68	0.0713	6.000	2.000	0.625	2.601	50	0.764	0.573	0.872	-1.410	2.886	0.001295	3.167	2.316	0.724
600D200-97	0.1017	6.000	2.000	0.625	3.632	50	1.067	0.570	0.850	-1.369	3.753	0.003679	3.161	2.294	0.706

Flange = 2"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
600D200-33	18.479	51.2	3.89	4.19	0.817	0.295	2.059	0.209	0.692
600D200-43	25.953	51.0	5.19	6.31	1.812	0.499	2.683	0.269	0.894
600D200-54	45.823	41.2	9.67	11.79	3.613	1.134	3.320	0.329	1.107
600D200-68	65.812	40.9	12.14	14.76	6.848	1.696	4.101	0.400	1.367
600D200-97	97.044	40.4	16.69	20.67	13.404	2.300	5.615	0.532	1.872

6" DeltaStud Section Properties

Flange = 2.50"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
600D250-33	0.0346	6.000	2.500	0.625	1.407	33	0.413	0.760	1.140	-1.883	2.586	0.000165	3.358	2.401	0.928
600D250-43	0.0451	6.000	2.500	0.625	1.828	33	0.537	0.760	1.133	-1.871	3.308	0.000364	3.350	2.396	0.923
600D250-54	0.0566	6.000	2.500	0.625	2.278	50	0.670	0.759	1.125	-1.856	4.037	0.000715	3.343	2.388	0.917
600D250-68	0.0713	6.000	2.500	0.625	2.843	50	0.836	0.758	1.114	-1.836	4.897	0.001416	3.334	2.379	0.908
600D250-97	0.1017	6.000	2.500	0.625	3.978	50	1.169	0.754	1.091	-1.795	6.443	0.004030	3.316	2.358	0.889

Flange = 2.50"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
600D250-33	19.299	62.1	5.35	5.87	0.817	0.295	2.293	0.356	0.794
600D250-43	27.307	61.9	7.16	8.82	1.812	0.499	3.083	0.458	1.028
600D250-54	48.188	50.0	13.39	16.53	3.613	1.134	3.769	0.563	1.273
600D250-68	62.564	49.8	16.95	20.81	6.848	1.696	4.728	0.689	1.576
600D250-97	104.422	49.3	23.67	29.44	13.404	2.300	6.499	0.925	2.166

Flange = 3"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
600D300-33	0.0346	6.000	3.000	0.625	1.525	33	0.448	0.956	1.384	-2.323	3.971	0.000179	3.630	2.451	1.110
600D300-43	0.0451	6.000	3.000	0.625	1.981	33	0.582	0.956	1.377	-2.311	5.089	0.000395	3.619	2.446	1.105
600D300-54	0.0566	6.000	3.000	0.625	2.471	50	0.726	0.955	1.369	-2.296	6.229	0.000775	3.610	2.439	1.098
600D300-68	0.0713	6.000	3.000	0.625	3.086	50	0.907	0.953	1.358	-2.275	7.583	0.001537	3.597	2.430	1.089
600D300-97	0.1017	6.000	3.000	0.625	4.324	50	1.271	0.949	1.334	-2.233	10.058	0.004381	3.572	2.411	1.070

Flange = 3"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
600D300-33	19.720	72.5	6.98	7.46	0.817	0.295	2.495	0.552	0.897
600D300-43	28.109	72.4	9.37	11.66	1.812	0.499	3.369	0.711	1.161
600D300-54	49.888	58.5	17.57	21.90	3.613	1.134	4.020	0.876	1.440
600D300-68	65.235	58.3	22.34	27.66	6.848	1.696	5.225	1.076	1.785
600D300-97	101.602	57.9	31.57	39.44	13.404	2.300	7.290	1.456	2.461

8" DeltaStud Section Properties

Flange = 1.625"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
800D162-33	0.0346	8.000	1.625	0.500	1.407	33	0.413	0.347	0.601	-0.931	1.567	0.000165	4.830	2.943	0.550
800D162-43	0.0451	8.000	1.625	0.500	1.828	33	0.537	0.348	0.595	-0.921	1.994	0.000364	4.840	2.937	0.546
800D162-54	0.0566	8.000	1.625	0.500	2.278	50	0.670	0.348	0.587	-0.908	2.415	0.000715	4.862	2.927	0.539
800D162-68	0.0713	8.000	1.625	0.500	2.843	50	0.836	0.349	0.577	-0.890	2.897	0.001416	4.895	2.913	0.530
800D162-97	0.1017	8.000	1.625	0.500	3.978	50	1.169	0.349	0.555	-0.853	3.717	0.004030	4.966	2.883	0.512

Flange = 1.625"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
800D162-33	21.100	39.7	2.60	3.29	0.607	0.189	3.385	0.125	0.896
800D162-43	30.275	39.5	3.46	4.31	1.345	0.324	4.501	0.160	1.158
800D162-54	55.368	31.7	6.40	8.02	2.676	0.754	5.600	0.195	1.434
800D162-68	74.874	31.4	7.96	10.01	5.401	1.158	7.071	0.235	1.773
800D162-97	124.701	30.6	10.72	13.90	13.933	2.233	9.717	0.306	2.429

Flange = 2"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
800D200-33	0.0346	8.000	2.000	0.625	1.525	33	0.448	0.488	0.813	-1.284	2.878	0.000179	4.393	3.023	0.713
800D200-43	0.0451	8.000	2.000	0.625	1.981	33	0.582	0.489	0.807	-1.273	3.677	0.000395	4.394	3.018	0.708
800D200-54	0.0566	8.000	2.000	0.625	2.471	50	0.726	0.489	0.799	-1.259	4.480	0.000775	4.402	3.009	0.701
800D200-68	0.0713	8.000	2.000	0.625	3.086	50	0.907	0.488	0.788	-1.241	5.422	0.001537	4.414	2.996	0.693
800D200-97	0.1017	8.000	2.000	0.625	4.324	50	1.271	0.487	0.766	-1.202	7.094	0.004381	4.442	2.970	0.674

Flange = 2"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
800D200-33	24.254	50.2	3.91	4.30	0.607	0.189	4.096	0.227	1.024
800D200-43	38.453	49.9	5.22	6.47	1.345	0.324	5.302	0.292	1.326
800D200-54	67.640	40.3	9.73	12.09	2.676	0.754	6.573	0.357	1.643
800D200-68	98.130	39.9	12.25	15.15	5.401	1.158	8.142	0.435	2.035
800D200-97	145.275	39.3	16.92	21.23	13.933	2.233	11.207	0.578	2.802

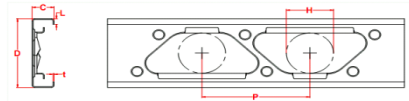
8" DeltaStud Section Properties

Flange = 2.50"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
800D250-33	0.0346	8.000	2.500	0.625	1.642	33	0.483	0.654	1.045	-1.682	4.830	0.000193	4.302	3.102	0.897
800D250-43	0.0451	8.000	2.500	0.625	2.134	33	0.627	0.654	1.039	-1.671	6.188	0.000425	4.298	3.097	0.893
800D250-54	0.0566	8.000	2.500	0.625	2.663	50	0.783	0.654	1.031	-1.656	7.568	0.000836	4.299	3.088	0.886
800D250-68	0.0713	8.000	2.500	0.625	3.329	50	0.978	0.653	1.020	-1.637	9.205	0.001658	4.301	3.077	0.877
800D250-97	0.1017	8.000	2.500	0.625	4.670	50	1.372	0.650	0.997	-1.596	12.183	0.004731	4.306	3.053	0.858

Flange = 2.50"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
800D250-33	25.505	61.3	5.37	6.04	0.607	0.189	4.529	0.389	1.161
800D250-43	39.076	61.1	7.20	9.08	1.345	0.324	6.016	0.500	1.504
800D250-54	68.772	49.3	13.48	17.02	2.676	0.754	7.383	0.615	1.866
800D250-68	92.860	49.0	17.10	21.44	5.401	1.158	9.252	0.752	2.316
800D250-97	154.501	48.4	24.02	30.37	13.933	2.233	12.793	1.011	3.198

Flange = 3"															
Section Identification	Dimensions				Properties										
	Thickness	Depth	Flange	Lip	Weight	Yield	Area	x_{cg}	m	x_o	Cw	J	j	r_x	r_y
	t (in.)	A (in.)	B (in.)	C (in.)	(lbs/ft)	F_y (ksi)	(in. ²)	(in.)	(in.)	(in.)	(in. ⁶)	(in. ⁴)	(in.)	(in.)	(in.)
800D300-33	0.0346	8.000	3.000	0.625	1.760	33	0.517	0.831	1.281	-2.094	7.416	0.000206	4.385	3.169	1.081
800D300-43	0.0451	8.000	3.000	0.625	2.288	33	0.672	0.831	1.274	-2.083	9.519	0.000456	4.379	3.164	1.076
800D300-54	0.0566	8.000	3.000	0.625	2.856	50	0.839	0.830	1.266	-2.068	11.674	0.000896	4.374	3.156	1.069
800D300-68	0.0713	8.000	3.000	0.625	3.571	50	1.050	0.829	1.255	-2.048	14.250	0.001778	4.370	3.145	1.060
800D300-97	0.1017	8.000	3.000	0.625	5.016	50	1.474	0.825	1.232	-2.006	19.010	0.005082	4.361	3.123	1.041

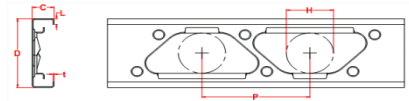
Flange = 3"									
Section Identification	Properties								
	$M_{rx, LB}$	L_u	$M_{ry, LB}$	$M_{ry, LB}$	Shear	Web Cripp.	I_x	I_y	Sf
	(in-kips)	(in.)	web comp (in-kips)	lips comp (in-kips)	V_r (kips)	P_t (kips)	defl (in. ⁴)	(in. ⁴)	(in. ³)
800D300-33	26.199	72.0	7.01	7.56	0.607	0.189	4.900	0.604	1.298
800D300-43	39.055	71.8	9.42	12.04	1.345	0.324	6.566	0.779	1.682
800D300-54	69.212	58.1	17.67	22.62	2.676	0.754	7.872	0.960	2.090
800D300-68	96.670	57.8	22.54	28.59	5.401	1.158	10.105	1.180	2.596
800D300-97	149.144	57.1	32.05	40.81	13.933	2.233	14.185	1.597	3.595



3-5/8" DeltaStud 1-5/8" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
362D162-33	Strength Only	12	28.8	20.3	16.6	14.4	12.9	11.7	10.9	10.2	9.1
		16	24.9	17.6	14.4	12.5	11.1	10.2	9.4	8.5	6.8
		24	20.3	14.4	11.7	10.2	9.1	7.6	6.5	5.7	4.5
	Deflection (L/360)	12	16.9	13.4	11.7	10.6	9.9	9.3	8.8	8.4	7.8
		16	15.3	12.2	10.6	9.7	9.0	8.4	8.0	7.7	7.1
		24	13.4	10.6	9.3	8.4	7.8	7.4	7.0	6.7	6.2
	Deflection (L/600)	12	14.2	11.3	9.9	9.0	8.3	7.8	7.4	7.1	6.6
		16	12.9	10.3	9.0	8.2	7.6	7.1	6.8	6.5	6.0
		24	11.3	9.0	7.8	7.1	6.6	6.2	5.9	5.7	5.2
	Deflection (L/720)	12	13.4	10.6	9.3	8.4	7.8	7.4	7.0	6.7	6.2
		16	12.2	9.7	8.4	7.7	7.1	6.7	6.4	6.1	5.7
		24	10.6	8.4	7.4	6.7	6.2	5.9	5.6	5.3	4.9
362D162-43	Strength Only	12	34.8	24.6	20.1	17.4	15.6	14.2	13.2	12.3	11.0
		16	30.2	21.3	17.4	15.1	13.5	12.3	11.4	10.7	9.5
		24	24.6	17.4	14.2	12.3	11.0	10.1	9.3	8.7	7.6
	Deflection (L/360)	12	18.4	14.6	12.7	11.6	10.7	10.1	9.6	9.2	8.5
		16	16.7	13.3	11.6	10.5	9.8	9.2	8.7	8.3	7.7
		24	14.6	11.6	10.1	9.2	8.5	8.0	7.6	7.3	6.8
	Deflection (L/600)	12	15.5	12.3	10.7	9.8	9.1	8.5	8.1	7.7	7.2
		16	14.1	11.2	9.8	8.9	8.2	7.7	7.4	7.0	6.5
		24	12.3	9.8	8.5	7.7	7.2	6.8	6.4	6.2	5.7
	Deflection (L/720)	12	14.6	11.6	10.1	9.2	8.5	8.0	7.6	7.3	6.8
		16	13.3	10.5	9.2	8.3	7.7	7.3	6.9	6.6	6.2
		24	11.6	9.2	8.0	7.3	6.8	6.4	6.1	5.8	5.4

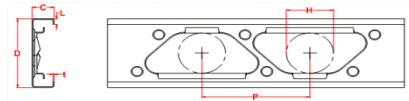
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



3-5/8" DeltaStud 1-5/8" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
362D162-54	Strength Only	12	46.8	33.1	27.0	23.4	20.9	19.1	17.7	16.6	14.8
		16	40.5	28.7	23.4	20.3	18.1	16.6	15.3	14.3	12.8
		24	33.1	23.4	19.1	16.6	14.8	13.5	12.5	11.7	10.5
	Deflection (L/360)	12	19.7	15.6	13.7	12.4	11.5	10.8	10.3	9.8	9.1
		16	17.9	14.2	12.4	11.3	10.5	9.8	9.4	8.9	8.3
		24	15.6	12.4	10.8	9.8	9.1	8.6	8.2	7.8	7.3
	Deflection (L/600)	12	16.6	13.2	11.5	10.5	9.7	9.1	8.7	8.3	7.7
		16	15.1	12.0	10.5	9.5	8.8	8.3	7.9	7.5	7.0
		24	13.2	10.5	9.1	8.3	7.7	7.3	6.9	6.6	6.1
	Deflection (L/720)	12	15.6	12.4	10.8	9.8	9.1	8.6	8.2	7.8	7.3
		16	14.2	11.3	9.8	8.9	8.3	7.8	7.4	7.1	6.6
		24	12.4	9.8	8.6	7.8	7.3	6.8	6.5	6.2	5.8
362D162-68	Strength Only	12	53.4	37.7	30.8	26.7	23.9	21.8	20.2	18.9	16.9
		16	46.2	32.7	26.7	23.1	20.7	18.9	17.5	16.3	14.6
		24	37.7	26.7	21.8	18.9	16.9	15.4	14.3	13.3	11.9
	Deflection (L/360)	12	21.1	16.7	14.6	13.3	12.3	11.6	11.0	10.5	9.8
		16	19.1	15.2	13.3	12.1	11.2	10.5	10.0	9.6	8.9
		24	16.7	13.3	11.6	10.5	9.8	9.2	8.7	8.4	7.8
	Deflection (L/600)	12	17.8	14.1	12.3	11.2	10.4	9.8	9.3	8.9	8.2
		16	16.1	12.8	11.2	10.2	9.4	8.9	8.4	8.1	7.5
		24	14.1	11.2	9.8	8.9	8.2	7.8	7.4	7.1	6.5
	Deflection (L/720)	12	16.7	13.3	11.6	10.5	9.8	9.2	8.7	8.4	7.8
		16	15.2	12.1	10.5	9.6	8.9	8.4	7.9	7.6	7.1
		24	13.3	10.5	9.2	8.4	7.8	7.3	6.9	6.6	6.2

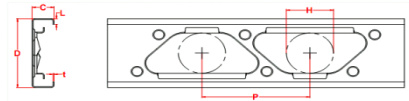
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



4" DeltaStud 1-5/8" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
400D162-33	Strength Only	12	30.7	21.7	17.7	15.3	13.7	12.5	11.6	10.9	9.0
		16	26.6	18.8	15.3	13.3	11.9	10.9	9.6	8.4	6.7
		24	21.7	15.3	12.5	10.9	9.0	7.5	6.4	5.6	4.5
	Deflection (L/360)	12	18.2	14.5	12.6	11.5	10.7	10.0	9.5	9.1	8.5
		16	16.6	13.1	11.5	10.4	9.7	9.1	8.7	8.3	7.7
		24	14.5	11.5	10.0	9.1	8.5	8.0	7.6	7.2	6.7
	Deflection (L/600)	12	15.4	12.2	10.7	9.7	9.0	8.5	8.0	7.7	7.1
		16	14.0	11.1	9.7	8.8	8.2	7.7	7.3	7.0	6.5
		24	12.2	9.7	8.5	7.7	7.1	6.7	6.4	6.1	5.7
	Deflection (L/720)	12	14.5	11.5	10.0	9.1	8.5	8.0	7.6	7.2	6.7
		16	13.1	10.4	9.1	8.3	7.7	7.2	6.9	6.6	6.1
		24	11.5	9.1	8.0	7.2	6.7	6.3	6.0	5.7	5.3
400D162-43	Strength Only	12	37.2	26.3	21.5	18.6	16.6	15.2	14.1	13.1	11.8
		16	32.2	22.8	18.6	16.1	14.4	13.1	12.2	11.4	10.2
		24	26.3	18.6	15.2	13.1	11.8	10.7	9.9	9.3	7.5
	Deflection (L/360)	12	19.8	15.7	13.7	12.5	11.6	10.9	10.4	9.9	9.2
		16	18.0	14.3	12.5	11.3	10.5	9.9	9.4	9.0	8.4
		24	15.7	12.5	10.9	9.9	9.2	8.7	8.2	7.9	7.3
	Deflection (L/600)	12	16.7	13.3	11.6	10.5	9.8	9.2	8.7	8.4	7.8
		16	15.2	12.1	10.5	9.6	8.9	8.4	7.9	7.6	7.1
		24	13.3	10.5	9.2	8.4	7.8	7.3	6.9	6.6	6.2
	Deflection (L/720)	12	15.7	12.5	10.9	9.9	9.2	8.7	8.2	7.9	7.3
		16	14.3	11.3	9.9	9.0	8.4	7.9	7.5	7.1	6.6
		24	12.5	9.9	8.7	7.9	7.3	6.9	6.5	6.2	5.8

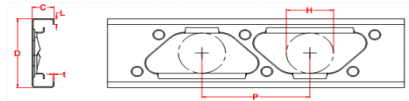
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



4" DeltaStud 1-5/8" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
400D162-54	Strength Only	12	50.0	35.3	28.9	25.0	22.4	20.4	18.9	17.7	15.8
		16	43.3	30.6	25.0	21.6	19.4	17.7	16.4	15.3	13.7
		24	35.3	25.0	20.4	17.7	15.8	14.4	13.4	12.5	11.2
	Deflection (L/360)	12	21.3	16.9	14.7	13.4	12.4	11.7	11.1	10.6	9.9
		16	19.3	15.3	13.4	12.2	11.3	10.6	10.1	9.7	9.0
		24	16.9	13.4	11.7	10.6	9.9	9.3	8.8	8.4	7.8
	Deflection (L/600)	12	17.9	14.2	12.4	11.3	10.5	9.9	9.4	9.0	8.3
		16	16.3	12.9	11.3	10.3	9.5	9.0	8.5	8.1	7.6
		24	14.2	11.3	9.9	9.0	8.3	7.8	7.4	7.1	6.6
	Deflection (L/720)	12	16.9	13.4	11.7	10.6	9.9	9.3	8.8	8.4	7.8
		16	15.3	12.2	10.6	9.7	9.0	8.4	8.0	7.7	7.1
		24	13.4	10.6	9.3	8.4	7.8	7.4	7.0	6.7	6.2
400D162-68	Strength Only	12	57.0	40.3	32.9	28.5	25.5	23.3	21.5	20.2	18.0
		16	49.4	34.9	28.5	24.7	22.1	20.2	18.7	17.5	15.6
		24	40.3	28.5	23.3	20.2	18.0	16.5	15.2	14.3	12.7
	Deflection (L/360)	12	22.7	18.1	15.8	14.3	13.3	12.5	11.9	11.4	10.6
		16	20.7	16.4	14.3	13.0	12.1	11.4	10.8	10.3	9.6
		24	18.1	14.3	12.5	11.4	10.6	9.9	9.4	9.0	8.4
	Deflection (L/600)	12	19.2	15.2	13.3	12.1	11.2	10.6	10.0	9.6	8.9
		16	17.4	13.8	12.1	11.0	10.2	9.6	9.1	8.7	8.1
		24	15.2	12.1	10.6	9.6	8.9	8.4	8.0	7.6	7.1
	Deflection (L/720)	12	18.1	14.3	12.5	11.4	10.6	9.9	9.4	9.0	8.4
		16	16.4	13.0	11.4	10.3	9.6	9.0	8.6	8.2	7.6
		24	14.3	11.4	9.9	9.0	8.4	7.9	7.5	7.2	6.7

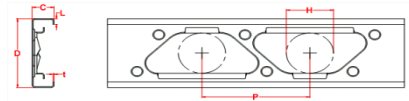
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



6" DeltaStud 1-5/8" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
600D162-33	Strength Only	12	40.4	28.6	23.3	20.2	16.9	14.1	12.1	10.6	8.4
		16	35.0	24.8	20.2	15.8	12.7	10.6	9.0	7.9	6.3
		24	28.6	20.2	14.1	10.6	8.4	7.0	6.0	5.3	4.2
	Deflection (L/360)	12	25.0	19.9	17.4	15.8	14.6	13.8	13.1	12.5	11.6
		16	22.7	18.0	15.8	14.3	13.3	12.5	11.9	11.4	10.6
		24	19.9	15.8	13.8	12.5	11.6	10.9	10.4	9.9	9.2
	Deflection (L/600)	12	21.1	16.8	14.6	13.3	12.3	11.6	11.0	10.6	9.8
		16	19.2	15.2	13.3	12.1	11.2	10.6	10.0	9.6	8.9
		24	16.8	13.3	11.6	10.6	9.8	9.2	8.8	8.4	7.8
	Deflection (L/720)	12	19.9	15.8	13.8	12.5	11.6	10.9	10.4	9.9	9.2
		16	18.0	14.3	12.5	11.4	10.6	9.9	9.4	9.0	8.4
		24	15.8	12.5	10.9	9.9	9.2	8.7	8.2	7.9	7.3
600D162-43	Strength Only	12	48.9	34.6	28.2	24.4	21.9	20.0	18.5	17.3	14.3
		16	42.3	29.9	24.4	21.2	18.9	17.3	15.3	13.4	10.7
		24	34.6	24.4	20.0	17.3	14.3	11.9	10.2	8.9	7.1
	Deflection (L/360)	12	27.3	21.6	18.9	17.2	15.9	15.0	14.2	13.6	12.7
		16	24.8	19.7	17.2	15.6	14.5	13.6	12.9	12.4	11.5
		24	21.6	17.2	15.0	13.6	12.7	11.9	11.3	10.8	10.0
	Deflection (L/600)	12	23.0	18.2	15.9	14.5	13.4	12.7	12.0	11.5	10.7
		16	20.9	16.6	14.5	13.2	12.2	11.5	10.9	10.4	9.7
		24	18.2	14.5	12.7	11.5	10.7	10.0	9.5	9.1	8.5
	Deflection (L/720)	12	21.6	17.2	15.0	13.6	12.7	11.9	11.3	10.8	10.0
		16	19.7	15.6	13.6	12.4	11.5	10.8	10.3	9.8	9.1
		24	17.2	13.6	11.9	10.8	10.0	9.4	9.0	8.6	8.0

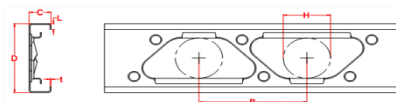
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



6" DeltaStud 1-5/8" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
600D162-54	Strength Only	12	65.9	46.6	38.1	33.0	29.5	26.9	24.9	23.3	20.8
		16	57.1	40.4	33.0	28.5	25.5	23.3	21.6	20.2	18.1
		24	46.6	33.0	26.9	23.3	20.8	19.0	17.6	16.5	14.7
	Deflection (L/360)	12	29.2	23.2	20.3	18.4	17.1	16.1	15.3	14.6	13.6
		16	26.6	21.1	18.4	16.7	15.5	14.6	13.9	13.3	12.3
		24	23.2	18.4	16.1	14.6	13.6	12.8	12.1	11.6	10.8
	Deflection (L/600)	12	24.7	19.6	17.1	15.5	14.4	13.6	12.9	12.3	11.4
		16	22.4	17.8	15.5	14.1	13.1	12.3	11.7	11.2	10.4
		24	19.6	15.5	13.6	12.3	11.4	10.8	10.2	9.8	9.1
	Deflection (L/720)	12	23.2	18.4	16.1	14.6	13.6	12.8	12.1	11.6	10.8
		16	21.1	16.7	14.6	13.3	12.3	11.6	11.0	10.5	9.8
		24	18.4	14.6	12.8	11.6	10.8	10.1	9.6	9.2	8.6
600D162-68	Strength Only	12	75.3	53.2	43.5	37.6	33.7	30.7	28.4	26.6	23.8
		16	65.2	46.1	37.6	32.6	29.1	26.6	24.6	23.0	20.6
		24	53.2	37.6	30.7	26.6	23.8	21.7	20.1	18.8	16.8
	Deflection (L/360)	12	31.4	24.9	21.7	19.8	18.3	17.3	16.4	15.7	14.6
		16	28.5	22.6	19.8	17.9	16.7	15.7	14.9	14.2	13.2
		24	24.9	19.8	17.3	15.7	14.6	13.7	13.0	12.4	11.5
	Deflection (L/600)	12	26.4	21.0	18.3	16.7	15.5	14.6	13.8	13.2	12.3
		16	24.0	19.1	16.7	15.1	14.0	13.2	12.6	12.0	11.2
		24	21.0	16.7	14.6	13.2	12.3	11.5	11.0	10.5	9.7
	Deflection (L/720)	12	24.9	19.8	17.3	15.7	14.6	13.7	13.0	12.4	11.5
		16	22.6	17.9	15.7	14.2	13.2	12.4	11.8	11.3	10.5
		24	19.8	15.7	13.7	12.4	11.5	10.9	10.3	9.9	9.2

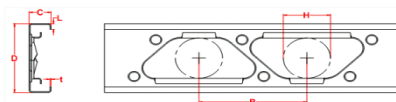
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



8" DeltaStud 1-5/8" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
800D162-33 ⁷	Strength Only	12	44.8	27.0	25.9	22.4	20.0	18.3	16.9	15.8	14.2
		16	38.8	27.5	22.4	19.4	17.4	15.8	14.7	13.7	12.3
		24	31.7	22.4	18.3	15.8	14.2	12.9	12.0	11.2	10.0
	Deflection (L/360)	12	31.3	24.8	21.6	19.6	18.1	17.0	16.2	15.5	14.3
		16	28.4	22.5	19.6	17.7	16.4	15.5	14.7	14.0	13.0
		24	24.8	19.6	17.0	15.5	14.3	13.5	12.8	12.2	11.2
	Deflection (L/600)	12	26.6	21.0	18.3	16.6	15.4	14.5	13.7	13.1	12.2
		16	24.1	19.1	16.6	15.1	14.0	13.1	12.5	11.9	11.0
		24	21.0	16.6	14.5	13.1	12.2	11.4	10.9	10.4	9.6
	Deflection (L/720)	12	25.0	19.8	17.3	15.7	14.5	13.7	13.0	12.4	11.5
		16	22.7	18.0	15.7	14.2	13.2	12.4	11.8	11.2	10.4
		24	19.8	15.7	13.7	12.4	11.5	10.8	10.2	9.8	9.1
800D162-43	Strength Only	12	53.7	38.0	30.9	23.1	18.5	15.4	13.2	11.6	9.3
		16	46.5	32.9	23.1	17.4	13.9	11.6	9.9	8.7	6.9
		24	38.0	23.1	15.4	11.6	9.3	7.7	6.6	5.8	4.6
	Deflection (L/360)	12	34.3	27.3	23.8	21.6	20.0	18.8	17.8	17.1	15.8
		16	31.2	24.7	21.6	19.6	18.1	17.1	16.2	15.5	14.3
		24	27.3	21.6	18.8	17.1	15.8	14.9	14.1	13.5	12.5
	Deflection (L/600)	12	29.0	23.0	20.1	18.2	16.9	15.9	15.1	14.5	13.4
		16	26.3	20.9	18.2	16.6	15.4	14.5	13.7	13.1	12.2
		24	23.0	18.2	15.9	14.5	13.4	12.6	12.0	11.4	10.6
	Deflection (L/720)	12	27.3	21.6	18.9	17.2	15.9	15.0	14.2	13.6	12.6
		16	24.8	19.7	17.2	15.6	14.5	13.6	12.9	12.4	11.5
		24	21.6	17.2	15.0	13.6	12.6	11.9	11.3	10.8	10.0

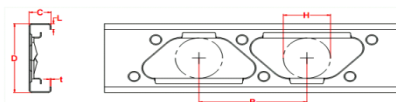
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.
 - 7 - Web stiffeners required.



8" DeltaStud 1-5/8" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
800D162-54	Strength Only	12	84.4	59.7	48.8	42.2	37.8	34.5	31.9	29.9	26.7
		16	73.1	51.7	42.2	36.6	32.7	29.9	27.6	25.9	23.1
		24	59.7	42.2	34.5	29.9	26.7	24.4	22.6	20.7	16.5
	Deflection (L/360)	12	39.6	31.4	27.4	24.9	23.1	21.8	20.7	19.8	18.4
		16	36.0	28.5	24.9	22.7	21.0	19.8	18.8	18.0	16.7
		24	31.4	24.9	21.8	19.8	18.4	17.3	16.4	15.7	14.6
	Deflection (L/600)	12	33.4	26.5	23.1	21.0	19.5	18.4	17.4	16.7	15.5
		16	30.3	24.1	21.0	19.1	17.7	16.7	15.9	15.2	14.1
		24	26.5	21.0	18.4	16.7	15.5	14.6	13.8	13.2	12.3
	Deflection (L/720)	12	31.4	24.9	21.8	19.8	18.4	17.3	16.4	15.7	14.6
		16	28.5	22.7	19.8	18.0	16.7	15.7	14.9	14.3	13.2
		24	24.9	19.8	17.3	15.7	14.6	13.7	13.0	12.5	11.6
800D162-68	Strength Only	12	109.0	77.1	62.9	54.5	48.7	44.5	41.2	38.5	34.5
		16	94.4	66.7	54.5	47.2	42.2	38.5	35.7	33.4	29.8
		24	77.1	54.5	44.5	38.5	34.5	31.5	29.1	27.2	24.4
	Deflection (L/360)	12	44.0	34.9	30.5	27.7	25.7	24.2	23.0	22.0	20.4
		16	39.9	31.7	27.7	25.2	23.4	22.0	20.9	20.0	18.5
		24	34.9	27.7	24.2	22.0	20.4	19.2	18.2	17.4	16.2
	Deflection (L/600)	12	37.1	29.4	25.7	23.4	21.7	20.4	19.4	18.5	17.2
		16	33.7	26.7	23.4	21.2	19.7	18.5	17.6	16.8	15.6
		24	29.4	23.4	20.4	18.5	17.2	16.2	15.4	14.7	13.7
	Deflection (L/720)	12	34.9	27.7	24.2	22.0	20.4	19.2	18.2	17.4	16.2
		16	31.7	25.2	22.0	20.0	18.5	17.4	16.6	15.8	14.7
		24	27.7	22.0	19.2	17.4	16.2	15.2	14.5	13.8	12.9

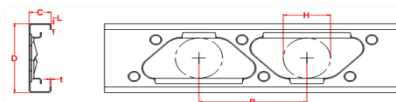
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



3-5/8" DeltaStud 2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
362D200-33	Strength Only	12	30.1	21.3	17.4	15.1	13.5	12.3	11.4	10.7	9.1
		16	26.1	18.5	15.1	13.1	11.7	10.7	9.7	8.5	6.8
		24	21.3	15.1	12.3	10.7	9.1	7.6	6.5	5.7	4.5
	Deflection (L/360)	12	17.8	14.2	12.4	11.2	10.4	9.8	9.3	8.9	8.3
		16	16.2	12.9	11.2	10.2	9.5	8.9	8.5	8.1	7.5
		24	14.2	11.2	9.8	8.9	8.3	7.8	7.4	7.1	6.5
	Deflection (L/600)	12	15.0	11.9	10.4	9.5	8.8	8.3	7.9	7.5	7.0
		16	13.7	10.8	9.5	8.6	8.0	7.5	7.1	6.8	6.3
		24	11.9	9.5	8.3	7.5	7.0	6.6	6.2	6.0	5.5
	Deflection (L/720)	12	14.2	11.2	9.8	8.9	8.3	7.8	7.4	7.1	6.6
		16	12.9	10.2	8.9	8.1	7.5	7.1	6.7	6.4	6.0
		24	11.2	8.9	7.8	7.1	6.6	6.2	5.9	5.6	5.2
362D200-43	Strength Only	12	35.6	25.2	20.6	17.8	15.9	14.6	13.5	12.6	11.3
		16	30.9	21.8	17.8	15.4	13.8	12.6	11.7	10.9	9.8
		24	25.2	17.8	14.6	12.6	11.3	10.3	9.5	8.9	7.6
	Deflection (L/360)	12	19.4	15.4	13.5	12.2	11.3	10.7	10.1	9.7	9.0
		16	17.6	14.0	12.2	11.1	10.3	9.7	9.2	8.8	8.2
		24	15.4	12.2	10.7	9.7	9.0	8.5	8.1	7.7	7.1
	Deflection (L/600)	12	16.4	13.0	11.3	10.3	9.6	9.0	8.6	8.2	7.6
		16	14.9	11.8	10.3	9.4	8.7	8.2	7.8	7.4	6.9
		24	13.0	10.3	9.0	8.2	7.6	7.1	6.8	6.5	6.0
	Deflection (L/720)	12	15.4	12.2	10.7	9.7	9.0	8.5	8.1	7.7	7.1
		16	14.0	11.1	9.7	8.8	8.2	7.7	7.3	7.0	6.5
		24	12.2	9.7	8.5	7.7	7.1	6.7	6.4	6.1	5.7

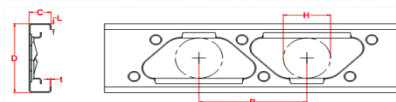
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



3-5/8" DeltaStud 2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
362D200-54	Strength Only	12	47.2	33.4	27.3	23.6	21.1	19.3	17.9	16.7	14.9
		16	40.9	28.9	23.6	20.5	18.3	16.7	15.5	14.5	12.9
		24	33.4	23.6	19.3	16.7	14.9	13.6	12.6	11.8	10.6
	Deflection (L/360)	12	20.8	16.5	14.4	13.1	12.2	11.4	10.9	10.4	9.7
		16	18.9	15.0	13.1	11.9	11.1	10.4	9.9	9.5	8.8
		24	16.5	13.1	11.4	10.4	9.7	9.1	8.6	8.3	7.7
	Deflection (L/600)	12	17.5	13.9	12.2	11.1	10.3	9.7	9.2	8.8	8.1
		16	15.9	12.7	11.1	10.0	9.3	8.8	8.3	8.0	7.4
		24	13.9	11.1	9.7	8.8	8.1	7.7	7.3	7.0	6.5
	Deflection (L/720)	12	16.5	13.1	11.4	10.4	9.7	9.1	8.6	8.3	7.7
		16	15.0	11.9	10.4	9.5	8.8	8.3	7.8	7.5	7.0
		24	13.1	10.4	9.1	8.3	7.7	7.2	6.9	6.6	6.1
362D200-68	Strength Only	12	56.6	40.1	32.7	28.3	25.3	23.1	21.4	20.0	17.9
		16	49.1	34.7	28.3	24.5	21.9	20.0	18.5	17.3	15.5
		24	40.1	28.3	23.1	20.0	17.9	16.4	15.1	14.2	12.7
	Deflection (L/360)	12	22.3	17.7	15.5	14.0	13.0	12.3	11.6	11.1	10.3
		16	20.2	16.1	14.0	12.8	11.8	11.1	10.6	10.1	9.4
		24	17.7	14.0	12.3	11.1	10.3	9.7	9.2	8.8	8.2
	Deflection (L/600)	12	18.8	14.9	13.0	11.8	11.0	10.3	9.8	9.4	8.7
		16	17.1	13.6	11.8	10.8	10.0	9.4	8.9	8.5	7.9
		24	14.9	11.8	10.3	9.4	8.7	8.2	7.8	7.5	6.9
	Deflection (L/720)	12	17.7	14.0	12.3	11.1	10.3	9.7	9.2	8.8	8.2
		16	16.1	12.8	11.1	10.1	9.4	8.8	8.4	8.0	7.5
		24	14.0	11.1	9.7	8.8	8.2	7.7	7.3	7.0	6.5

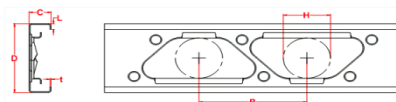
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



4" DeltaStud 2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
400D200-33	Strength Only	12	32.2	22.7	18.6	16.1	14.4	13.1	12.2	11.2	9.0
		16	27.8	19.7	16.1	13.9	12.5	11.2	9.6	8.4	6.7
		24	22.7	16.1	13.1	11.2	9.0	7.5	6.4	5.6	4.5
	Deflection (L/360)	12	19.2	15.3	13.3	12.1	11.2	10.6	10.0	9.6	8.9
		16	17.5	13.9	12.1	11.0	10.2	9.6	9.1	8.7	8.1
		24	15.3	12.1	10.6	9.6	8.9	8.4	8.0	7.6	7.1
	Deflection (L/600)	12	16.2	12.9	11.2	10.2	9.5	8.9	8.5	8.1	7.5
		16	14.7	11.7	10.2	9.3	8.6	8.1	7.7	7.4	6.8
		24	12.9	10.2	8.9	8.1	7.5	7.1	6.7	6.4	6.0
	Deflection (L/720)	12	15.3	12.1	10.6	9.6	8.9	8.4	8.0	7.6	7.1
		16	13.9	11.0	9.6	8.7	8.1	7.6	7.2	6.9	6.4
		24	12.1	9.6	8.4	7.6	7.1	6.7	6.3	6.1	5.6
400D200-43	Strength Only	12	38.0	26.9	21.9	19.0	17.0	15.5	14.4	13.4	12.0
		16	32.9	23.3	19.0	16.5	14.7	13.4	12.4	11.6	10.4
		24	26.9	19.0	15.5	13.4	12.0	11.0	10.2	9.4	7.5
	Deflection (L/360)	12	20.9	16.6	14.5	13.2	12.2	11.5	10.9	10.5	9.7
		16	19.0	15.1	13.2	12.0	11.1	10.5	9.9	9.5	8.8
		24	16.6	13.2	11.5	10.5	9.7	9.1	8.7	8.3	7.7
	Deflection (L/600)	12	17.6	14.0	12.2	11.1	10.3	9.7	9.2	8.8	8.2
		16	16.0	12.7	11.1	10.1	9.4	8.8	8.4	8.0	7.4
		24	14.0	11.1	9.7	8.8	8.2	7.7	7.3	7.0	6.5
	Deflection (L/720)	12	16.6	13.2	11.5	10.5	9.7	9.1	8.7	8.3	7.7
		16	15.1	12.0	10.5	9.5	8.8	8.3	7.9	7.5	7.0
		24	13.2	10.5	9.1	8.3	7.7	7.3	6.9	6.6	6.1

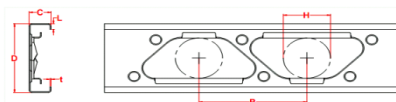
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



4" DeltaStud 2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
400D200-54	Strength Only	12	50.4	35.6	29.1	25.2	22.5	20.6	19.0	17.8	15.9
		16	43.6	30.8	25.2	21.8	19.5	17.8	16.5	15.4	13.8
		24	35.6	25.2	20.6	17.8	15.9	14.5	13.5	12.6	11.3
	Deflection (L/360)	12	22.4	17.8	15.6	14.1	13.1	12.3	11.7	11.2	10.4
		16	20.4	16.2	14.1	12.8	11.9	11.2	10.7	10.2	9.5
		24	17.8	14.1	12.3	11.2	10.4	9.8	9.3	8.9	8.3
	Deflection (L/600)	12	18.9	15.0	13.1	11.9	11.1	10.4	9.9	9.5	8.8
		16	17.2	13.6	11.9	10.8	10.1	9.5	9.0	8.6	8.0
		24	15.0	11.9	10.4	9.5	8.8	8.3	7.9	7.5	7.0
	Deflection (L/720)	12	17.8	14.1	12.3	11.2	10.4	9.8	9.3	8.9	8.3
		16	16.2	12.8	11.2	10.2	9.5	8.9	8.5	8.1	7.5
		24	14.1	11.2	9.8	8.9	8.3	7.8	7.4	7.1	6.6
400D200-68	Strength Only	12	60.4	42.7	34.9	30.2	27.0	24.7	22.8	21.4	19.1
		16	52.3	37.0	30.2	26.2	23.4	21.4	19.8	18.5	16.5
		24	42.7	30.2	24.7	21.4	19.1	17.4	16.1	15.1	13.5
	Deflection (L/360)	12	24.0	19.1	16.7	15.1	14.1	13.2	12.6	12.0	11.2
		16	21.8	17.3	15.1	13.8	12.8	12.0	11.4	10.9	10.1
		24	19.1	15.1	13.2	12.0	11.2	10.5	10.0	9.5	8.9
	Deflection (L/600)	12	20.3	16.1	14.1	12.8	11.9	11.2	10.6	10.1	9.4
		16	18.4	14.6	12.8	11.6	10.8	10.1	9.6	9.2	8.6
		24	16.1	12.8	11.2	10.1	9.4	8.9	8.4	8.0	7.5
	Deflection (L/720)	12	19.1	15.1	13.2	12.0	11.2	10.5	10.0	9.5	8.9
		16	17.3	13.8	12.0	10.9	10.1	9.5	9.1	8.7	8.0
		24	15.1	12.0	10.5	9.5	8.9	8.3	7.9	7.6	7.0

- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.

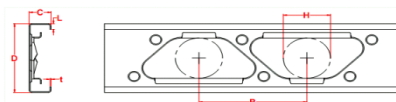


6" DeltaStud 2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
600D200-33	Strength Only	12	42.0	29.7	24.2	21.0	16.9	14.1	12.1	10.6	8.4
		16	36.3	25.7	21.0	15.8	12.7	10.6	9.0	7.9	6.3
		24	29.7	21.0	14.1	10.6	8.4	7.0	6.0	5.3	4.2
	Deflection (L/360)	12	26.3	20.9	18.2	16.6	15.4	14.5	13.7	13.1	12.2
		16	23.9	18.9	16.6	15.0	14.0	13.1	12.5	11.9	11.1
		24	20.9	16.6	14.5	13.1	12.2	11.5	10.9	10.4	9.6
	Deflection (L/600)	12	22.2	17.6	15.4	14.0	13.0	12.2	11.6	11.1	10.3
		16	20.1	16.0	14.0	12.7	11.8	11.1	10.5	10.1	9.3
		24	17.6	14.0	12.2	11.1	10.3	9.7	9.2	8.8	8.2
	Deflection (L/720)	12	20.9	16.6	14.5	13.1	12.2	11.5	10.9	10.4	9.7
		16	18.9	15.0	13.1	11.9	11.1	10.4	9.9	9.5	8.8
		24	16.6	13.1	11.5	10.4	9.7	9.1	8.7	8.3	7.7
600D200-43	Strength Only	12	49.7	35.2	28.7	24.9	22.2	20.3	18.8	17.6	14.3
		16	43.1	30.4	24.9	21.5	19.3	17.6	15.3	13.4	10.7
		24	35.2	24.9	20.3	17.6	14.3	11.9	10.2	8.9	7.1
	Deflection (L/360)	12	28.6	22.7	19.8	18.0	16.7	15.8	15.0	14.3	13.3
		16	26.0	20.6	18.0	16.4	15.2	14.3	13.6	13.0	12.1
		24	22.7	18.0	15.8	14.3	13.3	12.5	11.9	11.4	10.5
	Deflection (L/600)	12	24.1	19.2	16.7	15.2	14.1	13.3	12.6	12.1	11.2
		16	21.9	17.4	15.2	13.8	12.8	12.1	11.5	11.0	10.2
		24	19.2	15.2	13.3	12.1	11.2	10.5	10.0	9.6	8.9
	Deflection (L/720)	12	22.7	18.0	15.8	14.3	13.3	12.5	11.9	11.4	10.5
		16	20.6	16.4	14.3	13.0	12.1	11.4	10.8	10.3	9.6
		24	18.0	14.3	12.5	11.4	10.5	9.9	9.4	9.0	8.4

- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.

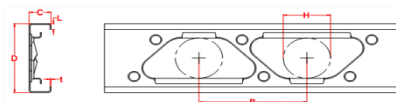
Maximum Single Span Height for Wind Bearing DeltaStud in feet ^{1,2}



6" DeltaStud 2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
600D200-54	Strength Only	12	66.1	46.7	38.1	33.0	29.5	27.0	25.0	23.4	20.9
		16	57.2	40.5	33.0	28.6	25.6	23.4	21.6	20.2	18.1
		24	46.7	33.0	27.0	23.4	20.9	19.1	17.7	16.5	14.8
	Deflection (L/360)	12	30.7	24.4	21.3	19.4	18.0	16.9	16.1	15.4	14.3
		16	27.9	22.2	19.4	17.6	16.3	15.4	14.6	14.0	13.0
		24	24.4	19.4	16.9	15.4	14.3	13.4	12.8	12.2	11.3
	Deflection (L/600)	12	25.9	20.6	18.0	16.3	15.2	14.3	13.5	13.0	12.0
		16	23.5	18.7	16.3	14.8	13.8	13.0	12.3	11.8	10.9
		24	20.6	16.3	14.3	13.0	12.0	11.3	10.8	10.3	9.5
	Deflection (L/720)	12	24.4	19.4	16.9	15.4	14.3	13.4	12.8	12.2	11.3
		16	22.2	17.6	15.4	14.0	13.0	12.2	11.6	11.1	10.3
		24	19.4	15.4	13.4	12.2	11.3	10.7	10.1	9.7	9.0
600D200-68	Strength Only	12	79.2	56.0	45.7	39.6	35.4	32.3	29.9	28.0	25.0
		16	68.6	48.5	39.6	34.3	30.7	28.0	25.9	24.2	21.7
		24	56.0	39.6	32.3	28.0	25.0	22.9	21.2	19.8	17.7
	Deflection (L/360)	12	33.0	26.2	22.9	20.8	19.3	18.1	17.2	16.5	15.3
		16	30.0	23.8	20.8	18.9	17.5	16.5	15.7	15.0	13.9
		24	26.2	20.8	18.1	16.5	15.3	14.4	13.7	13.1	12.1
	Deflection (L/600)	12	27.8	22.1	19.3	17.5	16.3	15.3	14.5	13.9	12.9
		16	25.3	20.1	17.5	15.9	14.8	13.9	13.2	12.6	11.7
		24	22.1	17.5	15.3	13.9	12.9	12.1	11.5	11.0	10.2
	Deflection (L/720)	12	26.2	20.8	18.1	16.5	15.3	14.4	13.7	13.1	12.1
		16	23.8	18.9	16.5	15.0	13.9	13.1	12.4	11.9	11.0
		24	20.8	16.5	14.4	13.1	12.1	11.4	10.9	10.4	9.6

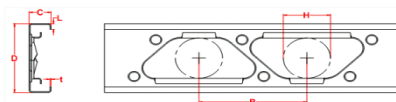
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



8" DeltaStud 2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
800D200-33 ⁷	Strength Only	12	48.1	34.0	27.7	24.0	21.5	19.6	18.2	17.0	15.2
		16	41.6	29.4	24.0	20.8	18.6	17.0	15.7	14.7	13.2
		24	34.0	24.0	19.6	17.0	15.2	13.9	12.8	12.0	10.7
	Deflection (L/360)	12	33.0	26.2	22.9	20.8	19.3	18.1	17.2	16.5	15.3
		16	29.9	23.8	20.8	18.9	17.5	16.5	15.6	14.9	13.8
		24	26.2	20.8	18.1	16.5	15.3	14.4	13.6	13.0	12.0
	Deflection (L/600)	12	27.8	22.1	19.3	17.5	16.3	15.3	14.5	13.9	12.9
		16	25.3	20.0	17.5	15.9	14.8	13.9	13.2	12.6	11.7
		24	22.1	17.5	15.3	13.9	12.9	12.1	11.5	11.0	10.2
	Deflection (L/720)	12	26.2	20.8	18.1	16.5	15.3	14.4	13.7	13.1	12.1
		16	23.8	18.9	16.5	15.0	13.9	13.1	12.4	11.9	11.0
		24	20.8	16.5	14.4	13.1	12.1	11.4	10.9	10.4	9.6
800D200-43	Strength Only	12	60.5	42.8	30.9	23.1	18.5	15.4	13.2	11.6	9.3
		16	52.4	34.7	23.1	17.4	13.9	11.6	9.9	8.7	6.9
		24	42.8	23.1	15.4	11.6	9.3	7.7	6.6	5.8	4.6
	Deflection (L/360)	12	35.9	28.5	24.9	22.6	21.0	19.8	18.8	18.0	16.7
		16	32.6	25.9	22.6	20.6	19.1	18.0	17.1	16.3	15.1
		24	28.5	22.6	19.8	18.0	16.7	15.7	14.9	14.3	13.2
	Deflection (L/600)	12	30.3	24.0	21.0	19.1	17.7	16.7	15.8	15.1	14.1
		16	27.5	21.8	19.1	17.3	16.1	15.1	14.4	13.8	12.8
		24	24.0	19.1	16.7	15.1	14.1	13.2	12.6	12.0	11.2
	Deflection (L/720)	12	28.5	22.6	19.8	18.0	16.7	15.7	14.9	14.3	13.2
		16	25.9	20.6	18.0	16.3	15.1	14.3	13.5	13.0	12.0
		24	22.6	18.0	15.7	14.3	13.2	12.5	11.8	11.3	10.5

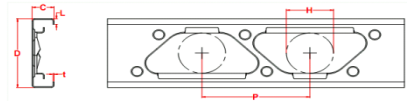
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.
 - 7 - Web stiffeners required.



8" DeltaStud 2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
800D200-54	Strength Only	12	80.3	56.8	46.3	40.1	35.9	32.8	30.3	26.9	21.5
		16	69.5	49.1	40.1	34.8	31.1	26.9	23.1	20.2	16.2
		24	56.8	40.1	32.8	26.9	21.5	18.0	15.4	13.5	10.8
	Deflection (L/360)	12	38.6	30.6	26.8	24.3	22.6	21.2	20.2	19.3	17.9
		16	35.1	27.8	24.3	22.1	20.5	19.3	18.3	17.5	16.3
		24	30.6	24.3	21.2	19.3	17.9	16.9	16.0	15.3	14.2
	Deflection (L/600)	12	32.5	25.8	22.6	20.5	19.0	17.9	17.0	16.3	15.1
		16	29.6	23.5	20.5	18.6	17.3	16.3	15.5	14.8	13.7
		24	25.8	20.5	17.9	16.3	15.1	14.2	13.5	12.9	12.0
	Deflection (L/720)	12	30.6	24.3	21.2	19.3	17.9	16.9	16.0	15.3	14.2
		16	27.8	22.1	19.3	17.5	16.3	15.3	14.5	13.9	12.9
		24	24.3	19.3	16.9	15.3	14.2	13.4	12.7	12.2	11.3
800D200-68	Strength Only	12	96.7	68.4	55.8	48.3	43.2	39.5	36.5	34.2	30.6
		16	83.7	59.2	48.3	41.9	37.4	34.2	31.6	29.6	24.8
		24	68.4	48.3	39.5	34.2	30.6	27.6	23.6	20.7	16.5
	Deflection (L/360)	12	41.4	32.9	28.7	26.1	24.2	22.8	21.7	20.7	19.2
		16	37.7	29.9	26.1	23.7	22.0	20.7	19.7	18.8	17.5
		24	32.9	26.1	22.8	20.7	19.2	18.1	17.2	16.4	15.3
	Deflection (L/600)	12	35.0	27.7	24.2	22.0	20.4	19.2	18.3	17.5	16.2
		16	31.8	25.2	22.0	20.0	18.6	17.5	16.6	15.9	14.7
		24	27.7	22.0	19.2	17.5	16.2	15.3	14.5	13.9	12.9
	Deflection (L/720)	12	32.9	26.1	22.8	20.7	19.2	18.1	17.2	16.4	15.3
		16	29.9	23.7	20.7	18.8	17.5	16.4	15.6	14.9	13.9
		24	26.1	20.7	18.1	16.4	15.3	14.4	13.6	13.1	12.1

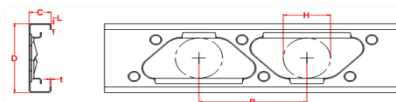
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



3-5/8" DeltaStud 2-1/2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
362D250-33	Strength Only	12	31.3	22.1	18.1	15.6	14.0	12.8	11.8	11.1	9.1
		16	27.1	19.1	15.6	13.5	12.1	11.1	9.7	8.5	6.8
		24	22.1	15.6	12.8	11.1	9.1	7.6	6.5	5.7	4.5
	Deflection (L/360)	12	18.8	14.9	13.0	11.8	11.0	10.3	9.8	9.4	8.7
		16	17.1	13.6	11.8	10.8	10.0	9.4	8.9	8.5	7.8
		24	14.9	11.8	10.3	9.4	8.7	8.1	7.7	7.4	6.8
	Deflection (L/600)	12	15.9	12.6	11.0	10.0	9.3	8.7	8.3	7.9	7.4
		16	14.4	11.4	10.0	9.1	8.4	7.9	7.5	7.2	6.7
		24	12.6	10.0	8.7	7.9	7.4	6.9	6.6	6.3	5.8
	Deflection (L/720)	12	14.9	11.8	10.3	9.4	8.7	8.2	7.8	7.5	6.9
		16	13.6	10.8	9.4	8.5	7.9	7.5	7.1	6.8	6.3
		24	11.8	9.4	8.2	7.5	6.9	6.5	6.2	5.9	5.5
362D250-43	Strength Only	12	36.7	26.0	21.2	18.4	16.4	15.0	13.9	13.0	11.6
		16	31.8	22.5	18.4	15.9	14.2	13.0	12.0	11.2	10.1
		24	26.0	18.4	15.0	13.0	11.6	10.6	9.8	9.2	7.6
	Deflection (L/360)	12	20.5	16.2	14.2	12.9	12.0	11.3	10.7	10.2	9.5
		16	18.6	14.8	12.9	11.7	10.9	10.2	9.7	9.3	8.6
		24	16.2	12.9	11.3	10.2	9.5	8.9	8.5	8.1	7.5
	Deflection (L/600)	12	17.3	13.7	12.0	10.9	10.1	9.5	9.0	8.6	8.0
		16	15.7	12.4	10.9	9.9	9.2	8.6	8.2	7.8	7.3
		24	13.7	10.9	9.5	8.6	8.0	7.5	7.2	6.8	6.4
	Deflection (L/720)	12	16.2	12.9	11.3	10.2	9.5	8.9	8.5	8.1	7.5
		16	14.8	11.7	10.2	9.3	8.6	8.1	7.7	7.4	6.8
		24	12.9	10.2	8.9	8.1	7.5	7.1	6.7	6.4	6.0

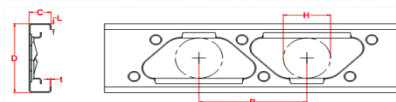
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



3-5/8" DeltaStud 2-1/2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
362D250-54	Strength Only	12	48.5	34.3	28.0	24.3	21.7	19.8	18.3	17.2	15.4
		16	42.0	29.7	24.3	21.0	18.8	17.2	15.9	14.9	13.3
		24	34.3	24.3	19.8	17.2	15.4	14.0	13.0	12.1	10.9
	Deflection (L/360)	12	22.0	17.4	15.2	13.8	12.8	12.1	11.5	11.0	10.2
		16	19.9	15.8	13.8	12.6	11.7	11.0	10.4	10.0	9.3
		24	17.4	13.8	12.1	11.0	10.2	9.6	9.1	8.7	8.1
	Deflection (L/600)	12	18.5	14.7	12.8	11.7	10.8	10.2	9.7	9.3	8.6
		16	16.8	13.4	11.7	10.6	9.8	9.3	8.8	8.4	7.8
		24	14.7	11.7	10.2	9.3	8.6	8.1	7.7	7.3	6.8
	Deflection (L/720)	12	17.4	13.8	12.1	11.0	10.2	9.6	9.1	8.7	8.1
		16	15.8	12.6	11.0	10.0	9.3	8.7	8.3	7.9	7.3
		24	13.8	11.0	9.6	8.7	8.1	7.6	7.2	6.9	6.4
362D250-68	Strength Only	12	54.2	38.3	31.3	27.1	24.3	22.1	20.5	19.2	17.1
		16	47.0	33.2	27.1	23.5	21.0	19.2	17.8	16.6	14.9
		24	38.3	27.1	22.1	19.2	17.1	15.7	14.5	13.6	12.1
	Deflection (L/360)	12	22.3	17.7	15.5	14.0	13.0	12.3	11.6	11.1	10.3
		16	20.2	16.1	14.0	12.8	11.8	11.1	10.6	10.1	9.4
		24	17.7	14.0	12.3	11.1	10.3	9.7	9.2	8.8	8.2
	Deflection (L/600)	12	18.8	14.9	13.0	11.8	11.0	10.3	9.8	9.4	8.7
		16	17.1	13.6	11.8	10.8	10.0	9.4	8.9	8.5	7.9
		24	14.9	11.8	10.3	9.4	8.7	8.2	7.8	7.5	6.9
	Deflection (L/720)	12	17.7	14.0	12.3	11.1	10.3	9.7	9.2	8.8	8.2
		16	16.1	12.8	11.1	10.1	9.4	8.8	8.4	8.0	7.5
		24	14.0	11.1	9.7	8.8	8.2	7.7	7.3	7.0	6.5

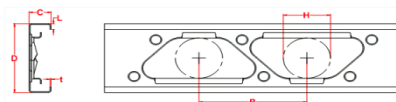
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



4" DeltaStud 2-1/2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
400D250-33	Strength Only	12	33.3	23.6	19.2	16.7	14.9	13.6	12.6	11.2	9.0
		16	28.9	20.4	16.7	14.4	12.9	11.2	9.6	8.4	6.7
		24	23.6	16.7	13.6	11.2	9.0	7.5	6.4	5.6	4.5
	Deflection (L/360)	12	20.2	16.1	14.0	12.7	11.8	11.1	10.5	10.1	9.3
		16	18.4	14.6	12.7	11.6	10.7	10.1	9.5	9.1	8.4
		24	16.1	12.7	11.1	10.1	9.3	8.7	8.3	7.9	7.3
	Deflection (L/600)	12	17.1	13.5	11.8	10.8	10.0	9.4	8.9	8.5	7.9
		16	15.5	12.3	10.8	9.8	9.1	8.5	8.1	7.8	7.2
		24	13.5	10.8	9.4	8.5	7.9	7.5	7.1	6.8	6.3
	Deflection (L/720)	12	16.1	12.7	11.1	10.1	9.4	8.8	8.4	8.0	7.5
		16	14.6	11.6	10.1	9.2	8.5	8.0	7.6	7.3	6.8
		24	12.7	10.1	8.8	8.0	7.5	7.0	6.7	6.4	5.9
400D250-43	Strength Only	12	39.1	27.7	22.6	19.6	17.5	16.0	14.8	13.8	12.4
		16	33.9	23.9	19.6	16.9	15.1	13.8	12.8	12.0	10.7
		24	27.7	19.6	16.0	13.8	12.4	11.3	10.5	9.4	7.5
	Deflection (L/360)	12	22.0	17.5	15.3	13.9	12.9	12.1	11.5	11.0	10.2
		16	20.0	15.9	13.9	12.6	11.7	11.0	10.5	10.0	9.3
		24	17.5	13.9	12.1	11.0	10.2	9.6	9.1	8.7	8.1
	Deflection (L/600)	12	18.6	14.8	12.9	11.7	10.9	10.2	9.7	9.3	8.6
		16	16.9	13.4	11.7	10.6	9.9	9.3	8.8	8.4	7.8
		24	14.8	11.7	10.2	9.3	8.6	8.1	7.7	7.4	6.8
	Deflection (L/720)	12	17.5	13.9	12.1	11.0	10.2	9.6	9.1	8.7	8.1
		16	15.9	12.6	11.0	10.0	9.3	8.7	8.3	7.9	7.4
		24	13.9	11.0	9.6	8.7	8.1	7.6	7.3	6.9	6.4

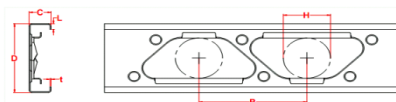
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



4" DeltaStud 2-1/2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
400D250-54	Strength Only	12	51.7	36.6	29.9	25.9	23.1	21.1	19.6	18.3	16.4
		16	44.8	31.7	25.9	22.4	20.0	18.3	16.9	15.8	14.2
		24	36.6	25.9	21.1	18.3	16.4	14.9	13.8	12.9	11.6
	Deflection (L/360)	12	23.6	18.8	16.4	14.9	13.8	13.0	12.4	11.8	11.0
		16	21.5	17.1	14.9	13.5	12.6	11.8	11.2	10.7	10.0
		24	18.8	14.9	13.0	11.8	11.0	10.3	9.8	9.4	8.7
	Deflection (L/600)	12	19.9	15.8	13.8	12.6	11.7	11.0	10.4	10.0	9.3
		16	18.1	14.4	12.6	11.4	10.6	10.0	9.5	9.1	8.4
		24	15.8	12.6	11.0	10.0	9.3	8.7	8.3	7.9	7.3
	Deflection (L/720)	12	18.8	14.9	13.0	11.8	11.0	10.3	9.8	9.4	8.7
		16	17.1	13.5	11.8	10.7	10.0	9.4	8.9	8.5	7.9
		24	14.9	11.8	10.3	9.4	8.7	8.2	7.8	7.4	6.9
400D250-68	Strength Only	12	59.1	41.8	34.1	29.6	26.4	24.1	22.3	20.9	18.7
		16	51.2	36.2	29.6	25.6	22.9	20.9	19.4	18.1	16.2
		24	41.8	29.6	24.1	20.9	18.7	17.1	15.8	14.8	13.2
	Deflection (L/360)	12	25.4	20.1	17.6	16.0	14.8	14.0	13.3	12.7	11.8
		16	23.0	18.3	16.0	14.5	13.5	12.7	12.0	11.5	10.7
		24	20.1	16.0	14.0	12.7	11.8	11.1	10.5	10.1	9.3
	Deflection (L/600)	12	21.4	17.0	14.8	13.5	12.5	11.8	11.2	10.7	9.9
		16	19.4	15.4	13.5	12.2	11.4	10.7	10.2	9.7	9.0
		24	17.0	13.5	11.8	10.7	9.9	9.3	8.9	8.5	7.9
	Deflection (L/720)	12	20.1	16.0	14.0	12.7	11.8	11.1	10.5	10.1	9.3
		16	18.3	14.5	12.7	11.5	10.7	10.1	9.6	9.1	8.5
		24	16.0	12.7	11.1	10.1	9.3	8.8	8.4	8.0	7.4

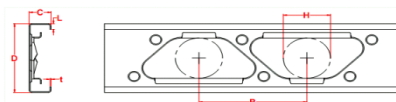
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



6" DeltaStud 2-1/2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
600D250-33	Strength Only	12	42.9	30.3	24.8	21.1	16.9	14.1	12.1	10.6	8.4
		16	37.1	26.3	21.1	15.8	12.7	10.6	9.0	7.9	6.3
		24	30.3	21.1	14.1	10.6	8.4	7.0	6.0	5.3	4.2
	Deflection (L/360)	12	27.5	21.8	19.1	17.3	16.0	15.1	14.3	13.6	12.6
		16	25.0	19.8	17.3	15.7	14.5	13.6	12.9	12.4	11.4
		24	21.8	17.3	15.1	13.6	12.6	11.9	11.3	10.7	10.0
	Deflection (L/600)	12	23.2	18.4	16.1	14.6	13.6	12.8	12.1	11.6	10.8
		16	21.1	16.7	14.6	13.3	12.3	11.6	11.0	10.5	9.8
		24	18.4	14.6	12.8	11.6	10.8	10.1	9.6	9.2	8.5
	Deflection (L/720)	12	21.8	17.3	15.1	13.8	12.8	12.0	11.4	10.9	10.1
		16	19.8	15.7	13.8	12.5	11.6	10.9	10.4	9.9	9.2
		24	17.3	13.8	12.0	10.9	10.1	9.5	9.1	8.7	8.0
600D250-43	Strength Only	12	51.0	36.1	29.4	25.5	22.8	20.8	19.3	17.8	14.3
		16	44.2	31.2	25.5	22.1	19.8	17.8	15.3	13.4	10.7
		24	36.1	25.5	20.8	17.8	14.3	11.9	10.2	8.9	7.1
	Deflection (L/360)	12	30.0	23.8	20.8	18.9	17.5	16.5	15.7	15.0	13.9
		16	27.2	21.6	18.9	17.2	15.9	15.0	14.2	13.6	12.6
		24	23.8	18.9	16.5	15.0	13.9	13.1	12.4	11.9	11.0
	Deflection (L/600)	12	25.3	20.1	17.5	15.9	14.8	13.9	13.2	12.6	11.7
		16	23.0	18.2	15.9	14.5	13.4	12.6	12.0	11.5	10.7
		24	20.1	15.9	13.9	12.6	11.7	11.0	10.5	10.0	9.3
	Deflection (L/720)	12	23.8	18.9	16.5	15.0	13.9	13.1	12.4	11.9	11.0
		16	21.6	17.2	15.0	13.6	12.6	11.9	11.3	10.8	10.0
		24	18.9	15.0	13.1	11.9	11.0	10.4	9.9	9.4	8.8

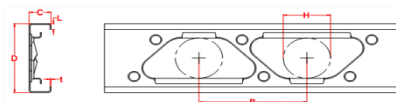
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



6" DeltaStud 2-1/2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
600D250-54	Strength Only	12	67.7	47.9	39.1	33.9	30.3	27.7	25.6	24.0	21.4
		16	58.7	41.5	33.9	29.3	26.2	24.0	22.2	20.7	18.6
		24	47.9	33.9	27.7	24.0	21.4	19.6	18.1	16.9	15.1
	Deflection (L/360)	12	32.2	25.6	22.3	20.3	18.8	17.7	16.8	16.1	14.9
		16	29.3	23.2	20.3	18.4	17.1	16.1	15.3	14.6	13.6
		24	25.6	20.3	17.7	16.1	14.9	14.1	13.4	12.8	11.9
	Deflection (L/600)	12	27.2	21.6	18.8	17.1	15.9	14.9	14.2	13.6	12.6
		16	24.7	19.6	17.1	15.5	14.4	13.6	12.9	12.3	11.5
		24	21.6	17.1	14.9	13.6	12.6	11.9	11.3	10.8	10.0
	Deflection (L/720)	12	25.6	20.3	17.7	16.1	14.9	14.1	13.4	12.8	11.9
		16	23.2	18.4	16.1	14.6	13.6	12.8	12.1	11.6	10.8
		24	20.3	16.1	14.1	12.8	11.9	11.2	10.6	10.1	9.4
600D250-68	Strength Only	12	77.2	54.6	44.6	38.6	34.5	31.5	29.2	27.3	24.4
		16	66.8	47.3	38.6	33.4	29.9	27.3	25.3	23.6	21.1
		24	54.6	38.6	31.5	27.3	24.4	22.3	20.6	19.3	17.3
	Deflection (L/360)	12	34.6	27.4	24.0	21.8	20.2	19.0	18.1	17.3	16.0
		16	31.4	24.9	21.8	19.8	18.4	17.3	16.4	15.7	14.6
		24	27.4	21.8	19.0	17.3	16.0	15.1	14.3	13.7	12.7
	Deflection (L/600)	12	29.2	23.1	20.2	18.4	17.1	16.0	15.2	14.6	13.5
		16	26.5	21.0	18.4	16.7	15.5	14.6	13.8	13.2	12.3
		24	23.1	18.4	16.0	14.6	13.5	12.7	12.1	11.6	10.7
	Deflection (L/720)	12	27.4	21.8	19.0	17.3	16.0	15.1	14.3	13.7	12.7
		16	24.9	19.8	17.3	15.7	14.6	13.7	13.0	12.5	11.6
		24	21.8	17.3	15.1	13.7	12.7	12.0	11.4	10.9	10.1

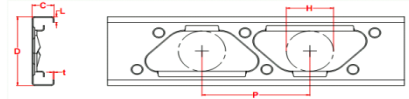
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



8" DeltaStud 2-1/2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
800D250-33 ⁷	Strength Only	12	49.3	34.9	28.5	24.6	22.0	20.1	18.6	17.4	15.6
		16	42.7	30.2	24.6	21.3	19.1	17.4	16.1	15.1	13.5
		24	34.9	24.6	20.1	17.4	15.6	14.2	13.2	12.3	11.0
	Deflection (L/360)	12	34.4	27.3	23.8	21.6	20.0	18.8	17.8	17.0	15.8
		16	31.2	24.8	21.6	19.6	18.1	17.0	16.2	15.4	14.2
		24	27.3	21.6	18.8	17.0	15.8	14.8	14.0	13.3	12.3
	Deflection (L/600)	12	29.0	23.0	20.1	18.3	17.0	16.0	15.2	14.5	13.4
		16	26.3	20.9	18.3	16.6	15.4	14.5	13.8	13.1	12.2
		24	23.0	18.3	16.0	14.5	13.4	12.6	12.0	11.4	10.6
	Deflection (L/720)	12	27.3	21.7	18.9	17.2	16.0	15.0	14.3	13.6	12.7
		16	24.8	19.7	17.2	15.6	14.5	13.6	13.0	12.4	11.5
		24	21.7	17.2	15.0	13.6	12.7	11.9	11.3	10.8	10.0
800D250-43	Strength Only	12	61.0	43.1	30.9	23.1	18.5	15.4	13.2	11.6	9.3
		16	52.8	34.7	23.1	17.4	13.9	11.6	9.9	8.7	6.9
		24	43.1	23.1	15.4	11.6	9.3	7.7	6.6	5.8	4.6
	Deflection (L/360)	12	37.5	29.7	26.0	23.6	21.9	20.6	19.6	18.7	17.4
		16	34.0	27.0	23.6	21.4	19.9	18.7	17.8	17.0	15.8
		24	29.7	23.6	20.6	18.7	17.4	16.4	15.5	14.9	13.8
	Deflection (L/600)	12	31.6	25.1	21.9	19.9	18.5	17.4	16.5	15.8	14.7
		16	28.7	22.8	19.9	18.1	16.8	15.8	15.0	14.4	13.3
		24	25.1	19.9	17.4	15.8	14.7	13.8	13.1	12.5	11.6
	Deflection (L/720)	12	29.7	23.6	20.6	18.7	17.4	16.4	15.5	14.9	13.8
		16	27.0	21.4	18.7	17.0	15.8	14.9	14.1	13.5	12.5
		24	23.6	18.7	16.4	14.9	13.8	13.0	12.3	11.8	11.0

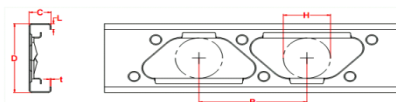
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.
 - 7 - Web stiffeners required.



8" DeltaStud 2-1/2" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
800D250-54	Strength Only	12	80.9	57.2	46.7	40.5	36.2	33.0	30.6	26.9	21.5
		16	70.1	49.6	40.5	35.0	31.3	26.9	23.1	20.2	16.2
		24	57.2	40.5	33.0	26.9	21.5	18.0	15.4	13.5	10.8
	Deflection (L/360)	12	40.3	32.0	27.9	25.4	23.5	22.2	21.0	20.1	18.7
		16	36.6	29.0	25.4	23.0	21.4	20.1	19.1	18.3	17.0
		24	32.0	25.4	22.2	20.1	18.7	17.6	16.7	16.0	14.8
	Deflection (L/600)	12	34.0	27.0	23.5	21.4	19.9	18.7	17.8	17.0	15.8
		16	30.9	24.5	21.4	19.4	18.0	17.0	16.1	15.4	14.3
		24	27.0	21.4	18.7	17.0	15.8	14.8	14.1	13.5	12.5
	Deflection (L/720)	12	32.0	25.4	22.2	20.1	18.7	17.6	16.7	16.0	14.8
		16	29.0	23.0	20.1	18.3	17.0	16.0	15.2	14.5	13.5
		24	25.4	20.1	17.6	16.0	14.8	14.0	13.3	12.7	11.8
800D250-68	Strength Only	12	94.0	66.5	54.3	47.0	42.1	38.4	35.5	33.2	29.7
		16	81.4	57.6	47.0	40.7	36.4	33.2	30.8	28.8	24.8
		24	66.5	47.0	38.4	33.2	29.7	27.1	23.6	20.7	16.5
	Deflection (L/360)	12	43.3	34.3	30.0	27.3	25.3	23.8	22.6	21.6	20.1
		16	39.3	31.2	27.3	24.8	23.0	21.6	20.5	19.7	18.2
		24	34.3	27.3	23.8	21.6	20.1	18.9	17.9	17.2	15.9
	Deflection (L/600)	12	36.5	29.0	25.3	23.0	21.3	20.1	19.1	18.2	16.9
		16	33.2	26.3	23.0	20.9	19.4	18.2	17.3	16.6	15.4
		24	29.0	23.0	20.1	18.2	16.9	15.9	15.1	14.5	13.4
	Deflection (L/720)	12	34.3	27.3	23.8	21.6	20.1	18.9	17.9	17.2	15.9
		16	31.2	24.8	21.6	19.7	18.2	17.2	16.3	15.6	14.5
		24	27.3	21.6	18.9	17.2	15.9	15.0	14.2	13.6	12.6

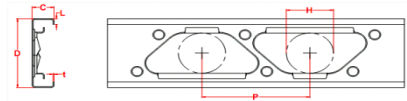
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



3-5/8" DeltaStud 3" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
362D300-33	Strength Only	12	32.0	22.6	18.5	16.0	14.3	13.1	12.1	11.3	9.1
		16	27.7	19.6	16.0	13.9	12.4	11.3	9.7	8.5	6.8
		24	22.6	16.0	13.1	11.3	9.1	7.6	6.5	5.7	4.5
	Deflection (L/360)	12	19.7	15.6	13.6	12.3	11.4	10.7	10.1	9.7	8.9
		16	17.9	14.1	12.3	11.1	10.3	9.7	9.2	8.7	8.1
		24	15.6	12.3	10.7	9.7	8.9	8.4	7.9	7.6	7.0
	Deflection (L/600)	12	16.6	13.2	11.5	10.5	9.7	9.1	8.7	8.3	7.6
		16	15.1	12.0	10.5	9.5	8.8	8.3	7.8	7.5	6.9
		24	13.2	10.5	9.1	8.3	7.6	7.2	6.8	6.5	6.0
	Deflection (L/720)	12	15.6	12.4	10.8	9.8	9.1	8.6	8.2	7.8	7.2
		16	14.2	11.3	9.8	8.9	8.3	7.8	7.4	7.1	6.5
		24	12.4	9.8	8.6	7.8	7.2	6.8	6.4	6.1	5.7
362D300-43	Strength Only	12	37.3	26.4	21.5	18.6	16.7	15.2	14.1	13.2	11.8
		16	32.3	22.8	18.6	16.1	14.4	13.2	12.2	11.4	10.2
		24	26.4	18.6	15.2	13.2	11.8	10.8	10.0	9.3	7.6
	Deflection (L/360)	12	21.4	17.0	14.9	13.5	12.5	11.8	11.2	10.7	9.9
		16	19.5	15.4	13.5	12.3	11.4	10.7	10.2	9.7	9.0
		24	17.0	13.5	11.8	10.7	9.9	9.4	8.9	8.5	7.8
	Deflection (L/600)	12	18.1	14.3	12.5	11.4	10.6	9.9	9.4	9.0	8.4
		16	16.4	13.0	11.4	10.3	9.6	9.0	8.6	8.2	7.6
		24	14.3	11.4	9.9	9.0	8.4	7.9	7.5	7.2	6.7
	Deflection (L/720)	12	17.0	13.5	11.8	10.7	9.9	9.4	8.9	8.5	7.9
		16	15.4	12.3	10.7	9.7	9.0	8.5	8.1	7.7	7.2
		24	13.5	10.7	9.4	8.5	7.9	7.4	7.1	6.7	6.3

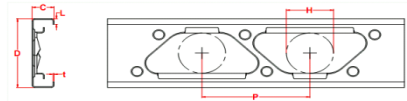
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



3-5/8" DeltaStud 3" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
362D300-54	Strength Only	12	49.4	34.9	28.5	24.7	22.1	20.2	18.7	17.5	15.6
		16	42.8	30.3	24.7	21.4	19.1	17.5	16.2	15.1	13.5
		24	34.9	24.7	20.2	17.5	15.6	14.3	13.2	12.4	11.1
	Deflection (L/360)	12	23.0	18.2	15.9	14.5	13.4	12.7	12.0	11.5	10.7
		16	20.9	16.6	14.5	13.2	12.2	11.5	10.9	10.4	9.7
		24	18.2	14.5	12.7	11.5	10.7	10.0	9.5	9.1	8.5
	Deflection (L/600)	12	19.4	15.4	13.4	12.2	11.3	10.7	10.1	9.7	9.0
		16	17.6	14.0	12.2	11.1	10.3	9.7	9.2	8.8	8.2
		24	15.4	12.2	10.7	9.7	9.0	8.5	8.0	7.7	7.1
	Deflection (L/720)	12	18.2	14.5	12.7	11.5	10.7	10.0	9.5	9.1	8.5
		16	16.6	13.2	11.5	10.4	9.7	9.1	8.7	8.3	7.7
		24	14.5	11.5	10.0	9.1	8.5	8.0	7.6	7.2	6.7
362D300-68	Strength Only	12	56.7	40.1	32.7	28.4	25.4	23.2	21.4	20.1	17.9
		16	49.1	34.7	28.4	24.6	22.0	20.1	18.6	17.4	15.5
		24	40.1	28.4	23.2	20.1	17.9	16.4	15.2	14.2	12.7
	Deflection (L/360)	12	24.7	19.6	17.1	15.5	14.4	13.6	12.9	12.3	11.4
		16	22.4	17.8	15.5	14.1	13.1	12.3	11.7	11.2	10.4
		24	19.6	15.5	13.6	12.3	11.4	10.8	10.2	9.8	9.1
	Deflection (L/600)	12	20.8	16.5	14.4	13.1	12.2	11.4	10.9	10.4	9.7
		16	18.9	15.0	13.1	11.9	11.1	10.4	9.9	9.4	8.8
		24	16.5	13.1	11.4	10.4	9.7	9.1	8.6	8.3	7.7
	Deflection (L/720)	12	19.6	15.5	13.6	12.3	11.4	10.8	10.2	9.8	9.1
		16	17.8	14.1	12.3	11.2	10.4	9.8	9.3	8.9	8.3
		24	15.5	12.3	10.8	9.8	9.1	8.6	8.1	7.8	7.2

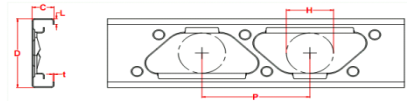
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



4" DeltaStud 3" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
400D300-33	Strength Only	12	34.1	24.1	19.7	17.0	15.2	13.9	12.8	11.2	9.0
		16	29.5	20.9	17.0	14.8	13.2	11.2	9.6	8.4	6.7
		24	24.1	17.0	13.9	11.2	9.0	7.5	6.4	5.6	4.5
	Deflection (L/360)	12	21.2	16.8	14.6	13.2	12.2	11.5	10.9	10.4	9.6
		16	19.2	15.2	13.2	12.0	11.1	10.4	9.8	9.4	8.7
		24	16.8	13.2	11.5	10.4	9.6	9.0	8.5	8.2	7.5
	Deflection (L/600)	12	17.8	14.2	12.4	11.2	10.4	9.8	9.3	8.9	8.2
		16	16.2	12.9	11.2	10.2	9.5	8.9	8.4	8.0	7.4
		24	14.2	11.2	9.8	8.9	8.2	7.7	7.3	7.0	6.5
	Deflection (L/720)	12	16.8	13.3	11.6	10.6	9.8	9.2	8.8	8.4	7.8
		16	15.3	12.1	10.6	9.6	8.9	8.4	8.0	7.6	7.0
		24	13.3	10.6	9.2	8.4	7.8	7.3	6.9	6.6	6.1
400D300-43	Strength Only	12	39.7	28.1	22.9	19.9	17.8	16.2	15.0	14.0	12.6
		16	34.4	24.3	19.9	17.2	15.4	14.0	13.0	12.2	10.9
		24	28.1	19.9	16.2	14.0	12.6	11.5	10.6	9.4	7.5
	Deflection (L/360)	12	23.0	18.3	16.0	14.5	13.5	12.7	12.0	11.5	10.7
		16	20.9	16.6	14.5	13.2	12.2	11.5	10.9	10.5	9.7
		24	18.3	14.5	12.7	11.5	10.7	10.1	9.5	9.1	8.4
	Deflection (L/600)	12	19.4	15.4	13.5	12.2	11.4	10.7	10.2	9.7	9.0
		16	17.7	14.0	12.2	11.1	10.3	9.7	9.2	8.8	8.2
		24	15.4	12.2	10.7	9.7	9.0	8.5	8.1	7.7	7.2
	Deflection (L/720)	12	18.3	14.5	12.7	11.5	10.7	10.1	9.6	9.1	8.5
		16	16.6	13.2	11.5	10.5	9.7	9.1	8.7	8.3	7.7
		24	14.5	11.5	10.1	9.1	8.5	8.0	7.6	7.3	6.7

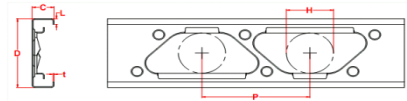
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



4" DeltaStud 3" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
400D300-54	Strength Only	12	52.7	37.2	30.4	26.3	23.6	21.5	19.9	18.6	16.7
		16	45.6	32.3	26.3	22.8	20.4	18.6	17.2	16.1	14.4
		24	37.2	26.3	21.5	18.6	16.7	15.2	14.1	13.2	11.8
	Deflection (L/360)	12	24.7	19.6	17.2	15.6	14.5	13.6	12.9	12.4	11.5
		16	22.5	17.8	15.6	14.2	13.1	12.4	11.8	11.2	10.4
		24	19.6	15.6	13.6	12.4	11.5	10.8	10.3	9.8	9.1
	Deflection (L/600)	12	20.9	16.6	14.5	13.1	12.2	11.5	10.9	10.4	9.7
		16	19.0	15.0	13.1	11.9	11.1	10.4	9.9	9.5	8.8
		24	16.6	13.1	11.5	10.4	9.7	9.1	8.7	8.3	7.7
	Deflection (L/720)	12	19.6	15.6	13.6	12.4	11.5	10.8	10.3	9.8	9.1
		16	17.8	14.2	12.4	11.2	10.4	9.8	9.3	8.9	8.3
		24	15.6	12.4	10.8	9.8	9.1	8.6	8.1	7.8	7.2
400D300-68	Strength Only	12	60.4	42.7	34.9	30.2	27.0	24.7	22.8	21.4	19.1
		16	52.3	37.0	30.2	26.2	23.4	21.4	19.8	18.5	16.5
		24	42.7	30.2	24.7	21.4	19.1	17.4	16.1	15.1	13.5
	Deflection (L/360)	12	26.5	21.1	18.4	16.7	15.5	14.6	13.9	13.3	12.3
		16	24.1	19.1	16.7	15.2	14.1	13.3	12.6	12.1	11.2
		24	21.1	16.7	14.6	13.3	12.3	11.6	11.0	10.5	9.8
	Deflection (L/600)	12	22.4	17.8	15.5	14.1	13.1	12.3	11.7	11.2	10.4
		16	20.3	16.1	14.1	12.8	11.9	11.2	10.6	10.2	9.4
		24	17.8	14.1	12.3	11.2	10.4	9.8	9.3	8.9	8.2
	Deflection (L/720)	12	21.1	16.7	14.6	13.3	12.3	11.6	11.0	10.5	9.8
		16	19.1	15.2	13.3	12.1	11.2	10.5	10.0	9.6	8.9
		24	16.7	13.3	11.6	10.5	9.8	9.2	8.7	8.4	7.8

- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.

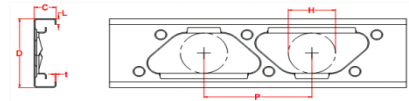


6" DeltaStud 3" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
600D300-33	Strength Only	12	43.3	30.6	25.0	21.1	16.9	14.1	12.1	10.6	8.4
		16	37.5	26.5	21.1	15.8	12.7	10.6	9.0	7.9	6.3
		24	30.6	21.1	14.1	10.6	8.4	7.0	6.0	5.3	4.2
	Deflection (L/360)	12	28.7	22.6	19.7	17.8	16.5	15.5	14.7	14.0	13.0
		16	26.0	20.5	17.8	16.1	14.9	14.0	13.3	12.7	11.8
		24	22.6	17.8	15.5	14.0	13.0	12.2	11.6	11.0	10.2
	Deflection (L/600)	12	24.2	19.2	16.8	15.2	14.1	13.2	12.5	12.0	11.1
		16	22.0	17.4	15.2	13.8	12.8	12.0	11.4	10.9	10.0
		24	19.2	15.2	13.2	12.0	11.1	10.4	9.9	9.4	8.7
	Deflection (L/720)	12	22.7	18.0	15.8	14.3	13.3	12.5	11.9	11.3	10.5
		16	20.7	16.4	14.3	13.0	12.1	11.3	10.7	10.3	9.5
		24	18.0	14.3	12.5	11.3	10.5	9.8	9.3	8.9	8.3
600D300-43	Strength Only	12	51.7	36.6	29.9	25.9	23.1	21.1	19.6	17.8	14.3
		16	44.8	31.7	25.9	22.4	20.0	17.8	15.3	13.4	10.7
		24	36.6	25.9	21.1	17.8	14.3	11.9	10.2	8.9	7.1
	Deflection (L/360)	12	31.2	24.8	21.6	19.7	18.3	17.2	16.3	15.6	14.5
		16	28.4	22.5	19.7	17.9	16.6	15.6	14.8	14.1	13.1
		24	24.8	19.7	17.2	15.6	14.5	13.6	12.9	12.3	11.4
	Deflection (L/600)	12	26.3	20.9	18.3	16.6	15.4	14.5	13.8	13.2	12.2
		16	23.9	19.0	16.6	15.1	14.0	13.2	12.5	12.0	11.1
		24	20.9	16.6	14.5	13.2	12.2	11.5	10.9	10.5	9.7
	Deflection (L/720)	12	24.8	19.7	17.2	15.6	14.5	13.6	13.0	12.4	11.5
		16	22.5	17.9	15.6	14.2	13.2	12.4	11.8	11.3	10.5
		24	19.7	15.6	13.6	12.4	11.5	10.8	10.3	9.8	9.1

- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.

Maximum Single Span Height for Wind Bearing DeltaStud in feet ^{1,2}

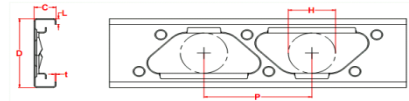


6" DeltaStud 3" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
600D300-54	Strength Only	12	68.9	48.7	39.8	34.5	30.8	28.1	26.1	24.4	21.8
		16	59.7	42.2	34.5	29.8	26.7	24.4	22.6	21.1	18.9
		24	48.7	34.5	28.1	24.4	21.8	19.9	18.4	17.2	15.4
	Deflection (L/360)	12	33.5	26.6	23.3	21.1	19.6	18.5	17.5	16.8	15.6
		16	30.5	24.2	21.1	19.2	17.8	16.8	15.9	15.2	14.1
		24	26.6	21.1	18.5	16.8	15.6	14.7	13.9	13.3	12.3
	Deflection (L/600)	12	28.3	22.5	19.6	17.8	16.5	15.6	14.8	14.1	13.1
		16	25.7	20.4	17.8	16.2	15.0	14.1	13.4	12.9	11.9
		24	22.5	17.8	15.6	14.1	13.1	12.4	11.7	11.2	10.4
	Deflection (L/720)	12	26.6	21.1	18.5	16.8	15.6	14.7	13.9	13.3	12.4
		16	24.2	19.2	16.8	15.2	14.1	13.3	12.6	12.1	11.2
		24	21.1	16.8	14.7	13.3	12.4	11.6	11.0	10.6	9.8
600D300-68	Strength Only	12	78.8	55.7	45.5	39.4	35.3	32.2	29.8	27.9	24.9
		16	68.3	48.3	39.4	34.1	30.5	27.9	25.8	24.1	21.6
		24	55.7	39.4	32.2	27.9	24.9	22.8	21.1	19.7	17.1
	Deflection (L/360)	12	36.0	28.6	25.0	22.7	21.1	19.8	18.8	18.0	16.7
		16	32.7	26.0	22.7	20.6	19.1	18.0	17.1	16.4	15.2
		24	28.6	22.7	19.8	18.0	16.7	15.7	15.0	14.3	13.3
	Deflection (L/600)	12	30.4	24.1	21.1	19.1	17.8	16.7	15.9	15.2	14.1
		16	27.6	21.9	19.1	17.4	16.1	15.2	14.4	13.8	12.8
		24	24.1	19.1	16.7	15.2	14.1	13.3	12.6	12.1	11.2
	Deflection (L/720)	12	28.6	22.7	19.8	18.0	16.7	15.7	15.0	14.3	13.3
		16	26.0	20.6	18.0	16.4	15.2	14.3	13.6	13.0	12.1
		24	22.7	18.0	15.7	14.3	13.3	12.5	11.9	11.4	10.5

- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.

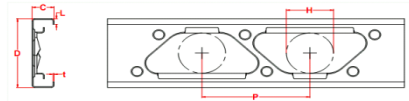
Maximum Single Span Height for Wind Bearing DeltaStud in feet ^{1,2}



8" DeltaStud 3" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
800D300-33 ⁷	Strength Only	12	50.0	27.0	28.8	25.0	22.3	20.4	18.9	17.7	15.8
		16	43.3	30.6	25.0	21.6	19.3	17.7	16.4	15.3	13.7
		24	35.3	25.0	20.4	17.7	15.8	14.4	13.4	12.5	11.2
	Deflection (L/360)	12	35.7	28.1	24.5	22.2	20.5	19.3	18.3	17.5	16.1
		16	32.3	25.5	22.2	20.1	18.6	17.5	16.6	15.8	14.5
		24	28.1	22.2	19.3	17.5	16.1	15.1	14.3	13.6	12.4
	Deflection (L/600)	12	30.1	23.9	20.9	18.9	17.5	16.4	15.6	14.9	13.8
		16	27.3	21.7	18.9	17.1	15.9	14.9	14.1	13.5	12.5
		24	23.9	18.9	16.4	14.9	13.8	13.0	12.3	11.7	10.9
	Deflection (L/720)	12	28.3	22.5	19.6	17.8	16.5	15.5	14.7	14.1	13.0
		16	25.7	20.4	17.8	16.2	15.0	14.1	13.3	12.7	11.8
		24	22.5	17.8	15.5	14.1	13.0	12.2	11.6	11.1	10.3
800D300-43	Strength Only	12	61.0	43.1	30.9	23.1	18.5	15.4	13.2	11.6	9.3
		16	52.8	34.7	23.1	17.4	13.9	11.6	9.9	8.7	6.9
		24	43.1	23.1	15.4	11.6	9.3	7.7	6.6	5.8	4.6
	Deflection (L/360)	12	38.9	30.9	27.0	24.5	22.7	21.4	20.3	19.4	18.0
		16	35.3	28.0	24.5	22.3	20.7	19.4	18.4	17.6	16.3
		24	30.9	24.5	21.4	19.4	18.0	16.9	16.0	15.3	14.1
	Deflection (L/600)	12	32.8	26.0	22.7	20.7	19.2	18.1	17.1	16.4	15.2
		16	29.8	23.7	20.7	18.8	17.4	16.4	15.6	14.9	13.8
		24	26.0	20.7	18.1	16.4	15.2	14.3	13.6	13.0	12.1
	Deflection (L/720)	12	30.9	24.5	21.4	19.4	18.1	17.0	16.1	15.4	14.3
		16	28.0	22.3	19.4	17.7	16.4	15.4	14.7	14.0	13.0
		24	24.5	19.4	17.0	15.4	14.3	13.5	12.8	12.2	11.4

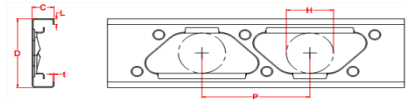
- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.
 - 7 - Web stiffeners required.



8" DeltaStud 3" flange

Section Identification	Design Condition ^{3,4,5,6}	Stud Spacing (in.)	Maximum Height (feet) Specified Wind Loads (psf)								
			5	10	15	20	25	30	35	40	50
800D300-54	Strength Only	12	81.2	57.4	46.9	40.6	36.3	33.1	30.7	26.9	21.5
		16	70.3	49.7	40.6	35.2	31.4	26.9	23.1	20.2	16.2
		24	57.4	40.6	33.1	26.9	21.5	18.0	15.4	13.5	10.8
	Deflection (L/360)	12	41.8	33.2	29.0	26.3	24.4	23.0	21.9	20.9	19.4
		16	38.0	30.1	26.3	23.9	22.2	20.9	19.9	19.0	17.6
		24	33.2	26.3	23.0	20.9	19.4	18.3	17.3	16.6	15.3
	Deflection (L/600)	12	35.3	28.0	24.4	22.2	20.6	19.4	18.4	17.6	16.4
		16	32.0	25.4	22.2	20.2	18.7	17.6	16.7	16.0	14.9
		24	28.0	22.2	19.4	17.6	16.4	15.4	14.6	14.0	13.0
	Deflection (L/720)	12	33.2	26.3	23.0	20.9	19.4	18.3	17.3	16.6	15.4
		16	30.1	23.9	20.9	19.0	17.6	16.6	15.8	15.1	14.0
		24	26.3	20.9	18.3	16.6	15.4	14.5	13.8	13.2	12.2
800D300-68	Strength Only	12	96.0	67.8	55.4	48.0	42.9	39.2	36.3	33.9	30.3
		16	83.1	58.8	48.0	41.5	37.2	33.9	31.4	29.4	24.8
		24	67.8	48.0	39.2	33.9	30.3	27.6	23.6	20.7	16.5
	Deflection (L/360)	12	44.9	35.7	31.2	28.3	26.3	24.7	23.5	22.5	20.9
		16	40.8	32.4	28.3	25.7	23.9	22.5	21.3	20.4	19.0
		24	35.7	28.3	24.7	22.5	20.9	19.6	18.6	17.8	16.6
	Deflection (L/600)	12	37.9	30.1	26.3	23.9	22.2	20.9	19.8	19.0	17.6
		16	34.4	27.3	23.9	21.7	20.1	19.0	18.0	17.2	16.0
		24	30.1	23.9	20.9	19.0	17.6	16.6	15.7	15.0	14.0
	Deflection (L/720)	12	35.7	28.3	24.7	22.5	20.9	19.6	18.6	17.8	16.6
		16	32.4	25.7	22.5	20.4	19.0	17.8	16.9	16.2	15.0
		24	28.3	22.5	19.6	17.8	16.6	15.6	14.8	14.2	13.1

- Notes:**
- 1 - For a detailed explanation of this table refer to the Design Criteria section.
 - 2 - Shaded heights in table are controlled by web crippling.
 - 3 - Strength values are based upon **factored** loads with factor of 1.4.
 - 4 - Deflection values are based upon **specified** load.
 - 5 - For other deflection limits see the Design Criteria section.
 - 6 - The lesser of the lengths given for strength and deflection will govern.



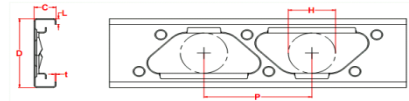
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 1-5/8" flange 33 mils - Section Identification: 362D162-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	4.46	4.46	4.46	3.83	3.63	3.25	3.25	2.88	2.20	2.71	2.20	1.27	2.20	1.57	0.42	1.72	0.98	-	1.27	0.42	-	0.83	-	-
9	4.28	4.28	4.28	3.47	3.23	2.78	2.78	2.35	1.57	2.15	1.57	0.54	1.57	0.87	-	1.04	0.22	-	0.54	-	-	0.07	-	-
10	4.05	4.05	4.05	3.08	2.80	2.29	2.29	1.82	0.98	1.60	0.98	-	0.98	0.24	-	0.42	-	-	-	-	-	-	-	-
11	3.82	3.82	3.82	2.69	2.38	1.82	1.82	1.33	0.46	1.10	0.46	-	0.46	-	-	-	-	-	-	-	-	-	-	-
12	3.58	3.58	3.58	2.31	1.98	1.40	1.40	0.89	0.01	0.66	0.01	-	0.01	-	-	-	-	-	-	-	-	-	-	-
13	3.34	3.34	3.34	1.95	1.62	1.03	1.03	0.52	-	0.28	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14	3.09	3.09	3.09	1.63	1.29	0.70	0.70	0.20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	2.85	2.85	2.85	1.33	1.00	0.43	0.43	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	2.62	2.62	2.62	1.08	0.75	0.19	0.19	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.39	2.39	2.39	0.86	0.54	0.00	0.00	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.16	2.16	2.16	0.66	0.36	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	1.97	1.97	1.97	0.50	0.21	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	1.80	1.80	1.80	0.37	0.09	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

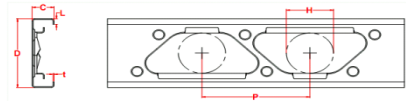
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 1-5/8" flange 43 mils - Section Identification: 362D162-43

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	6.24	6.24	6.24	5.61	5.41	5.03	5.03	4.66	3.96	4.48	3.96	2.99	3.96	3.30	2.09	3.46	2.68	1.26	2.99	2.09	0.48	2.53	1.53	-
9	5.95	5.95	5.95	5.15	4.90	4.43	4.43	3.99	3.18	3.78	3.18	2.09	3.18	2.44	1.11	2.62	1.75	0.21	2.09	1.11	-	1.59	0.51	-
10	5.64	5.64	5.64	4.65	4.36	3.83	3.83	3.34	2.44	3.10	2.44	1.27	2.44	1.64	0.24	1.83	0.92	-	1.27	0.24	-	0.74	-	-
11	5.32	5.32	5.32	4.15	3.83	3.24	3.24	2.70	1.76	2.45	1.76	0.55	1.76	0.93	-	1.13	0.19	-	0.55	-	-	0.01	-	-
12	4.98	4.98	4.98	3.65	3.30	2.68	2.68	2.12	1.16	1.87	1.16	-	1.16	0.33	-	0.53	-	-	-	-	-	-	-	-
13	4.65	4.65	4.65	3.17	2.81	2.17	2.17	1.61	0.65	1.35	0.65	-	0.65	-	-	0.02	-	-	-	-	-	-	-	-
14	4.31	4.31	4.31	2.72	2.35	1.71	1.71	1.16	0.22	0.90	0.22	-	0.22	-	-	-	-	-	-	-	-	-	-	-
15	3.97	3.97	3.97	2.32	1.95	1.32	1.32	0.78	-	0.53	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.64	3.64	3.64	1.95	1.60	0.98	0.98	0.46	-	0.22	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.31	3.31	3.31	1.63	1.29	0.69	0.69	0.19	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.00	3.00	3.00	1.35	1.02	0.45	0.45	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.73	2.73	2.73	1.12	0.80	0.25	0.25	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.49	2.49	2.49	0.92	0.61	0.09	0.09	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



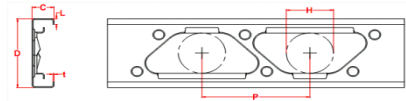
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 1-5/8" flange 54 mills - Section Identification: 362D162-54

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	10.78	10.78	10.78	10.07	9.85	9.41	9.41	9.00	8.21	8.80	8.21	7.13	8.21	7.48	6.14	7.66	6.80	5.23	7.13	6.14	4.37	6.63	5.53	3.55
9	10.02	10.02	10.02	9.11	8.83	8.31	8.31	7.82	6.92	7.59	6.92	5.72	6.92	6.10	4.64	6.30	5.35	3.66	5.72	4.64	2.75	5.17	3.98	1.91
10	9.24	9.24	9.24	8.12	7.80	7.20	7.20	6.66	5.68	6.40	5.68	4.42	5.68	4.82	3.31	5.03	4.03	2.31	4.42	3.31	1.39	3.85	2.63	0.55
11	8.44	8.44	8.44	7.13	6.78	6.14	6.14	5.57	4.56	5.30	4.56	3.29	4.56	3.69	2.17	3.90	2.90	1.19	3.29	2.17	0.29	2.72	1.51	-
12	7.64	7.64	7.64	6.19	5.82	5.16	5.16	4.59	3.59	4.32	3.59	2.33	3.59	2.72	1.25	2.93	1.95	0.30	2.33	1.25	-	1.77	0.60	-
13	6.86	6.86	6.86	5.32	4.94	4.29	4.29	3.73	2.75	3.47	2.75	1.54	2.75	1.92	0.51	2.12	1.18	-	1.54	0.51	-	1.01	-	-
14	6.09	6.09	6.09	4.52	4.16	3.53	3.53	2.99	2.06	2.74	2.06	0.90	2.06	1.26	-	1.45	0.56	-	0.90	-	-	0.40	-	-
15	5.40	5.40	5.40	3.84	3.49	2.89	2.89	2.37	1.49	2.14	1.49	0.40	1.49	0.74	-	0.92	0.07	-	0.40	-	-	-	-	-
16	4.82	4.82	4.82	3.28	2.95	2.37	2.37	1.87	1.04	1.65	1.04	0.00	1.04	0.32	-	0.49	-	-	0.00	-	-	-	-	-
17	4.29	4.29	4.29	2.80	2.47	1.93	1.93	1.46	0.66	1.24	0.66	-	0.66	-	-	0.15	-	-	-	-	-	-	-	-
18	3.82	3.82	3.82	2.39	2.08	1.56	1.56	1.12	0.36	0.91	0.36	-	0.36	-	-	-	-	-	-	-	-	-	-	-
19	3.43	3.43	3.43	2.04	1.75	1.26	1.26	0.84	0.12	0.64	0.12	-	0.12	-	-	-	-	-	-	-	-	-	-	-
20	3.10	3.10	3.10	1.76	1.48	1.01	1.01	0.60	-	0.42	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



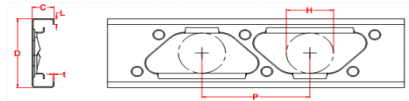
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 1-5/8" flange 68 mills - Section Identification: 362D162-68

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	14.31	14.31	14.31	13.54	13.30	12.82	12.82	12.37	11.51	12.15	11.51	10.34	11.51	10.72	9.25	10.92	9.96	8.23	10.34	9.25	7.28	9.78	8.56	6.37
9	13.28	13.28	13.28	12.27	11.96	11.39	11.39	10.85	9.86	10.59	9.86	8.52	9.86	8.95	7.33	9.17	8.11	6.24	8.52	7.33	5.23	7.91	6.59	4.27
10	12.21	12.21	12.21	10.97	10.61	9.94	9.94	9.34	8.25	9.05	8.25	6.84	8.25	7.28	5.60	7.52	6.41	4.48	6.84	5.60	3.46	6.20	4.84	2.51
11	11.12	11.12	11.12	9.66	9.26	8.54	8.54	7.90	6.77	7.60	6.77	5.34	6.77	5.79	4.11	6.03	4.91	3.01	5.34	4.11	2.00	4.71	3.36	1.07
12	10.04	10.04	10.04	8.39	7.98	7.24	7.24	6.59	5.47	6.29	5.47	4.07	5.47	4.51	2.87	4.74	3.65	1.80	4.07	2.87	0.83	3.45	2.14	-
13	8.89	8.89	8.89	7.17	6.76	6.04	6.04	5.41	4.33	5.12	4.33	2.99	4.33	3.41	1.85	3.63	2.59	0.83	2.99	1.85	-	2.40	1.16	-
14	7.74	7.74	7.74	6.05	5.65	4.97	4.97	4.37	3.36	4.10	3.36	2.10	3.36	2.49	1.03	2.70	1.73	0.08	2.10	1.03	-	1.55	0.38	-
15	6.74	6.74	6.74	5.11	4.74	4.09	4.09	3.53	2.58	3.28	2.58	1.40	2.58	1.77	0.39	1.96	1.05	-	1.40	0.39	-	0.88	-	-
16	5.93	5.93	5.93	4.35	4.00	3.38	3.38	2.86	1.96	2.62	1.96	0.85	1.96	1.20	-	1.38	0.52	-	0.85	-	-	0.36	-	-
17	5.25	5.25	5.25	3.73	3.39	2.81	2.81	2.31	1.46	2.08	1.46	0.42	1.46	0.74	-	0.91	0.11	-	0.42	-	-	-	-	-
18	4.68	4.68	4.68	3.21	2.89	2.34	2.34	1.87	1.06	1.65	1.06	0.07	1.06	0.38	-	0.54	-	-	0.07	-	-	-	-	-
19	4.20	4.20	4.20	2.78	2.47	1.95	1.95	1.50	0.73	1.29	0.73	-	0.73	0.09	-	0.24	-	-	-	-	-	-	-	-
20	3.79	3.79	3.79	2.42	2.12	1.62	1.62	1.19	0.46	0.99	0.46	-	0.46	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

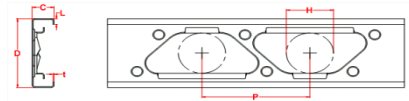


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 1-5/8" flange 33 mills - Section Identification: 400D162-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	4.60	4.60	4.60	4.05	3.87	3.53	3.53	3.20	2.57	3.04	2.57	1.69	2.57	1.98	0.88	2.13	1.42	0.11	1.69	0.88	-	1.28	0.36	-
9	4.44	4.44	4.44	3.75	3.53	3.11	3.11	2.71	1.98	2.52	1.98	0.97	1.98	1.30	0.06	1.46	0.66	-	0.97	0.06	-	0.51	-	-
10	4.27	4.27	4.27	3.41	3.15	2.67	2.67	2.22	1.39	2.00	1.39	0.30	1.39	0.65	-	0.83	-	-	0.30	-	-	-	-	-
11	4.06	4.06	4.06	3.05	2.76	2.22	2.22	1.72	0.85	1.49	0.85	-	0.85	0.06	-	0.25	-	-	-	-	-	-	-	-
12	3.85	3.85	3.85	2.69	2.37	1.79	1.79	1.27	0.36	1.03	0.36	-	0.36	-	-	-	-	-	-	-	-	-	-	-
13	3.63	3.63	3.63	2.34	2.00	1.39	1.39	0.86	-	0.61	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14	3.41	3.41	3.41	2.00	1.65	1.04	1.04	0.50	-	0.26	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.19	3.19	3.19	1.69	1.34	0.72	0.72	0.20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	2.97	2.97	2.97	1.41	1.06	0.45	0.45	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.75	2.75	2.75	1.16	0.82	0.22	0.22	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.53	2.53	2.53	0.94	0.60	0.03	0.03	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.32	2.32	2.32	0.75	0.42	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.13	2.13	2.13	0.58	0.27	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

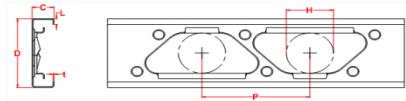


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 1-5/8" flange 43 mills - Section Identification: 400D162-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	6.48	6.48	6.48	5.94	5.76	5.42	5.42	5.08	4.44	4.92	4.44	3.54	4.44	3.83	2.69	3.98	3.25	1.89	3.54	2.69	1.12	3.11	2.15	0.40
9	6.23	6.23	6.23	5.53	5.31	4.89	4.89	4.49	3.72	4.29	3.72	2.67	3.72	3.01	1.71	3.18	2.34	0.82	2.67	1.71	-	2.18	1.11	-
10	5.96	5.96	5.96	5.10	4.83	4.33	4.33	3.87	3.00	3.64	3.00	1.84	3.00	2.21	0.80	2.40	1.48	-	1.84	0.80	-	1.31	0.16	-
11	5.67	5.67	5.67	4.64	4.34	3.77	3.77	3.25	2.31	3.00	2.31	1.08	2.31	1.47	0.00	1.68	0.71	-	1.08	0.00	-	0.53	-	-
12	5.38	5.38	5.38	4.18	3.84	3.22	3.22	2.67	1.68	2.41	1.68	0.41	1.68	0.81	-	1.02	0.03	-	0.41	-	-	-	-	-
13	5.07	5.07	5.07	3.71	3.35	2.70	2.70	2.12	1.11	1.86	1.11	-	1.11	0.24	-	0.45	-	-	-	-	-	-	-	-
14	4.77	4.77	4.77	3.27	2.89	2.22	2.22	1.64	0.62	1.36	0.62	-	0.62	-	-	-	-	-	-	-	-	-	-	-
15	4.45	4.45	4.45	2.85	2.46	1.79	1.79	1.20	0.21	0.93	0.21	-	0.21	-	-	-	-	-	-	-	-	-	-	-
16	4.15	4.15	4.15	2.46	2.07	1.41	1.41	0.83	-	0.57	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.84	3.84	3.84	2.11	1.73	1.07	1.07	0.51	-	0.25	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.54	3.54	3.54	1.79	1.42	0.78	0.78	0.24	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.24	3.24	3.24	1.51	1.15	0.53	0.53	0.01	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.97	2.97	2.97	1.26	0.92	0.32	0.32	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

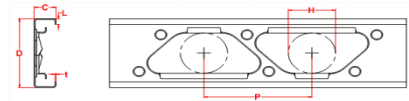


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 1-5/8" flange 54 mills - Section Identification: 400D162-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	11.39	11.39	11.39	10.78	10.59	10.21	10.21	9.83	9.12	9.65	9.12	8.12	9.12	8.45	7.18	8.61	7.80	6.29	8.12	7.18	5.45	7.64	6.58	4.65
9	10.72	10.72	10.72	9.94	9.69	9.22	9.22	8.77	7.92	8.55	7.92	6.76	7.92	7.13	5.70	7.32	6.39	4.72	6.76	5.70	3.80	6.21	5.03	2.93
10	10.02	10.02	10.02	9.05	8.75	8.20	8.20	7.68	6.73	7.43	6.73	5.46	6.73	5.87	4.33	6.07	5.07	3.29	5.46	4.33	2.33	4.88	3.62	1.44
11	9.30	9.30	9.30	8.14	7.80	7.18	7.18	6.61	5.59	6.35	5.59	4.27	5.59	4.69	3.10	4.91	3.86	2.05	4.27	3.10	1.09	3.67	2.39	0.20
12	8.57	8.57	8.57	7.24	6.87	6.21	6.21	5.61	4.56	5.33	4.56	3.22	4.56	3.64	2.05	3.86	2.81	1.01	3.22	2.05	0.07	2.62	1.35	-
13	7.84	7.84	7.84	6.37	5.98	5.30	5.30	4.69	3.64	4.41	3.64	2.32	3.64	2.74	1.18	2.95	1.92	0.17	2.32	1.18	-	1.73	0.49	-
14	7.12	7.12	7.12	5.55	5.16	4.48	4.48	3.88	2.85	3.61	2.85	1.56	2.85	1.97	0.46	2.18	1.18	-	1.56	0.46	-	0.99	-	-
15	6.42	6.42	6.42	4.81	4.42	3.75	3.75	3.17	2.18	2.91	2.18	0.94	2.18	1.33	-	1.53	0.57	-	0.94	-	-	0.40	-	-
16	5.75	5.75	5.75	4.14	3.76	3.12	3.12	2.56	1.61	2.31	1.61	0.43	1.61	0.80	-	0.99	0.08	-	0.43	-	-	-	-	-
17	5.18	5.18	5.18	3.58	3.21	2.59	2.59	2.06	1.15	1.82	1.15	0.03	1.15	0.38	-	0.56	-	-	0.03	-	-	-	-	-
18	4.69	4.69	4.69	3.10	2.75	2.15	2.15	1.64	0.77	1.41	0.77	-	0.77	0.03	-	0.21	-	-	-	-	-	-	-	-
19	4.27	4.27	4.27	2.70	2.36	1.79	1.79	1.29	0.46	1.07	0.46	-	0.46	-	-	-	-	-	-	-	-	-	-	-
20	3.89	3.89	3.89	2.35	2.02	1.47	1.47	1.00	0.20	0.78	0.20	-	0.20	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



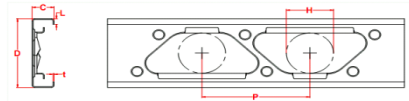
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 1-5/8" flange 68 mils - Section Identification: 400D162-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	15.20	15.20	15.20	14.55	14.34	13.93	13.93	13.53	12.75	13.33	12.75	11.66	12.75	12.02	10.63	12.20	11.31	9.66	11.66	10.63	8.73	11.14	9.98	7.85
9	14.30	14.30	14.30	13.45	13.18	12.66	12.66	12.16	11.24	11.92	11.24	9.96	11.24	10.37	8.78	10.58	9.55	7.69	9.96	8.78	6.67	9.36	8.05	5.70
10	13.35	13.35	13.35	12.27	11.94	11.34	11.34	10.76	9.70	10.49	9.70	8.29	9.70	8.74	7.02	8.97	7.85	5.87	8.29	7.02	4.80	7.64	6.24	3.79
11	12.37	12.37	12.37	11.07	10.70	10.00	10.00	9.36	8.22	9.06	8.22	6.73	8.22	7.21	5.43	7.45	6.28	4.25	6.73	5.43	3.17	6.06	4.63	2.17
12	11.38	11.38	11.38	9.87	9.46	8.71	8.71	8.04	6.86	7.72	6.86	5.35	6.86	5.83	4.05	6.07	4.90	2.88	5.35	4.05	1.81	4.68	3.26	0.83
13	10.39	10.39	10.39	8.71	8.27	7.50	7.50	6.82	5.64	6.50	5.64	4.15	5.64	4.62	2.87	4.86	3.71	1.73	4.15	2.87	0.70	3.49	2.10	-
14	9.41	9.41	9.41	7.61	7.17	6.40	6.40	5.73	4.57	5.42	4.57	3.13	4.57	3.58	1.89	3.82	2.70	0.80	3.13	1.89	-	2.49	1.15	-
15	8.45	8.45	8.45	6.60	6.17	5.42	5.42	4.77	3.66	4.47	3.66	2.27	3.66	2.71	1.09	2.93	1.86	0.05	2.27	1.09	-	1.66	0.39	-
16	7.46	7.46	7.46	5.66	5.25	4.53	4.53	3.92	2.86	3.63	2.86	1.56	2.86	1.97	0.45	2.18	1.17	-	1.56	0.45	-	0.98	-	-
17	6.61	6.61	6.61	4.87	4.47	3.79	3.79	3.21	2.22	2.94	2.22	0.98	2.22	1.37	-	1.57	0.62	-	0.98	-	-	0.44	-	-
18	5.90	5.90	5.90	4.21	3.83	3.19	3.19	2.63	1.69	2.38	1.69	0.52	1.69	0.89	-	1.08	0.18	-	0.52	-	-	0.01	-	-
19	5.29	5.29	5.29	3.66	3.30	2.68	2.68	2.16	1.26	1.91	1.26	0.15	1.26	0.50	-	0.68	-	-	0.15	-	-	-	-	-
20	4.78	4.78	4.78	3.19	2.85	2.26	2.26	1.76	0.90	1.53	0.90	-	0.90	0.18	-	0.35	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

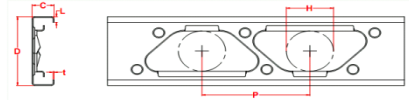


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 1-5/8" flange 33 mills - Section Identification: 600D162-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	4.96	4.96	4.96	4.66	4.56	4.36	4.36	4.17	3.77	4.07	3.77	3.20	3.77	3.39	2.63	3.48	3.01	2.08	3.20	2.63	1.54	2.91	2.27	1.01
9	4.89	4.89	4.89	4.51	4.38	4.13	4.13	3.88	3.39	3.76	3.39	2.68	3.39	2.91	1.99	3.03	2.45	1.32	2.68	1.99	0.67	2.33	1.55	0.04
10	4.82	4.82	4.82	4.34	4.18	3.87	3.87	3.57	2.98	3.42	2.98	2.13	2.98	2.41	1.33	2.55	1.86	0.54	2.13	1.33	-	1.72	0.80	-
11	4.72	4.72	4.72	4.14	3.95	3.59	3.59	3.23	2.54	3.05	2.54	1.56	2.54	1.88	0.64	2.04	1.25	-	1.56	0.64	-	1.10	0.05	-
12	4.62	4.62	4.62	3.93	3.71	3.28	3.28	2.87	2.08	2.67	2.08	0.99	2.08	1.34	-	1.52	0.64	-	0.99	-	-	0.47	-	-
13	4.51	4.51	4.51	3.70	3.45	2.96	2.96	2.50	1.63	2.27	1.63	0.43	1.63	0.82	-	1.01	0.06	-	0.43	-	-	-	-	-
14	4.39	4.39	4.39	3.47	3.18	2.64	2.64	2.12	1.18	1.88	1.18	-	1.18	0.31	-	0.52	-	-	-	-	-	-	-	-
15	4.25	4.25	4.25	3.21	2.89	2.30	2.30	1.75	0.75	1.49	0.75	-	0.75	-	-	0.06	-	-	-	-	-	-	-	-
16	4.11	4.11	4.11	2.95	2.61	1.97	1.97	1.39	0.34	1.11	0.34	-	0.34	-	-	-	-	-	-	-	-	-	-	-
17	3.96	3.96	3.96	2.69	2.32	1.65	1.65	1.05	-	0.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.81	3.81	3.81	2.43	2.05	1.35	1.35	0.73	-	0.44	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.66	3.66	3.66	2.18	1.78	1.07	1.07	0.44	-	0.15	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.50	3.50	3.50	1.94	1.53	0.81	0.81	0.17	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

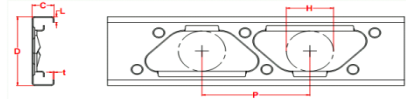


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 1-5/8" flange 43 mills - Section Identification: 600D162-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	7.16	7.16	7.16	6.86	6.76	6.56	6.56	6.37	5.97	6.27	5.97	5.39	5.97	5.58	4.82	5.68	5.20	4.25	5.39	4.82	3.70	5.10	4.44	3.15
9	7.03	7.03	7.03	6.65	6.52	6.26	6.26	6.01	5.52	5.89	5.52	4.80	5.52	5.04	4.10	5.16	4.57	3.41	4.80	4.10	2.74	4.45	3.64	2.09
10	6.88	6.88	6.88	6.40	6.25	5.94	5.94	5.63	5.04	5.48	5.04	4.17	5.04	4.46	3.34	4.60	3.89	2.53	4.17	3.34	1.76	3.75	2.80	1.00
11	6.72	6.72	6.72	6.15	5.96	5.59	5.59	5.23	4.52	5.05	4.52	3.52	4.52	3.85	2.56	4.01	3.19	1.65	3.52	2.56	0.77	3.03	1.95	-
12	6.56	6.56	6.56	5.87	5.65	5.21	5.21	4.79	3.98	4.59	3.98	2.85	3.98	3.22	1.78	3.41	2.48	0.78	2.85	1.78	-	2.31	1.11	-
13	6.38	6.38	6.38	5.57	5.32	4.82	4.82	4.34	3.44	4.11	3.44	2.18	3.44	2.59	1.03	2.80	1.79	-	2.18	1.03	-	1.59	0.30	-
14	6.19	6.19	6.19	5.26	4.97	4.41	4.41	3.88	2.89	3.63	2.89	1.54	2.89	1.98	0.31	2.20	1.12	-	1.54	0.31	-	0.91	-	-
15	6.00	6.00	6.00	4.94	4.62	4.00	4.00	3.42	2.36	3.15	2.36	0.93	2.36	1.39	-	1.62	0.49	-	0.93	-	-	0.28	-	-
16	5.80	5.80	5.80	4.61	4.26	3.59	3.59	2.97	1.85	2.68	1.85	0.37	1.85	0.84	-	1.08	-	-	0.37	-	-	-	-	-
17	5.59	5.59	5.59	4.28	3.89	3.19	3.19	2.54	1.37	2.23	1.37	-	1.37	0.34	-	0.58	-	-	-	-	-	-	-	-
18	5.38	5.38	5.38	3.95	3.54	2.80	2.80	2.12	0.93	1.81	0.93	-	0.93	-	-	0.13	-	-	-	-	-	-	-	-
19	5.17	5.17	5.17	3.62	3.20	2.42	2.42	1.74	0.52	1.41	0.52	-	0.52	-	-	-	-	-	-	-	-	-	-	-
20	4.96	4.96	4.96	3.31	2.87	2.07	2.07	1.38	0.16	1.05	0.16	-	0.16	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



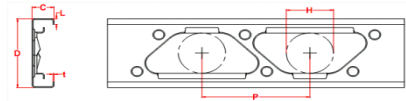
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 1-5/8" flange 54 mills - Section Identification: 600D162-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	13.02	13.02	13.02	12.70	12.60	12.39	12.39	12.17	11.76	12.07	11.76	11.14	11.76	11.35	10.54	11.45	10.94	9.94	11.14	10.54	9.35	10.84	10.14	8.77
9	12.77	12.77	12.77	12.37	12.22	11.95	11.95	11.68	11.16	11.55	11.16	10.38	11.16	10.64	9.62	10.77	10.13	8.89	10.38	9.62	8.16	10.00	9.13	7.45
10	12.37	12.37	12.37	11.86	11.69	11.36	11.36	11.03	10.39	10.87	10.39	9.45	10.39	9.76	8.55	9.91	9.15	7.68	9.45	8.55	6.84	9.00	7.97	6.02
11	11.94	11.94	11.94	11.32	11.12	10.72	10.72	10.33	9.57	10.13	9.57	8.48	9.57	8.83	7.44	9.01	8.13	6.45	8.48	7.44	5.50	7.95	6.77	4.59
12	11.49	11.49	11.49	10.75	10.51	10.04	10.04	9.58	8.71	9.36	8.71	7.48	8.71	7.88	6.32	8.08	7.09	5.23	7.48	6.32	4.20	6.89	5.59	3.21
13	11.02	11.02	11.02	10.15	9.87	9.34	9.34	8.82	7.84	8.57	7.84	6.48	7.84	6.92	5.24	7.15	6.06	4.07	6.48	5.24	2.97	5.85	4.45	1.94
14	10.54	10.54	10.54	9.53	9.22	8.62	8.62	8.04	6.97	7.77	6.97	5.52	6.97	5.99	4.20	6.23	5.07	2.98	5.52	4.20	1.85	4.85	3.38	0.78
15	10.04	10.04	10.04	8.90	8.55	7.89	7.89	7.27	6.13	6.98	6.13	4.61	6.13	5.09	3.24	5.35	4.14	1.99	4.61	3.24	0.84	3.91	2.40	-
16	9.54	9.54	9.54	8.27	7.89	7.19	7.19	6.53	5.34	6.22	5.34	3.76	5.34	4.26	2.36	4.52	3.28	1.10	3.76	2.36	-	3.04	1.51	-
17	9.03	9.03	9.03	7.64	7.24	6.49	6.49	5.81	4.58	5.49	4.58	2.98	4.58	3.48	1.58	3.75	2.49	0.32	2.98	1.58	-	2.26	0.72	-
18	8.53	8.53	8.53	7.03	6.60	5.83	5.83	5.13	3.88	4.80	3.88	2.28	3.88	2.78	0.88	3.04	1.79	-	2.28	0.88	-	1.56	0.03	-
19	8.02	8.02	8.02	6.43	5.99	5.20	5.20	4.49	3.25	4.16	3.25	1.65	3.25	2.15	0.28	2.41	1.17	-	1.65	0.28	-	0.94	-	-
20	7.52	7.52	7.52	5.86	5.41	4.62	4.62	3.91	2.67	3.58	2.67	1.10	2.67	1.59	-	1.85	0.63	-	1.10	-	-	0.41	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



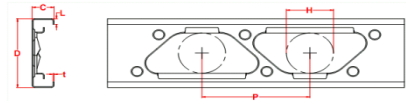
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 1-5/8" flange 68 mills - Section Identification: 600D162-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	17.80	17.80	17.80	17.47	17.36	17.14	17.14	16.90	16.45	16.78	16.45	15.79	16.45	16.01	15.14	16.12	15.57	14.50	15.79	15.14	13.86	15.46	14.71	13.23
9	17.30	17.30	17.30	16.88	16.72	16.43	16.43	16.15	15.58	16.01	15.58	14.75	15.58	15.03	13.94	15.17	14.48	13.15	14.75	13.94	12.37	14.35	13.41	11.60
10	16.76	16.76	16.76	16.22	16.04	15.68	15.68	15.33	14.63	15.15	14.63	13.63	14.63	13.96	12.65	14.13	13.30	11.70	13.63	12.65	10.78	13.13	12.02	9.89
11	16.19	16.19	16.19	15.52	15.30	14.86	14.86	14.44	13.61	14.23	13.61	12.43	13.61	12.82	11.29	13.01	12.04	10.21	12.43	11.29	9.16	11.85	10.57	8.16
12	15.58	15.58	15.58	14.77	14.51	14.00	14.00	13.50	12.54	13.26	12.54	11.19	12.54	11.63	9.91	11.85	10.75	8.71	11.19	9.91	7.56	10.54	9.10	6.46
13	14.95	14.95	14.95	13.99	13.69	13.09	13.09	12.52	11.44	12.25	11.44	9.94	11.44	10.42	8.55	10.67	9.46	7.25	9.94	8.55	6.02	9.23	7.67	4.86
14	14.29	14.29	14.29	13.18	12.83	12.16	12.16	11.52	10.33	11.22	10.33	8.72	10.33	9.24	7.23	9.51	8.20	5.86	8.72	7.23	4.59	7.95	6.30	3.38
15	13.62	13.62	13.62	12.35	11.96	11.21	11.21	10.53	9.25	10.20	9.25	7.54	9.25	8.09	5.98	8.37	7.00	4.58	7.54	5.98	3.27	6.75	5.03	2.05
16	12.94	12.94	12.94	11.51	11.09	10.29	10.29	9.55	8.20	9.20	8.20	6.42	8.20	6.99	4.84	7.28	5.88	3.41	6.42	4.84	2.10	5.61	3.87	0.87
17	12.25	12.25	12.25	10.68	10.22	9.37	9.37	8.60	7.20	8.23	7.20	5.39	7.20	5.97	3.80	6.27	4.84	2.37	5.39	3.80	1.06	4.58	2.83	-
18	11.56	11.56	11.56	9.85	9.37	8.49	8.49	7.69	6.27	7.32	6.27	4.45	6.27	5.03	2.87	5.33	3.90	1.45	4.45	2.87	0.15	3.64	1.91	-
19	10.88	10.88	10.88	9.05	8.55	7.64	7.64	6.84	5.42	6.46	5.42	3.61	5.42	4.18	2.04	4.47	3.06	0.65	3.61	2.04	-	2.79	1.09	-
20	10.20	10.20	10.20	8.28	7.77	6.85	6.85	6.05	4.64	5.67	4.64	2.85	4.64	3.41	1.31	3.70	2.31	-	2.85	1.31	-	2.05	0.39	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

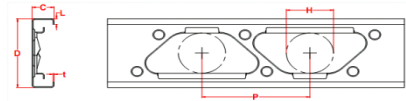


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 1-5/8" flange 33 mills - Section Identification: 800D162-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.10	5.10	5.10	4.86	4.78	4.62	4.62	4.46	4.14	4.38	4.14	3.66	4.14	3.82	3.19	3.90	3.51	2.72	3.66	3.19	2.26	3.43	2.88	1.80
9	5.06	5.06	5.06	4.75	4.65	4.45	4.45	4.24	3.84	4.14	3.84	3.25	3.84	3.45	2.66	3.54	3.05	2.08	3.25	2.66	1.50	2.95	2.27	0.94
10	5.01	5.01	5.01	4.63	4.51	4.26	4.26	4.01	3.52	3.88	3.52	2.79	3.52	3.03	2.08	3.15	2.55	1.38	2.79	2.08	0.69	2.43	1.61	0.02
11	4.96	4.96	4.96	4.50	4.35	4.05	4.05	3.75	3.16	3.60	3.16	2.30	3.16	2.59	1.46	2.73	2.02	0.65	2.30	1.46	-	1.88	0.92	-
12	4.90	4.90	4.90	4.36	4.17	3.81	3.81	3.46	2.78	3.29	2.78	1.78	2.78	2.11	0.82	2.27	1.46	-	1.78	0.82	-	1.30	0.20	-
13	4.84	4.84	4.84	4.20	3.99	3.57	3.57	3.17	2.38	2.97	2.38	1.25	2.38	1.62	0.17	1.81	0.88	-	1.25	0.17	-	0.70	-	-
14	4.77	4.77	4.77	4.02	3.78	3.31	3.31	2.85	1.96	2.62	1.96	0.71	1.96	1.12	-	1.33	0.30	-	0.71	-	-	0.10	-	-
15	4.69	4.69	4.69	3.84	3.57	3.03	3.03	2.52	1.54	2.27	1.54	0.17	1.54	0.61	-	0.84	-	-	0.17	-	-	-	-	-
16	4.61	4.61	4.61	3.65	3.34	2.75	2.75	2.18	1.12	1.91	1.12	-	1.12	0.12	-	0.36	-	-	-	-	-	-	-	-
17	4.52	4.52	4.52	3.44	3.11	2.46	2.46	1.85	0.70	1.55	0.70	-	0.70	-	-	-	-	-	-	-	-	-	-	-
18	4.43	4.43	4.43	3.24	2.87	2.17	2.17	1.51	0.29	1.20	0.29	-	0.29	-	-	-	-	-	-	-	-	-	-	-
19	4.33	4.33	4.33	3.02	2.62	1.88	1.88	1.18	-	0.85	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.21	4.21	4.21	2.80	2.38	1.59	1.59	0.86	-	0.52	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



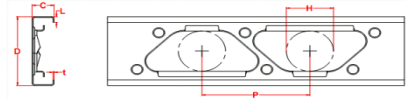
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 1-5/8" flange 43 mills - Section Identification: 800D162-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	7.44	7.44	7.44	7.19	7.11	6.95	6.95	6.78	6.46	6.70	6.46	5.97	6.46	6.13	5.48	6.21	5.81	5.00	5.97	5.48	4.52	5.73	5.16	4.05
9	7.36	7.36	7.36	7.05	6.94	6.73	6.73	6.53	6.11	6.42	6.11	5.50	6.11	5.71	4.90	5.81	5.30	4.30	5.50	4.90	3.70	5.20	4.49	3.11
10	7.27	7.27	7.27	6.88	6.75	6.50	6.50	6.24	5.74	6.12	5.74	4.99	5.74	5.24	4.26	5.36	4.75	3.53	4.99	4.26	2.82	4.62	3.77	2.11
11	7.17	7.17	7.17	6.70	6.55	6.24	6.24	5.93	5.32	5.78	5.32	4.43	5.32	4.73	3.56	4.88	4.14	2.72	4.43	3.56	1.89	4.00	3.00	1.07
12	7.06	7.06	7.06	6.51	6.32	5.95	5.95	5.59	4.88	5.41	4.88	3.85	4.88	4.19	2.85	4.36	3.51	1.88	3.85	2.85	0.93	3.34	2.20	0.00
13	6.95	6.95	6.95	6.29	6.08	5.65	5.65	5.23	4.42	5.03	4.42	3.24	4.42	3.63	2.11	3.82	2.86	1.02	3.24	2.11	-	2.67	1.38	-
14	6.82	6.82	6.82	6.07	5.82	5.33	5.33	4.86	3.94	4.62	3.94	2.62	3.94	3.05	1.37	3.27	2.19	0.17	2.62	1.37	-	1.98	0.57	-
15	6.70	6.70	6.70	5.83	5.55	5.00	5.00	4.47	3.44	4.21	3.44	1.99	3.44	2.46	0.63	2.70	1.53	-	1.99	0.63	-	1.31	-	-
16	6.56	6.56	6.56	5.58	5.27	4.65	4.65	4.07	2.94	3.78	2.94	1.37	2.94	1.88	-	2.14	0.88	-	1.37	-	-	0.63	-	-
17	6.42	6.42	6.42	5.32	4.97	4.30	4.30	3.66	2.44	3.35	2.44	0.77	2.44	1.31	-	1.59	0.25	-	0.77	-	-	-	-	-
18	6.28	6.28	6.28	5.06	4.67	3.94	3.94	3.25	1.95	2.92	1.95	0.19	1.95	0.76	-	1.05	-	-	0.19	-	-	-	-	-
19	6.13	6.13	6.13	4.78	4.37	3.58	3.58	2.85	1.48	2.49	1.48	-	1.48	0.23	-	0.54	-	-	-	-	-	-	-	-
20	5.98	5.98	5.98	4.51	4.06	3.23	3.23	2.45	1.02	2.08	1.02	-	1.02	-	-	0.04	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



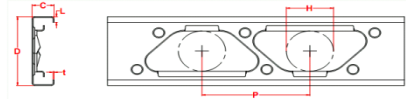
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 1-5/8" flange 54 mills - Section Identification: 800D162-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	13.56	13.56	13.56	13.31	13.22	13.06	13.06	12.89	12.55	12.81	12.55	12.06	12.55	12.22	11.56	12.31	11.89	11.07	12.06	11.56	10.57	11.81	11.23	10.08
9	13.40	13.40	13.40	13.08	12.97	12.76	12.76	12.55	12.13	12.44	12.13	11.49	12.13	11.71	10.86	11.81	11.28	10.23	11.49	10.86	9.62	11.18	10.44	9.01
10	13.23	13.23	13.23	12.83	12.70	12.44	12.44	12.17	11.64	12.04	11.64	10.86	11.64	11.12	10.09	11.25	10.60	9.33	10.86	10.09	8.58	10.47	9.58	7.85
11	13.05	13.05	13.05	12.56	12.40	12.07	12.07	11.75	11.11	11.59	11.11	10.18	11.11	10.49	9.26	10.64	9.87	8.36	10.18	9.26	7.48	9.72	8.66	6.62
12	12.81	12.81	12.81	12.22	12.02	11.64	11.64	11.26	10.51	11.07	10.51	9.41	10.51	9.77	8.35	9.96	9.05	7.31	9.41	8.35	6.30	8.88	7.65	5.32
13	12.49	12.49	12.49	11.80	11.58	11.13	11.13	10.69	9.82	10.47	9.82	8.57	9.82	8.98	7.37	9.19	8.16	6.20	8.57	7.37	5.08	7.96	6.59	3.99
14	12.16	12.16	12.16	11.36	11.10	10.59	10.59	10.09	9.11	9.84	9.11	7.71	9.11	8.17	6.37	8.40	7.25	5.10	7.71	6.37	3.87	7.03	5.51	2.69
15	11.82	11.82	11.82	10.90	10.61	10.03	10.03	9.46	8.38	9.19	8.38	6.83	8.38	7.34	5.38	7.59	6.34	4.01	6.83	5.38	2.70	6.10	4.46	1.45
16	11.46	11.46	11.46	10.43	10.10	9.45	9.45	8.83	7.64	8.52	7.64	5.97	7.64	6.51	4.41	6.79	5.44	2.96	5.97	4.41	1.58	5.18	3.43	0.27
17	11.10	11.10	11.10	9.94	9.57	8.86	8.86	8.18	6.90	7.85	6.90	5.12	6.90	5.69	3.48	5.99	4.56	1.96	5.12	3.48	0.53	4.29	2.46	-
18	10.72	10.72	10.72	9.44	9.04	8.27	8.27	7.54	6.17	7.18	6.17	4.30	6.17	4.90	2.60	5.21	3.72	1.04	4.30	2.60	-	3.43	1.54	-
19	10.34	10.34	10.34	8.93	8.50	7.68	7.68	6.90	5.46	6.53	5.46	3.52	5.46	4.14	1.78	4.46	2.92	0.17	3.52	1.78	-	2.63	0.70	-
20	9.95	9.95	9.95	8.43	7.96	7.09	7.09	6.27	4.78	5.89	4.78	2.79	4.78	3.43	1.01	3.75	2.18	-	2.79	1.01	-	1.88	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



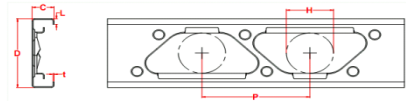
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 1-5/8" flange 68 mills - Section Identification: 800D162-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	18.89	18.89	18.89	18.63	18.54	18.37	18.37	18.20	17.85	18.11	17.85	17.33	17.85	17.50	16.81	17.59	17.16	16.29	17.33	16.81	15.78	17.07	16.47	15.27
9	18.58	18.58	18.58	18.25	18.14	17.92	17.92	17.70	17.26	17.59	17.26	16.60	17.26	16.82	15.93	16.93	16.38	15.28	16.60	15.93	14.64	16.27	15.50	14.00
10	18.24	18.24	18.24	17.83	17.69	17.42	17.42	17.15	16.58	17.01	16.58	15.77	16.58	16.04	14.97	16.18	15.50	14.18	15.77	14.97	13.40	15.37	14.44	12.63
11	17.86	17.86	17.86	17.37	17.20	16.85	16.85	16.52	15.86	16.36	15.86	14.89	15.86	15.21	13.94	15.38	14.57	13.00	14.89	13.94	12.08	14.41	13.31	11.17
12	17.46	17.46	17.46	16.87	16.66	16.26	16.26	15.87	15.09	15.67	15.09	13.95	15.09	14.33	12.84	14.51	13.57	11.75	13.95	12.84	10.69	13.39	12.11	9.65
13	17.04	17.04	17.04	16.33	16.09	15.63	15.63	15.17	14.27	14.94	14.27	12.95	14.27	13.39	11.69	13.60	12.53	10.46	12.95	11.69	9.27	12.32	10.86	8.11
14	16.60	16.60	16.60	15.77	15.49	14.96	14.96	14.43	13.40	14.17	13.40	11.92	13.40	12.41	10.51	12.65	11.44	9.15	11.92	10.51	7.84	11.21	9.59	6.57
15	16.13	16.13	16.13	15.18	14.87	14.26	14.26	13.66	12.51	13.37	12.51	10.87	12.51	11.41	9.32	11.68	10.34	7.84	10.87	9.32	6.43	10.08	8.32	5.07
16	15.65	15.65	15.65	14.57	14.22	13.53	13.53	12.87	11.60	12.55	11.60	9.81	11.60	10.39	8.13	10.69	9.24	6.56	9.81	8.13	5.06	8.96	7.07	3.63
17	15.15	15.15	15.15	13.93	13.54	12.79	12.79	12.06	10.68	11.71	10.68	8.76	10.68	9.38	6.98	9.70	8.15	5.32	8.76	6.98	3.76	7.85	5.86	2.28
18	14.65	14.65	14.65	13.29	12.86	12.03	12.03	11.24	9.76	10.86	9.76	7.73	9.76	8.38	5.87	8.72	7.09	4.15	7.73	5.87	2.54	6.78	4.71	1.03
19	14.13	14.13	14.13	12.63	12.16	11.27	11.27	10.43	8.86	10.02	8.86	6.74	8.86	7.42	4.82	7.77	6.08	3.06	6.74	4.82	1.41	5.76	3.63	-
20	13.60	13.60	13.60	11.96	11.46	10.51	10.51	9.62	7.98	9.19	7.98	5.79	7.98	6.49	3.83	6.85	5.12	2.04	5.79	3.83	0.38	4.79	2.62	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

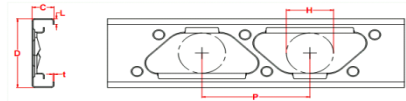


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 2" flange 33 mills - Section Identification: 362D200-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	4.85	4.85	4.85	4.24	4.04	3.66	3.66	3.30	2.61	3.12	2.61	1.66	2.61	1.97	0.79	2.13	1.36	-	1.66	0.79	-	1.22	0.24	-
9	4.68	4.68	4.68	3.89	3.65	3.19	3.19	2.76	1.96	2.55	1.96	0.89	1.96	1.24	-	1.41	0.56	-	0.89	-	-	0.40	-	-
10	4.49	4.49	4.49	3.52	3.24	2.71	2.71	2.22	1.34	1.99	1.34	0.19	1.34	0.56	-	0.75	-	-	0.19	-	-	-	-	-
11	4.29	4.29	4.29	3.13	2.82	2.23	2.23	1.70	0.77	1.46	0.77	-	0.77	-	-	0.15	-	-	-	-	-	-	-	-
12	4.06	4.06	4.06	2.75	2.40	1.78	1.78	1.23	0.27	0.97	0.27	-	0.27	-	-	-	-	-	-	-	-	-	-	-
13	3.84	3.84	3.84	2.37	2.00	1.36	1.36	0.80	-	0.55	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14	3.60	3.60	3.60	2.01	1.64	0.99	0.99	0.43	-	0.18	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.36	3.36	3.36	1.69	1.31	0.67	0.67	0.12	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.10	3.10	3.10	1.39	1.02	0.39	0.39	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.83	2.83	2.83	1.12	0.77	0.16	0.16	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.57	2.57	2.57	0.89	0.55	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.33	2.33	2.33	0.69	0.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.13	2.13	2.13	0.53	0.22	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

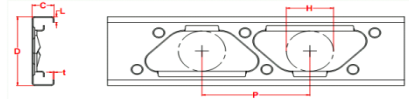


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 2" flange 43 mills - Section Identification: 362D200-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24			
8	7.26	7.26	7.26	6.56	6.34	5.91	5.91	5.50	4.71	5.30	4.71	3.62	4.71	3.98	2.62	4.16	3.28	1.68	3.62	2.62	0.80	3.11	1.99	-
9	6.92	6.92	6.92	6.04	5.76	5.24	5.24	4.74	3.83	4.51	3.83	2.60	3.83	3.00	1.49	3.20	2.22	0.48	2.60	1.49	-	2.03	0.80	-
10	6.57	6.57	6.57	5.47	5.15	4.54	4.54	3.99	2.98	3.72	2.98	1.66	2.98	2.08	0.49	2.29	1.25	-	1.66	0.49	-	1.05	-	-
11	6.20	6.20	6.20	4.90	4.53	3.87	3.87	3.26	2.19	2.98	2.19	0.82	2.19	1.26	-	1.48	0.41	-	0.82	-	-	0.21	-	-
12	5.82	5.82	5.82	4.33	3.93	3.22	3.22	2.59	1.50	2.30	1.50	0.11	1.50	0.55	-	0.78	-	-	0.11	-	-	-	-	-
13	5.43	5.43	5.43	3.77	3.36	2.63	2.63	1.99	0.90	1.70	0.90	-	0.90	-	-	0.18	-	-	-	-	-	-	-	-
14	5.04	5.04	5.04	3.26	2.83	2.10	2.10	1.47	0.39	1.18	0.39	-	0.39	-	-	-	-	-	-	-	-	-	-	-
15	4.65	4.65	4.65	2.78	2.36	1.64	1.64	1.02	-	0.74	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	4.27	4.27	4.27	2.36	1.95	1.24	1.24	0.64	-	0.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.90	3.90	3.90	1.98	1.58	0.90	0.90	0.32	-	0.06	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.53	3.53	3.53	1.65	1.27	0.61	0.61	0.06	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.21	3.21	3.21	1.37	1.00	0.38	0.38	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.93	2.93	2.93	1.13	0.78	0.18	0.18	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

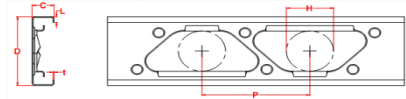


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 2" flange 54 mills - Section Identification: 362D200-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	12.55	12.55	12.55	11.74	11.49	10.99	10.99	10.52	9.62	10.29	9.62	8.38	9.62	8.78	7.24	8.98	7.99	6.19	8.38	7.24	5.20	7.80	6.53	4.26
9	11.69	11.69	11.69	10.65	10.33	9.74	9.74	9.18	8.14	8.91	8.14	6.76	8.14	7.20	5.52	7.43	6.33	4.37	6.76	5.52	3.33	6.12	4.75	2.35
10	10.79	10.79	10.79	9.52	9.15	8.46	8.46	7.84	6.71	7.54	6.71	5.25	6.71	5.71	3.96	5.95	4.80	2.80	5.25	3.96	1.74	4.59	3.17	0.75
11	9.88	9.88	9.88	8.38	7.97	7.24	7.24	6.57	5.41	6.27	5.41	3.93	5.41	4.39	2.64	4.64	3.48	1.49	3.93	2.64	0.45	3.26	1.86	-
12	8.96	8.96	8.96	7.29	6.86	6.10	6.10	5.43	4.27	5.12	4.27	2.81	4.27	3.27	1.55	3.51	2.37	0.44	2.81	1.55	-	2.16	0.80	-
13	8.06	8.06	8.06	6.28	5.85	5.09	5.09	4.43	3.30	4.13	3.30	1.88	3.30	2.33	0.68	2.56	1.46	-	1.88	0.68	-	1.26	-	-
14	7.18	7.18	7.18	5.36	4.94	4.20	4.20	3.56	2.48	3.27	2.48	1.13	2.48	1.55	-	1.77	0.73	-	1.13	-	-	0.54	-	-
15	6.36	6.36	6.36	4.55	4.14	3.44	3.44	2.84	1.81	2.56	1.81	0.53	1.81	0.93	-	1.13	0.15	-	0.53	-	-	-	-	-
16	5.67	5.67	5.67	3.89	3.49	2.82	2.82	2.24	1.27	1.98	1.27	0.05	1.27	0.43	-	0.63	-	-	0.05	-	-	-	-	-
17	5.06	5.06	5.06	3.32	2.95	2.30	2.30	1.76	0.83	1.51	0.83	-	0.83	0.04	-	0.23	-	-	-	-	-	-	-	-
18	4.51	4.51	4.51	2.83	2.48	1.87	1.87	1.35	0.47	1.11	0.47	-	0.47	-	-	-	-	-	-	-	-	-	-	-
19	4.05	4.05	4.05	2.43	2.09	1.51	1.51	1.02	0.18	0.79	0.18	-	0.18	-	-	-	-	-	-	-	-	-	-	-
20	3.65	3.65	3.65	2.09	1.77	1.21	1.21	0.74	-	0.53	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

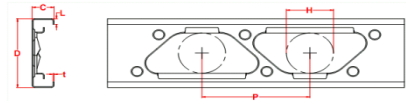


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 2" flange 68 mills - Section Identification: 362D200-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	16.56	16.56	16.56	15.78	15.53	15.04	15.04	14.57	13.68	14.34	13.68	12.45	13.68	12.85	11.30	13.04	12.06	10.22	12.45	11.30	9.21	11.86	10.57	8.24
9	15.39	15.39	15.39	14.37	14.05	13.46	13.46	12.89	11.84	12.62	11.84	10.42	11.84	10.88	9.14	11.11	9.98	7.97	10.42	9.14	6.88	9.77	8.35	5.85
10	14.18	14.18	14.18	12.91	12.53	11.83	11.83	11.19	10.03	10.89	10.03	8.51	10.03	8.99	7.17	9.24	8.04	5.95	8.51	7.17	4.84	7.82	6.35	3.79
11	12.95	12.95	12.95	11.43	11.01	10.25	10.25	9.56	8.34	9.24	8.34	6.79	8.34	7.28	5.44	7.53	6.32	4.23	6.79	5.44	3.13	6.09	4.62	2.10
12	11.72	11.72	11.72	10.00	9.55	8.76	8.76	8.05	6.84	7.73	6.84	5.30	6.84	5.78	3.98	6.03	4.84	2.80	5.30	3.98	1.73	4.62	3.18	0.74
13	10.46	10.46	10.46	8.63	8.18	7.38	7.38	6.70	5.51	6.38	5.51	4.03	5.51	4.49	2.76	4.73	3.59	1.63	4.03	2.76	0.62	3.37	1.99	-
14	9.16	9.16	9.16	7.34	6.90	6.14	6.14	5.48	4.36	5.18	4.36	2.96	4.36	3.40	1.76	3.63	2.54	0.71	2.96	1.76	-	2.34	1.05	-
15	7.98	7.98	7.98	6.22	5.80	5.09	5.09	4.47	3.41	4.19	3.41	2.10	3.41	2.51	0.98	2.72	1.71	-	2.10	0.98	-	1.52	0.31	-
16	7.02	7.02	7.02	5.31	4.92	4.24	4.24	3.65	2.66	3.39	2.66	1.42	2.66	1.81	0.37	2.01	1.05	-	1.42	0.37	-	0.88	-	-
17	6.21	6.21	6.21	4.56	4.19	3.55	3.55	2.99	2.05	2.74	2.05	0.88	2.05	1.25	-	1.44	0.54	-	0.88	-	-	0.37	-	-
18	5.54	5.54	5.54	3.94	3.59	2.98	2.98	2.45	1.56	2.21	1.56	0.45	1.56	0.80	-	0.98	0.13	-	0.45	-	-	-	-	-
19	4.97	4.97	4.97	3.43	3.09	2.50	2.50	2.00	1.15	1.77	1.15	0.11	1.15	0.43	-	0.60	-	-	0.11	-	-	-	-	-
20	4.49	4.49	4.49	2.99	2.66	2.11	2.11	1.63	0.82	1.41	0.82	-	0.82	0.14	-	0.30	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

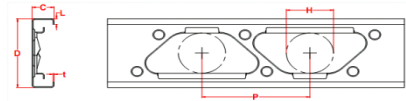


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 2" flange 33 mils - Section Identification: 400D200-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	4.99	4.99	4.99	4.45	4.28	3.94	3.94	3.62	2.99	3.46	2.99	2.10	2.99	2.39	1.27	2.54	1.82	0.49	2.10	1.27	-	1.68	0.75	-
9	4.84	4.84	4.84	4.16	3.94	3.53	3.53	3.13	2.38	2.94	2.38	1.36	2.38	1.69	0.42	1.86	1.03	-	1.36	0.42	-	0.88	-	-
10	4.68	4.68	4.68	3.83	3.58	3.09	3.09	2.63	1.78	2.41	1.78	0.64	1.78	1.01	-	1.19	0.29	-	0.64	-	-	0.12	-	-
11	4.51	4.51	4.51	3.49	3.19	2.63	2.63	2.12	1.20	1.88	1.20	-	1.20	0.38	-	0.57	-	-	-	-	-	-	-	-
12	4.32	4.32	4.32	3.13	2.80	2.19	2.19	1.64	0.67	1.38	0.67	-	0.67	-	-	0.02	-	-	-	-	-	-	-	-
13	4.12	4.12	4.12	2.77	2.41	1.76	1.76	1.19	0.19	0.93	0.19	-	0.19	-	-	-	-	-	-	-	-	-	-	-
14	3.91	3.91	3.91	2.42	2.04	1.37	1.37	0.79	-	0.52	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.70	3.70	3.70	2.09	1.70	1.02	1.02	0.44	-	0.17	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.48	3.48	3.48	1.78	1.39	0.71	0.71	0.13	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.25	3.25	3.25	1.49	1.10	0.44	0.44	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.00	3.00	3.00	1.23	0.85	0.20	0.20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.76	2.76	2.76	1.00	0.63	0.00	0.00	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.52	2.52	2.52	0.79	0.44	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

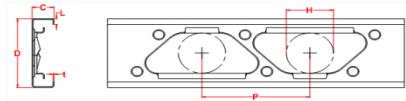


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 2" flange 43 mils - Section Identification: 400D200-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	7.51	7.51	7.51	6.91	6.71	6.33	6.33	5.96	5.25	5.78	5.25	4.24	5.25	4.57	3.29	4.73	3.91	2.38	4.24	3.29	1.53	3.75	2.68	0.71
9	7.24	7.24	7.24	6.47	6.22	5.75	5.75	5.30	4.44	5.08	4.44	3.26	4.44	3.64	2.18	3.84	2.89	1.17	3.26	2.18	0.22	2.71	1.50	-
10	6.93	6.93	6.93	5.97	5.68	5.12	5.12	4.59	3.62	4.34	3.62	2.31	3.62	2.73	1.13	2.95	1.91	0.06	2.31	1.13	-	1.71	0.41	-
11	6.61	6.61	6.61	5.45	5.11	4.48	4.48	3.89	2.83	3.61	2.83	1.44	2.83	1.88	0.21	2.11	1.01	-	1.44	0.21	-	0.80	-	-
12	6.27	6.27	6.27	4.93	4.55	3.85	3.85	3.22	2.10	2.92	2.10	0.66	2.10	1.11	-	1.35	0.22	-	0.66	-	-	0.01	-	-
13	5.92	5.92	5.92	4.40	3.99	3.25	3.25	2.59	1.44	2.29	1.44	-	1.44	0.45	-	0.68	-	-	-	-	-	-	-	-
14	5.57	5.57	5.57	3.88	3.45	2.69	2.69	2.02	0.87	1.71	0.87	-	0.87	-	-	0.11	-	-	-	-	-	-	-	-
15	5.21	5.21	5.21	3.40	2.96	2.19	2.19	1.52	0.38	1.21	0.38	-	0.38	-	-	-	-	-	-	-	-	-	-	-
16	4.86	4.86	4.86	2.95	2.50	1.74	1.74	1.08	-	0.78	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	4.50	4.50	4.50	2.54	2.10	1.35	1.35	0.70	-	0.41	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	4.16	4.16	4.16	2.17	1.74	1.01	1.01	0.38	-	0.10	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.82	3.82	3.82	1.83	1.42	0.71	0.71	0.11	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.49	3.49	3.49	1.54	1.14	0.46	0.46	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

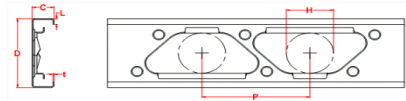


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 2" flange 54 mils - Section Identification: 400D200-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	13.03	13.03	13.03	12.36	12.14	11.71	11.71	11.30	10.50	11.10	10.50	9.37	10.50	9.74	8.31	9.92	9.01	7.31	9.37	8.31	6.35	8.83	7.63	5.44
9	12.49	12.49	12.49	11.59	11.31	10.77	10.77	10.25	9.28	10.00	9.28	7.94	9.28	8.37	6.72	8.59	7.52	5.59	7.94	6.72	4.53	7.32	5.96	3.53
10	11.69	11.69	11.69	10.57	10.23	9.60	9.60	9.01	7.91	8.72	7.91	6.44	7.91	6.91	5.13	7.15	5.99	3.93	6.44	5.13	2.83	5.77	4.32	1.79
11	10.86	10.86	10.86	9.54	9.15	8.43	8.43	7.78	6.60	7.47	6.60	5.06	6.60	5.55	3.71	5.80	4.59	2.49	5.06	3.71	1.37	4.37	2.88	0.34
12	10.02	10.02	10.02	8.50	8.07	7.30	7.30	6.61	5.40	6.29	5.40	3.84	5.40	4.33	2.48	4.58	3.37	1.28	3.84	2.48	0.17	3.14	1.67	-
13	9.19	9.19	9.19	7.49	7.05	6.25	6.25	5.55	4.33	5.22	4.33	2.79	4.33	3.27	1.46	3.52	2.32	0.28	2.79	1.46	-	2.10	0.66	-
14	8.36	8.36	8.36	6.55	6.09	5.30	5.30	4.60	3.40	4.28	3.40	1.90	3.40	2.37	0.62	2.62	1.45	-	1.90	0.62	-	1.24	-	-
15	7.56	7.56	7.56	5.68	5.23	4.45	4.45	3.77	2.61	3.46	2.61	1.17	2.61	1.62	-	1.86	0.74	-	1.17	-	-	0.53	-	-
16	6.77	6.77	6.77	4.89	4.46	3.71	3.71	3.06	1.95	2.76	1.95	0.57	1.95	1.00	-	1.22	0.16	-	0.57	-	-	-	-	-
17	6.09	6.09	6.09	4.23	3.81	3.08	3.08	2.46	1.40	2.17	1.40	0.09	1.40	0.50	-	0.71	-	-	0.09	-	-	-	-	-
18	5.51	5.51	5.51	3.66	3.26	2.56	2.56	1.96	0.95	1.69	0.95	-	0.95	0.09	-	0.29	-	-	-	-	-	-	-	-
19	5.01	5.01	5.01	3.19	2.79	2.12	2.12	1.55	0.58	1.29	0.58	-	0.58	-	-	-	-	-	-	-	-	-	-	-
20	4.57	4.57	4.57	2.78	2.40	1.75	1.75	1.20	0.27	0.95	0.27	-	0.27	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



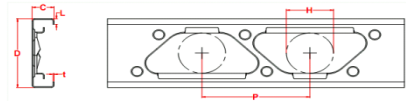
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 2" flange 68 mils - Section Identification: 400D200-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	17.56	17.56	17.56	16.89	16.68	16.25	16.25	15.84	15.04	15.64	15.04	13.90	15.04	14.27	12.82	14.46	13.53	11.80	13.90	12.82	10.82	13.35	12.13	9.88
9	16.53	16.53	16.53	15.67	15.39	14.85	14.85	14.34	13.36	14.09	13.36	12.02	13.36	12.46	10.78	12.68	11.60	9.61	12.02	10.78	8.52	11.39	9.99	7.47
10	15.46	15.46	15.46	14.36	14.02	13.39	13.39	12.78	11.66	12.49	11.66	10.15	11.66	10.64	8.79	10.88	9.69	7.55	10.15	8.79	6.39	9.46	7.95	5.30
11	14.36	14.36	14.36	13.02	12.63	11.89	11.89	11.22	9.99	10.90	9.99	8.39	9.99	8.90	6.98	9.16	7.90	5.70	8.39	6.98	4.52	7.66	6.11	3.42
12	13.23	13.23	13.23	11.67	11.23	10.43	10.43	9.71	8.43	9.37	8.43	6.80	8.43	7.31	5.37	7.58	6.30	4.09	6.80	5.37	2.92	6.06	4.51	1.84
13	12.11	12.11	12.11	10.35	9.88	9.05	9.05	8.31	7.02	7.97	7.02	5.40	7.02	5.91	3.99	6.18	4.91	2.74	5.40	3.99	1.60	4.67	3.15	0.56
14	11.00	11.00	11.00	9.10	8.62	7.78	7.78	7.05	5.78	6.71	5.78	4.19	5.78	4.69	2.83	4.95	3.72	1.62	4.19	2.83	0.53	3.49	2.01	-
15	9.91	9.91	9.91	7.94	7.47	6.64	6.64	5.93	4.70	5.60	4.70	3.17	4.70	3.65	1.87	3.90	2.72	0.72	3.17	1.87	-	2.50	1.09	-
16	8.81	8.81	8.81	6.86	6.40	5.61	5.61	4.93	3.76	4.62	3.76	2.31	3.76	2.77	1.08	3.00	1.88	-	2.31	1.08	-	1.67	0.34	-
17	7.80	7.80	7.80	5.91	5.48	4.73	4.73	4.08	2.98	3.79	2.98	1.61	2.98	2.04	0.44	2.26	1.20	-	1.61	0.44	-	1.01	-	-
18	6.96	6.96	6.96	5.13	4.71	4.00	4.00	3.38	2.34	3.10	2.34	1.04	2.34	1.45	-	1.66	0.66	-	1.04	-	-	0.47	-	-
19	6.25	6.25	6.25	4.47	4.07	3.39	3.39	2.81	1.81	2.54	1.81	0.58	1.81	0.96	-	1.16	0.22	-	0.58	-	-	0.04	-	-
20	5.64	5.64	5.64	3.91	3.53	2.88	2.88	2.32	1.38	2.07	1.38	0.20	1.38	0.57	-	0.76	-	-	0.20	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

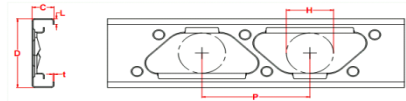


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 2" flange 33 mils - Section Identification: 600D200-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.36	5.36	5.36	5.06	4.96	4.76	4.76	4.56	4.17	4.47	4.17	3.59	4.17	3.79	3.03	3.88	3.41	2.47	3.59	3.03	1.92	3.31	2.65	1.38
9	5.28	5.28	5.28	4.90	4.78	4.52	4.52	4.27	3.78	4.15	3.78	3.07	3.78	3.30	2.37	3.42	2.83	1.70	3.07	2.37	1.03	2.72	1.92	0.38
10	5.20	5.20	5.20	4.73	4.57	4.26	4.26	3.96	3.36	3.81	3.36	2.51	3.36	2.79	1.68	2.93	2.23	0.89	2.51	1.68	0.12	2.09	1.15	-
11	5.11	5.11	5.11	4.53	4.35	3.98	3.98	3.62	2.92	3.44	2.92	1.92	2.92	2.25	0.98	2.41	1.60	0.08	1.92	0.98	-	1.44	0.37	-
12	5.01	5.01	5.01	4.33	4.10	3.67	3.67	3.25	2.45	3.05	2.45	1.33	2.45	1.70	0.28	1.88	0.98	-	1.33	0.28	-	0.80	-	-
13	4.91	4.91	4.91	4.10	3.85	3.35	3.35	2.88	1.98	2.65	1.98	0.75	1.98	1.14	-	1.35	0.36	-	0.75	-	-	0.16	-	-
14	4.80	4.80	4.80	3.87	3.58	3.02	3.02	2.50	1.51	2.24	1.51	0.18	1.51	0.61	-	0.83	-	-	0.18	-	-	-	-	-
15	4.69	4.69	4.69	3.62	3.30	2.69	2.69	2.11	1.06	1.84	1.06	-	1.06	0.10	-	0.33	-	-	-	-	-	-	-	-
16	4.57	4.57	4.57	3.37	3.01	2.35	2.35	1.74	0.62	1.44	0.62	-	0.62	-	-	-	-	-	-	-	-	-	-	-
17	4.44	4.44	4.44	3.11	2.73	2.02	2.02	1.37	0.21	1.07	0.21	-	0.21	-	-	-	-	-	-	-	-	-	-	-
18	4.30	4.30	4.30	2.85	2.44	1.70	1.70	1.02	-	0.71	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	4.16	4.16	4.16	2.59	2.16	1.39	1.39	0.70	-	0.38	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.01	4.01	4.01	2.33	1.89	1.10	1.10	0.40	-	0.08	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

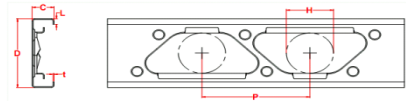


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 2" flange 43 mils - Section Identification: 600D200-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	8.18	8.18	8.18	7.84	7.74	7.52	7.52	7.30	6.86	7.19	6.86	6.22	6.86	6.43	5.59	6.54	6.01	4.96	6.22	5.59	4.35	5.90	5.17	3.75
9	8.05	8.05	8.05	7.63	7.49	7.20	7.20	6.93	6.38	6.79	6.38	5.58	6.38	5.85	4.80	5.98	5.32	4.04	5.58	4.80	3.29	5.19	4.29	2.56
10	7.91	7.91	7.91	7.38	7.21	6.86	6.86	6.52	5.86	6.36	5.86	4.89	5.86	5.21	3.96	5.37	4.58	3.06	4.89	3.96	2.19	4.43	3.36	1.35
11	7.76	7.76	7.76	7.11	6.90	6.49	6.49	6.08	5.30	5.88	5.30	4.17	5.30	4.54	3.10	4.73	3.81	2.07	4.17	3.10	1.09	3.63	2.41	0.14
12	7.60	7.60	7.60	6.82	6.57	6.09	6.09	5.61	4.71	5.38	4.71	3.42	4.71	3.84	2.22	4.05	3.01	1.09	3.42	2.22	0.01	2.81	1.46	-
13	7.40	7.40	7.40	6.49	6.21	5.65	5.65	5.11	4.09	4.85	4.09	2.67	4.09	3.13	1.36	3.36	2.22	0.14	2.67	1.36	-	2.00	0.54	-
14	7.19	7.19	7.19	6.14	5.81	5.18	5.18	4.59	3.46	4.30	3.46	1.94	3.46	2.43	0.55	2.68	1.46	-	1.94	0.55	-	1.22	-	-
15	6.97	6.97	6.97	5.78	5.41	4.71	4.71	4.06	2.86	3.75	2.86	1.24	2.86	1.76	-	2.02	0.74	-	1.24	-	-	0.50	-	-
16	6.74	6.74	6.74	5.40	5.00	4.25	4.25	3.55	2.27	3.21	2.27	0.59	2.27	1.12	-	1.40	0.07	-	0.59	-	-	-	-	-
17	6.51	6.51	6.51	5.02	4.59	3.78	3.78	3.05	1.72	2.70	1.72	-	1.72	0.54	-	0.82	-	-	-	-	-	-	-	-
18	6.27	6.27	6.27	4.64	4.18	3.33	3.33	2.57	1.21	2.21	1.21	-	1.21	0.01	-	0.30	-	-	-	-	-	-	-	-
19	6.03	6.03	6.03	4.27	3.79	2.91	2.91	2.12	0.74	1.75	0.74	-	0.74	-	-	-	-	-	-	-	-	-	-	-
20	5.79	5.79	5.79	3.91	3.40	2.50	2.50	1.70	0.31	1.33	0.31	-	0.31	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



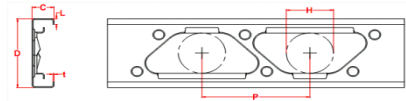
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 2" flange 54 mils - Section Identification: 600D200-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	14.28	14.28	14.28	13.95	13.83	13.61	13.61	13.37	12.92	13.26	12.92	12.25	12.92	12.47	11.59	12.58	12.03	10.94	12.25	11.59	10.30	11.92	11.16	9.67
9	14.09	14.09	14.09	13.65	13.49	13.20	13.20	12.91	12.33	12.76	12.33	11.49	12.33	11.77	10.66	11.91	11.21	9.85	11.49	10.66	9.06	11.07	10.12	8.28
10	13.88	13.88	13.88	13.31	13.12	12.75	12.75	12.39	11.68	12.21	11.68	10.64	11.68	10.98	9.63	11.15	10.30	8.66	10.64	9.63	7.72	10.13	8.98	6.80
11	13.55	13.55	13.55	12.85	12.62	12.17	12.17	11.73	10.88	11.52	10.88	9.65	10.88	10.05	8.48	10.26	9.25	7.36	9.65	8.48	6.29	9.06	7.73	5.25
12	13.21	13.21	13.21	12.36	12.09	11.55	11.55	11.03	10.03	10.78	10.03	8.62	10.03	9.08	7.30	9.31	8.17	6.05	8.62	7.30	4.86	7.95	6.46	3.73
13	12.82	12.82	12.82	11.81	11.49	10.86	10.86	10.27	9.13	9.97	9.13	7.56	9.13	8.07	6.11	8.33	7.06	4.76	7.56	6.11	3.49	6.82	5.20	2.28
14	12.27	12.27	12.27	11.10	10.74	10.04	10.04	9.37	8.13	9.05	8.13	6.44	8.13	6.99	4.91	7.27	5.92	3.50	6.44	4.91	2.18	5.66	3.96	0.94
15	11.71	11.71	11.71	10.38	9.97	9.20	9.20	8.48	7.17	8.14	7.17	5.39	7.17	5.95	3.80	6.24	4.84	2.35	5.39	3.80	1.00	4.57	2.82	-
16	11.13	11.13	11.13	9.65	9.21	8.39	8.39	7.63	6.24	7.26	6.24	4.40	6.24	4.98	2.78	5.29	3.84	1.31	4.40	2.78	-	3.57	1.79	-
17	10.56	10.56	10.56	8.93	8.46	7.59	7.59	6.79	5.36	6.42	5.36	3.50	5.36	4.08	1.87	4.40	2.93	0.40	3.50	1.87	-	2.66	0.87	-
18	9.98	9.98	9.98	8.22	7.73	6.82	6.82	6.00	4.55	5.62	4.55	2.68	4.55	3.27	1.06	3.58	2.12	-	2.68	1.06	-	1.84	0.07	-
19	9.40	9.40	9.40	7.53	7.02	6.10	6.10	5.27	3.81	4.88	3.81	1.95	3.81	2.53	0.35	2.84	1.39	-	1.95	0.35	-	1.12	-	-
20	8.83	8.83	8.83	6.87	6.35	5.42	5.42	4.59	3.14	4.20	3.14	1.31	3.14	1.88	-	2.18	0.76	-	1.31	-	-	0.50	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



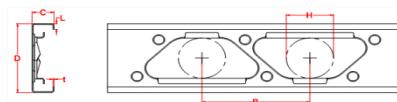
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 2" flange 68 mils - Section Identification: 600D200-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	20.42	20.42	20.42	20.08	19.96	19.73	19.73	19.48	19.02	19.37	19.02	18.34	19.02	18.57	17.66	18.68	18.11	16.99	18.34	17.66	16.33	18.00	17.21	15.68
9	19.86	19.86	19.86	19.43	19.26	18.96	18.96	18.67	18.08	18.52	18.08	17.22	18.08	17.50	16.37	17.65	16.93	15.54	17.22	16.37	14.72	16.79	15.81	13.92
10	19.26	19.26	19.26	18.69	18.51	18.14	18.14	17.77	17.05	17.59	17.05	16.00	17.05	16.34	14.97	16.52	15.65	13.98	16.00	14.97	13.01	15.48	14.31	12.07
11	18.62	18.62	18.62	17.92	17.69	17.24	17.24	16.80	15.94	16.58	15.94	14.69	15.94	15.10	13.50	15.31	14.29	12.35	14.69	13.50	11.24	14.09	12.73	10.17
12	17.93	17.93	17.93	17.10	16.82	16.29	16.29	15.77	14.76	15.51	14.76	13.33	14.76	13.80	11.98	14.04	12.87	10.70	13.33	11.98	9.47	12.65	11.12	8.30
13	17.22	17.22	17.22	16.23	15.91	15.29	15.29	14.69	13.55	14.40	13.55	11.95	13.55	12.47	10.47	12.73	11.44	9.07	11.95	10.47	7.76	11.20	9.53	6.51
14	16.49	16.49	16.49	15.33	14.96	14.26	14.26	13.59	12.32	13.26	12.32	10.58	12.32	11.14	8.99	11.43	10.03	7.52	10.58	8.99	6.14	9.77	8.00	4.84
15	15.73	15.73	15.73	14.40	13.99	13.21	13.21	12.48	11.11	12.13	11.11	9.27	11.11	9.86	7.60	10.16	8.69	6.06	9.27	7.60	4.65	8.41	6.56	3.32
16	14.97	14.97	14.97	13.46	13.01	12.17	12.17	11.38	9.93	11.00	9.93	8.01	9.93	8.62	6.30	8.94	7.42	4.73	8.01	6.30	3.30	7.13	5.23	1.95
17	14.19	14.19	14.19	12.53	12.04	11.14	11.14	10.30	8.80	9.91	8.80	6.83	8.80	7.46	5.10	7.78	6.23	3.53	6.83	5.10	2.09	5.94	4.03	0.76
18	13.41	13.41	13.41	11.60	11.08	10.13	10.13	9.27	7.74	8.87	7.74	5.75	7.74	6.38	4.01	6.70	5.15	2.45	5.75	4.01	1.03	4.86	2.96	-
19	12.64	12.64	12.64	10.69	10.15	9.18	9.18	8.30	6.75	7.89	6.75	4.77	6.75	5.39	3.04	5.72	4.17	1.51	4.77	3.04	0.10	3.88	2.00	-
20	11.87	11.87	11.87	9.82	9.26	8.27	8.27	7.39	5.84	6.98	5.84	3.88	5.84	4.50	2.19	4.82	3.29	0.68	3.88	2.19	-	3.00	1.17	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



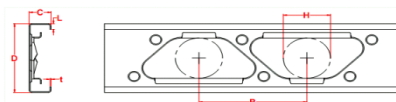
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 2" flange 33 mils - Section Identification: 800D200-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.51	5.51	5.51	5.28	5.20	5.05	5.05	4.90	4.60	4.83	4.60	4.16	4.60	4.31	3.71	4.38	4.01	3.27	4.16	3.71	2.83	3.93	3.42	2.40
9	5.46	5.46	5.46	5.17	5.08	4.89	4.89	4.70	4.32	4.60	4.32	3.76	4.32	3.95	3.20	4.04	3.57	2.65	3.76	3.20	2.11	3.48	2.84	1.57
10	5.41	5.41	5.41	5.06	4.94	4.70	4.70	4.47	4.01	4.35	4.01	3.32	4.01	3.55	2.65	3.66	3.10	1.98	3.32	2.65	1.33	2.98	2.20	0.69
11	5.36	5.36	5.36	4.93	4.78	4.50	4.50	4.22	3.67	4.08	3.67	2.85	3.67	3.12	2.05	3.26	2.58	1.27	2.85	2.05	0.51	2.45	1.53	-
12	5.29	5.29	5.29	4.78	4.62	4.28	4.28	3.94	3.29	3.78	3.29	2.35	3.29	2.66	1.43	2.82	2.04	0.54	2.35	1.43	-	1.88	0.83	-
13	5.23	5.23	5.23	4.63	4.43	4.04	4.04	3.65	2.91	3.47	2.91	1.83	2.91	2.18	0.79	2.36	1.48	-	1.83	0.79	-	1.30	0.12	-
14	5.16	5.16	5.16	4.46	4.23	3.79	3.79	3.35	2.50	3.13	2.50	1.29	2.50	1.69	0.14	1.89	0.90	-	1.29	0.14	-	0.71	-	-
15	5.09	5.09	5.09	4.29	4.03	3.52	3.52	3.03	2.09	2.79	2.09	0.75	2.09	1.19	-	1.41	0.33	-	0.75	-	-	0.12	-	-
16	5.01	5.01	5.01	4.10	3.81	3.25	3.25	2.70	1.66	2.44	1.66	0.22	1.66	0.69	-	0.93	-	-	0.22	-	-	-	-	-
17	4.93	4.93	4.93	3.91	3.59	2.96	2.96	2.37	1.24	2.08	1.24	-	1.24	0.20	-	0.45	-	-	-	-	-	-	-	-
18	4.84	4.84	4.84	3.71	3.35	2.68	2.68	2.03	0.83	1.72	0.83	-	0.83	-	-	-	-	-	-	-	-	-	-	-
19	4.76	4.76	4.76	3.50	3.12	2.39	2.39	1.70	0.43	1.37	0.43	-	0.43	-	-	-	-	-	-	-	-	-	-	-
20	4.66	4.66	4.66	3.29	2.88	2.10	2.10	1.37	0.04	1.02	0.04	-	0.04	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



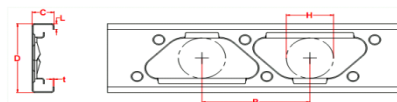
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 2" flange 43 mils - Section Identification: 800D200-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	8.44	8.44	8.44	8.22	8.15	8.00	8.00	7.85	7.56	7.78	7.56	7.12	7.56	7.27	6.69	7.34	6.98	6.26	7.12	6.69	5.82	6.91	6.40	5.40
9	8.37	8.37	8.37	8.09	7.99	7.81	7.81	7.62	7.25	7.53	7.25	6.70	7.25	6.88	6.15	6.98	6.52	5.61	6.70	6.15	5.07	6.43	5.79	4.54
10	8.28	8.28	8.28	7.94	7.82	7.59	7.59	7.36	6.91	7.25	6.91	6.23	6.91	6.46	5.56	6.57	6.01	4.90	6.23	5.56	4.25	5.90	5.12	3.61
11	8.19	8.19	8.19	7.77	7.63	7.35	7.35	7.08	6.52	6.94	6.52	5.71	6.52	5.98	4.92	6.11	5.44	4.13	5.71	4.92	3.37	5.31	4.39	2.62
12	8.09	8.09	8.09	7.59	7.42	7.08	7.08	6.76	6.11	6.59	6.11	5.16	6.11	5.47	4.23	5.63	4.85	3.33	5.16	4.23	2.46	4.69	3.63	1.59
13	7.98	7.98	7.98	7.39	7.19	6.80	6.80	6.42	5.67	6.23	5.67	4.58	5.67	4.94	3.52	5.12	4.22	2.50	4.58	3.52	1.52	4.05	2.84	0.55
14	7.86	7.86	7.86	7.17	6.94	6.50	6.50	6.06	5.20	5.84	5.20	3.97	5.20	4.38	2.79	4.58	3.57	1.67	3.97	2.79	0.57	3.38	2.03	-
15	7.74	7.74	7.74	6.94	6.68	6.18	6.18	5.68	4.72	5.44	4.72	3.35	4.72	3.80	2.06	4.03	2.92	0.83	3.35	2.06	-	2.70	1.24	-
16	7.60	7.60	7.60	6.70	6.41	5.84	5.84	5.29	4.22	5.02	4.22	2.73	4.22	3.22	1.34	3.46	2.25	0.01	2.73	1.34	-	2.02	0.44	-
17	7.45	7.45	7.45	6.43	6.11	5.48	5.48	4.87	3.71	4.57	3.71	2.11	3.71	2.63	0.62	2.89	1.60	-	2.11	0.62	-	1.35	-	-
18	7.28	7.28	7.28	6.15	5.79	5.10	5.10	4.45	3.21	4.13	3.21	1.50	3.21	2.05	-	2.33	0.97	-	1.50	-	-	0.70	-	-
19	7.11	7.11	7.11	5.86	5.48	4.73	4.73	4.02	2.70	3.68	2.70	0.92	2.70	1.49	-	1.78	0.36	-	0.92	-	-	0.08	-	-
20	6.94	6.94	6.94	5.57	5.15	4.35	4.35	3.60	2.22	3.24	2.22	0.36	2.22	0.95	-	1.26	-	-	0.36	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

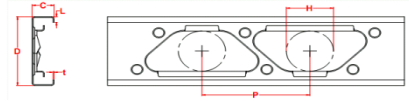
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 2" flange 54 mils - Section Identification: 800D200-54

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	14.75	14.75	14.75	14.53	14.45	14.30	14.30	14.15	13.86	14.08	13.86	13.41	13.86	13.56	12.97	13.63	13.26	12.53	13.41	12.97	12.09	13.19	12.68	11.65
9	14.62	14.62	14.62	14.34	14.24	14.05	14.05	13.87	13.49	13.77	13.49	12.93	13.49	13.11	12.37	13.21	12.74	11.81	12.93	12.37	11.24	12.64	11.99	10.69
10	14.49	14.49	14.49	14.13	14.01	13.78	13.78	13.54	13.08	13.43	13.08	12.36	13.08	12.60	11.67	12.71	12.13	10.99	12.36	11.67	10.31	12.02	11.21	9.64
11	14.34	14.34	14.34	13.91	13.76	13.48	13.48	13.18	12.60	13.03	12.60	11.76	12.60	12.04	10.92	12.18	11.48	10.10	11.76	10.92	9.30	11.34	10.37	8.50
12	14.18	14.18	14.18	13.66	13.48	13.13	13.13	12.78	12.10	12.61	12.10	11.09	12.10	11.43	10.11	11.59	10.77	9.16	11.09	10.11	8.22	10.60	9.47	7.30
13	14.02	14.02	14.02	13.38	13.18	12.76	12.76	12.36	11.55	12.15	11.55	10.39	11.55	10.77	9.25	10.97	10.00	8.16	10.39	9.25	7.09	9.82	8.52	6.05
14	13.76	13.76	13.76	13.02	12.78	12.30	12.30	11.83	10.91	11.60	10.91	9.58	10.91	10.02	8.31	10.24	9.15	7.08	9.58	8.31	5.90	8.94	7.49	4.76
15	13.50	13.50	13.50	12.64	12.36	11.81	11.81	11.28	10.24	11.01	10.24	8.75	10.24	9.24	7.34	9.49	8.28	6.00	8.75	7.34	4.71	8.04	6.44	3.48
16	13.23	13.23	13.23	12.25	11.93	11.31	11.31	10.71	9.55	10.41	9.55	7.91	9.55	8.45	6.38	8.72	7.39	4.93	7.91	6.38	3.55	7.13	5.40	2.24
17	12.89	12.89	12.89	11.78	11.43	10.74	10.74	10.07	8.80	9.74	8.80	7.04	8.80	7.61	5.40	7.90	6.48	3.87	7.04	5.40	2.42	6.20	4.37	1.06
18	12.47	12.47	12.47	11.23	10.84	10.09	10.09	9.37	8.01	9.02	8.01	6.14	8.01	6.75	4.43	7.05	5.56	2.84	6.14	4.43	1.35	5.27	3.36	-
19	12.03	12.03	12.03	10.67	10.25	9.44	9.44	8.67	7.23	8.30	7.23	5.28	7.23	5.91	3.51	6.23	4.67	1.88	5.28	3.51	0.36	4.38	2.41	-
20	11.59	11.59	11.59	10.11	9.66	8.79	8.79	7.98	6.48	7.59	6.48	4.46	6.48	5.11	2.65	5.44	3.84	1.00	4.46	2.65	-	3.54	1.53	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



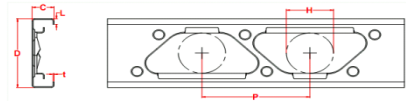
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 2" flange 68 mils - Section Identification: 800D200-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	21.35	21.35	21.35	21.12	21.05	20.90	20.90	20.75	20.44	20.67	20.44	20.00	20.44	20.14	19.55	20.22	19.85	19.10	20.00	19.55	18.66	19.77	19.25	18.21
9	21.12	21.12	21.12	20.83	20.73	20.54	20.54	20.35	19.97	20.26	19.97	19.40	19.97	19.59	18.83	19.68	19.21	18.25	19.40	18.83	17.68	19.12	18.46	17.12
10	20.87	20.87	20.87	20.52	20.40	20.16	20.16	19.92	19.45	19.80	19.45	18.71	19.45	18.95	18.01	19.07	18.48	17.31	18.71	18.01	16.62	18.36	17.54	15.93
11	20.46	20.46	20.46	20.03	19.88	19.59	19.59	19.28	18.71	19.14	18.71	17.85	18.71	18.13	17.00	18.27	17.56	16.17	17.85	17.00	15.34	17.42	16.44	14.53
12	20.02	20.02	20.02	19.50	19.33	18.96	18.96	18.61	17.93	18.44	17.93	16.91	17.93	17.25	15.92	17.42	16.58	14.95	16.91	15.92	13.99	16.41	15.27	13.05
13	19.54	19.54	19.54	18.92	18.71	18.30	18.30	17.89	17.09	17.69	17.09	15.92	17.09	16.31	14.78	16.50	15.54	13.67	15.92	14.78	12.58	15.35	14.04	11.53
14	19.05	19.05	19.05	18.32	18.08	17.60	17.60	17.13	16.22	16.90	16.22	14.88	16.22	15.32	13.60	15.54	14.45	12.35	14.88	13.60	11.15	14.24	12.76	9.98
15	18.53	18.53	18.53	17.69	17.41	16.87	16.87	16.34	15.30	16.08	15.30	13.81	15.30	14.30	12.39	14.55	13.33	11.02	13.81	12.39	9.71	13.09	11.47	8.44
16	17.99	17.99	17.99	17.03	16.72	16.11	16.11	15.51	14.36	15.22	14.36	12.72	14.36	13.26	11.17	13.53	12.19	9.70	12.72	11.17	8.29	11.93	10.18	6.94
17	17.43	17.43	17.43	16.35	16.01	15.33	15.33	14.67	13.40	14.34	13.40	11.62	13.40	12.20	9.96	12.50	11.06	8.40	11.62	9.96	6.91	10.78	8.91	5.50
18	16.87	16.87	16.87	15.66	15.27	14.52	14.52	13.80	12.43	13.45	12.43	10.53	12.43	11.15	8.78	11.46	9.94	7.14	10.53	8.78	5.60	9.64	7.68	4.13
19	16.28	16.28	16.28	14.95	14.52	13.71	13.71	12.93	11.47	12.56	11.47	9.47	11.47	10.11	7.64	10.45	8.84	5.95	9.47	7.64	4.36	8.54	6.50	2.86
20	15.70	15.70	15.70	14.23	13.77	12.89	12.89	12.06	10.52	11.67	10.52	8.44	10.52	9.11	6.55	9.45	7.79	4.82	8.44	6.55	3.21	7.48	5.38	1.69

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

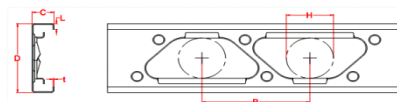


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 2-1/2" flange 33 mils - Section Identification: 362D250-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.15	5.15	5.15	4.56	4.36	3.99	3.99	3.64	2.95	3.46	2.95	2.00	2.95	2.31	1.11	2.47	1.70	0.28	2.00	1.11	-	1.55	0.55	-
9	4.98	4.98	4.98	4.22	3.98	3.53	3.53	3.10	2.30	2.89	2.30	1.20	2.30	1.56	0.21	1.74	0.86	-	1.20	0.21	-	0.70	-	-
10	4.80	4.80	4.80	3.86	3.58	3.05	3.05	2.55	1.65	2.32	1.65	0.46	1.65	0.84	-	1.04	0.10	-	0.46	-	-	-	-	-
11	4.59	4.59	4.59	3.47	3.15	2.56	2.56	2.02	1.05	1.76	1.05	-	1.05	0.20	-	0.40	-	-	-	-	-	-	-	-
12	4.38	4.38	4.38	3.08	2.73	2.08	2.08	1.51	0.51	1.25	0.51	-	0.51	-	-	-	-	-	-	-	-	-	-	-
13	4.15	4.15	4.15	2.69	2.32	1.65	1.65	1.06	0.04	0.78	0.04	-	0.04	-	-	-	-	-	-	-	-	-	-	-
14	3.92	3.92	3.92	2.32	1.93	1.25	1.25	0.65	-	0.38	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.68	3.68	3.68	1.98	1.58	0.89	0.89	0.30	-	0.03	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.44	3.44	3.44	1.66	1.27	0.59	0.59	0.01	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.20	3.20	3.20	1.38	0.99	0.32	0.32	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.96	2.96	2.96	1.12	0.74	0.10	0.10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.73	2.73	2.73	0.90	0.53	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.51	2.51	2.51	0.71	0.35	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



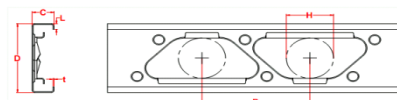
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 2-1/2" flange 43 mills - Section Identification: 362D250-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	7.61	7.61	7.61	6.94	6.73	6.31	6.31	5.91	5.13	5.71	5.13	4.05	5.13	4.40	3.03	4.58	3.70	2.08	4.05	3.03	1.17	3.53	2.39	0.31
9	7.36	7.36	7.36	6.50	6.23	5.71	5.71	5.22	4.30	4.98	4.30	3.03	4.30	3.44	1.89	3.65	2.64	0.83	3.03	1.89	-	2.45	1.17	-
10	7.10	7.10	7.10	6.01	5.69	5.07	5.07	4.50	3.45	4.22	3.45	2.06	3.45	2.50	0.82	2.73	1.63	-	2.06	0.82	-	1.43	0.06	-
11	6.81	6.81	6.81	5.49	5.12	4.42	4.42	3.78	2.64	3.48	2.64	1.17	2.64	1.64	-	1.88	0.72	-	1.17	-	-	0.51	-	-
12	6.51	6.51	6.51	4.95	4.53	3.77	3.77	3.09	1.90	2.78	1.90	0.38	1.90	0.86	-	1.11	-	-	0.38	-	-	-	-	-
13	6.19	6.19	6.19	4.41	3.96	3.16	3.16	2.45	1.23	2.13	1.23	-	1.23	0.19	-	0.44	-	-	-	-	-	-	-	-
14	5.83	5.83	5.83	3.87	3.40	2.58	2.58	1.87	0.65	1.54	0.65	-	0.65	-	-	-	-	-	-	-	-	-	-	-
15	5.40	5.40	5.40	3.33	2.86	2.05	2.05	1.35	0.16	1.03	0.16	-	0.16	-	-	-	-	-	-	-	-	-	-	-
16	4.97	4.97	4.97	2.84	2.38	1.58	1.58	0.90	-	0.59	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	4.55	4.55	4.55	2.40	1.95	1.18	1.18	0.53	-	0.23	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	4.13	4.13	4.13	2.02	1.58	0.84	0.84	0.21	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.75	3.75	3.75	1.68	1.27	0.56	0.56	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.42	3.42	3.42	1.40	1.00	0.32	0.32	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

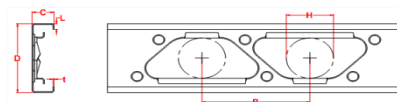
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 2-1/2" flange 54 mills - Section Identification: 362D250-54

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	12.46	12.46	12.46	11.76	11.53	11.09	11.09	10.67	9.85	10.46	9.85	8.70	9.85	9.07	7.62	9.26	8.33	6.61	8.70	7.62	5.65	8.15	6.94	4.73
9	11.83	11.83	11.83	10.92	10.63	10.08	10.08	9.56	8.58	9.30	8.58	7.25	8.58	7.68	6.02	7.90	6.82	4.89	7.25	6.02	3.84	6.61	5.26	2.85
10	11.16	11.16	11.16	10.01	9.66	9.02	9.02	8.41	7.30	8.12	7.30	5.83	7.30	6.30	4.52	6.54	5.38	3.33	5.83	4.52	2.23	5.16	3.71	1.20
11	10.61	10.61	10.61	9.19	8.78	8.03	8.03	7.35	6.14	7.03	6.14	4.58	6.14	5.07	3.20	5.33	4.10	1.98	4.58	3.20	0.86	3.87	2.38	-
12	10.03	10.03	10.03	8.31	7.86	7.04	7.04	6.31	5.05	5.98	5.05	3.44	5.05	3.95	2.06	4.21	2.96	0.83	3.44	2.06	-	2.73	1.22	-
13	9.27	9.27	9.27	7.34	6.86	6.02	6.02	5.28	4.01	4.94	4.01	2.43	4.01	2.92	1.07	3.18	1.95	-	2.43	1.07	-	1.72	0.25	-
14	8.41	8.41	8.41	6.37	5.88	5.05	5.05	4.33	3.09	4.00	3.09	1.56	3.09	2.04	0.25	2.29	1.10	-	1.56	0.25	-	0.88	-	-
15	7.45	7.45	7.45	5.42	4.96	4.16	4.16	3.47	2.30	3.16	2.30	0.84	2.30	1.29	-	1.53	0.41	-	0.84	-	-	0.20	-	-
16	6.63	6.63	6.63	4.63	4.19	3.42	3.42	2.77	1.65	2.47	1.65	0.28	1.65	0.71	-	0.93	-	-	0.28	-	-	-	-	-
17	5.94	5.94	5.94	3.98	3.55	2.82	2.82	2.19	1.13	1.91	1.13	-	1.13	0.24	-	0.45	-	-	-	-	-	-	-	-
18	5.30	5.30	5.30	3.40	3.00	2.30	2.30	1.71	0.71	1.44	0.71	-	0.71	-	-	0.06	-	-	-	-	-	-	-	-
19	4.76	4.76	4.76	2.92	2.54	1.88	1.88	1.31	0.36	1.06	0.36	-	0.36	-	-	-	-	-	-	-	-	-	-	-
20	4.29	4.29	4.29	2.52	2.15	1.52	1.52	0.98	0.08	0.74	0.08	-	0.08	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

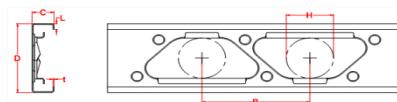
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 2-1/2" flange 68 mills - Section Identification: 362D250-68

Stud Height (ft)	Factored Wind Loads (psf)																										
	0			10			20			30			40			50			60			70					
	Stud Spacing (inches)																										
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24			
8	17.76	17.76	17.76	16.94	16.67	16.16	16.16	15.65	14.69	15.41	14.69	13.34	14.69	13.78	12.08	14.00	12.91	10.89	13.34	12.08	9.77	12.70	11.28	8.70			
9	16.72	16.72	16.72	15.63	15.29	14.63	14.63	14.03	12.88	13.73	12.88	11.30	12.88	11.81	9.88	12.07	10.81	8.56	11.30	9.88	7.33	10.58	8.99	6.17			
10	15.65	15.65	15.65	14.26	13.86	13.09	13.09	12.37	11.06	12.03	11.06	9.34	11.06	9.89	7.81	10.17	8.81	6.43	9.34	7.81	5.15	8.56	6.88	3.95			
11	14.61	14.61	14.61	12.91	12.43	11.55	11.55	10.76	9.36	10.39	9.36	7.55	9.36	8.12	5.98	8.42	7.01	4.57	7.55	5.98	3.27	6.74	5.02	2.08			
12	13.55	13.55	13.55	11.54	11.01	10.07	10.07	9.23	7.79	8.85	7.79	5.96	7.79	6.53	4.39	6.83	5.41	2.98	5.96	4.39	1.72	5.15	3.44	0.55			
13	12.20	12.20	12.20	10.02	9.48	8.53	8.53	7.71	6.29	7.33	6.29	4.52	6.29	5.07	3.00	5.36	3.99	1.66	4.52	3.00	0.44	3.73	2.09	-			
14	10.77	10.77	10.77	8.56	8.03	7.12	7.12	6.33	4.98	5.97	4.98	3.29	4.98	3.82	1.86	4.10	2.79	0.59	3.29	1.86	-	2.55	1.00	-			
15	9.40	9.40	9.40	7.26	6.76	5.89	5.89	5.15	3.88	4.81	3.88	2.30	3.88	2.79	0.95	3.05	1.82	-	2.30	0.95	-	1.60	0.15	-			
16	8.26	8.26	8.26	6.19	5.72	4.90	4.90	4.20	3.00	3.88	3.00	1.51	3.00	1.98	0.25	2.22	1.07	-	1.51	0.25	-	0.86	-	-			
17	7.32	7.32	7.32	5.32	4.87	4.09	4.09	3.42	2.29	3.12	2.29	0.89	2.29	1.32	-	1.56	0.47	-	0.89	-	-	0.27	-	-			
18	6.53	6.53	6.53	4.59	4.16	3.42	3.42	2.79	1.72	2.50	1.72	0.39	1.72	0.80	-	1.02	-	-	0.39	-	-	-	-	-			
19	5.86	5.86	5.86	3.98	3.57	2.87	2.87	2.27	1.25	1.99	1.25	-	1.25	0.38	-	0.58	-	-	-	-	-	-	-	-			
20	5.29	5.29	5.29	3.47	3.08	2.41	2.41	1.83	0.86	1.57	0.86	-	0.86	0.03	-	0.23	-	-	-	-	-	-	-	-			

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



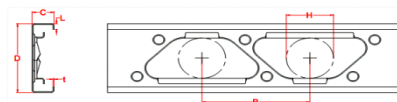
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 2-1/2" flange 33 mills - Section Identification: 400D250-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.28	5.28	5.28	4.77	4.60	4.27	4.27	3.95	3.33	3.79	3.33	2.44	3.33	2.73	1.61	2.88	2.16	0.82	2.44	1.61	0.06	2.02	1.07	-
9	5.14	5.14	5.14	4.48	4.27	3.86	3.86	3.47	2.72	3.27	2.72	1.68	2.72	2.02	0.73	2.19	1.35	-	1.68	0.73	-	1.19	0.12	-
10	4.98	4.98	4.98	4.16	3.91	3.42	3.42	2.96	2.10	2.74	2.10	0.94	2.10	1.31	-	1.51	0.58	-	0.94	-	-	0.40	-	-
11	4.81	4.81	4.81	3.82	3.53	2.97	2.97	2.45	1.50	2.20	1.50	0.25	1.50	0.65	-	0.86	-	-	0.25	-	-	-	-	-
12	4.63	4.63	4.63	3.47	3.13	2.51	2.51	1.95	0.94	1.68	0.94	-	0.94	0.04	-	0.26	-	-	-	-	-	-	-	-
13	4.43	4.43	4.43	3.10	2.73	2.07	2.07	1.48	0.43	1.20	0.43	-	0.43	-	-	-	-	-	-	-	-	-	-	-
14	4.22	4.22	4.22	2.74	2.35	1.66	1.66	1.04	-	0.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	4.01	4.01	4.01	2.40	1.99	1.28	1.28	0.66	-	0.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.80	3.80	3.80	2.07	1.66	0.94	0.94	0.32	-	0.03	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.58	3.58	3.58	1.77	1.35	0.64	0.64	0.02	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.36	3.36	3.36	1.49	1.08	0.37	0.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.14	3.14	3.14	1.24	0.84	0.15	0.15	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.92	2.92	2.92	1.02	0.62	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

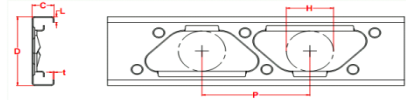


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 2-1/2" flange 43 mills - Section Identification: 400D250-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	7.82	7.82	7.82	7.24	7.05	6.69	6.69	6.33	5.63	6.15	5.63	4.63	5.63	4.96	3.68	5.12	4.31	2.78	4.63	3.68	1.92	4.15	3.08	1.09
9	7.61	7.61	7.61	6.87	6.63	6.17	6.17	5.72	4.87	5.50	4.87	3.68	4.87	4.07	2.58	4.26	3.30	1.55	3.68	2.58	0.57	3.12	1.88	-
10	7.38	7.38	7.38	6.45	6.16	5.60	5.60	5.07	4.08	4.81	4.08	2.73	4.08	3.16	1.51	3.39	2.31	0.38	2.73	1.51	-	2.11	0.75	-
11	7.14	7.14	7.14	5.99	5.65	5.00	5.00	4.39	3.29	4.10	3.29	1.82	3.29	2.29	0.52	2.53	1.37	-	1.82	0.52	-	1.15	-	-
12	6.87	6.87	6.87	5.51	5.11	4.39	4.39	3.73	2.53	3.41	2.53	0.99	2.53	1.48	-	1.73	0.52	-	0.99	-	-	0.29	-	-
13	6.60	6.60	6.60	5.01	4.58	3.79	3.79	3.08	1.83	2.75	1.83	0.24	1.83	0.75	-	1.01	-	-	0.24	-	-	-	-	-
14	6.31	6.31	6.31	4.51	4.04	3.21	3.21	2.48	1.20	2.14	1.20	-	1.20	0.10	-	0.37	-	-	-	-	-	-	-	-
15	6.01	6.01	6.01	4.02	3.53	2.67	2.67	1.93	0.64	1.58	0.64	-	0.64	-	-	-	-	-	-	-	-	-	-	-
16	5.62	5.62	5.62	3.51	3.02	2.16	2.16	1.42	0.16	1.08	0.16	-	0.16	-	-	-	-	-	-	-	-	-	-	-
17	5.23	5.23	5.23	3.04	2.55	1.70	1.70	0.98	-	0.65	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	4.84	4.84	4.84	2.61	2.13	1.31	1.31	0.60	-	0.28	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	4.46	4.46	4.46	2.23	1.76	0.96	0.96	0.28	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.08	4.08	4.08	1.89	1.44	0.66	0.66	0.01	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

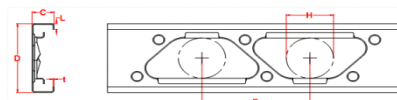


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 2-1/2" flange 54 mills - Section Identification: 400D250-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	12.97	12.97	12.97	12.37	12.17	11.79	11.79	11.41	10.68	11.23	10.68	9.64	10.68	9.98	8.64	10.15	9.30	7.69	9.64	8.64	6.77	9.13	8.00	5.89
9	12.43	12.43	12.43	11.65	11.40	10.92	10.92	10.45	9.55	10.22	9.55	8.29	9.55	8.70	7.13	8.91	7.90	6.03	8.29	7.13	5.00	7.70	6.39	4.01
10	11.85	11.85	11.85	10.87	10.56	9.97	9.97	9.41	8.36	9.14	8.36	6.93	8.36	7.39	5.64	7.63	6.49	4.45	6.93	5.64	3.33	6.27	4.83	2.28
11	11.24	11.24	11.24	10.03	9.67	8.99	8.99	8.35	7.18	8.05	7.18	5.64	7.18	6.13	4.25	6.39	5.16	3.00	5.64	4.25	1.84	4.93	3.41	0.77
12	10.74	10.74	10.74	9.28	8.86	8.08	8.08	7.37	6.11	7.04	6.11	4.47	6.11	4.99	3.03	5.26	3.97	1.75	4.47	3.03	0.57	3.73	2.16	-
13	10.24	10.24	10.24	8.50	8.03	7.18	7.18	6.42	5.10	6.07	5.10	3.41	5.10	3.94	1.95	4.22	2.91	0.66	3.41	1.95	-	2.65	1.08	-
14	9.54	9.54	9.54	7.60	7.10	6.22	6.22	5.45	4.12	5.10	4.12	2.44	4.12	2.97	1.01	3.24	1.94	-	2.44	1.01	-	1.70	0.14	-
15	8.82	8.82	8.82	6.72	6.21	5.33	5.33	4.56	3.25	4.21	3.25	1.61	3.25	2.12	0.21	2.39	1.12	-	1.61	0.21	-	0.89	-	-
16	7.93	7.93	7.93	5.82	5.32	4.47	4.47	3.73	2.47	3.39	2.47	0.90	2.47	1.39	-	1.64	0.43	-	0.90	-	-	0.21	-	-
17	7.12	7.12	7.12	5.03	4.55	3.73	3.73	3.02	1.82	2.70	1.82	0.33	1.82	0.79	-	1.03	-	-	0.33	-	-	-	-	-
18	6.43	6.43	6.43	4.36	3.90	3.11	3.11	2.43	1.28	2.13	1.28	-	1.28	0.30	-	0.54	-	-	-	-	-	-	-	-
19	5.84	5.84	5.84	3.80	3.35	2.59	2.59	1.94	0.84	1.65	0.84	-	0.84	-	-	0.13	-	-	-	-	-	-	-	-
20	5.32	5.32	5.32	3.32	2.89	2.16	2.16	1.53	0.48	1.25	0.48	-	0.48	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



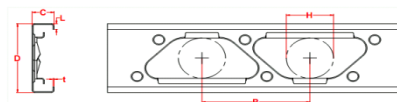
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 2-1/2" flange 68 mills - Section Identification: 400D250-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	18.51	18.51	18.51	17.81	17.59	17.14	17.14	16.70	15.86	16.49	15.86	14.64	15.86	15.04	13.48	15.24	14.25	12.37	14.64	13.48	11.31	14.05	12.74	10.29
9	17.76	17.76	17.76	16.84	16.54	15.97	15.97	15.41	14.35	15.14	14.35	12.87	14.35	13.35	11.50	13.60	12.40	10.21	12.87	11.50	9.00	12.17	10.63	7.85
10	16.80	16.80	16.80	15.63	15.26	14.55	14.55	13.90	12.67	13.58	12.67	10.99	12.67	11.53	9.47	11.80	10.46	8.07	10.99	9.47	6.75	10.21	8.52	5.53
11	15.83	15.83	15.83	14.37	13.94	13.13	13.13	12.37	10.99	12.01	10.99	9.17	10.99	9.75	7.56	10.05	8.62	6.10	9.17	7.56	4.74	8.35	6.57	3.48
12	14.88	14.88	14.88	13.12	12.62	11.71	11.71	10.88	9.41	10.49	9.41	7.51	9.41	8.11	5.85	8.42	6.94	4.37	7.51	5.85	3.00	6.66	4.85	1.74
13	13.97	13.97	13.97	11.90	11.35	10.36	10.36	9.49	7.96	9.08	7.96	6.03	7.96	6.64	4.37	6.96	5.45	2.88	6.03	4.37	1.53	5.17	3.36	0.29
14	12.73	12.73	12.73	10.49	9.92	8.92	8.92	8.05	6.54	7.65	6.54	4.66	6.54	5.25	3.04	5.56	4.09	1.60	4.66	3.04	0.30	3.82	2.06	-
15	11.52	11.52	11.52	9.18	8.61	7.63	7.63	6.77	5.31	6.38	5.31	3.49	5.31	4.06	1.93	4.36	2.95	0.56	3.49	1.93	-	2.69	1.00	-
16	10.33	10.33	10.33	7.97	7.42	6.47	6.47	5.65	4.25	5.27	4.25	2.51	4.25	3.06	1.03	3.34	1.99	-	2.51	1.03	-	1.74	0.14	-
17	9.15	9.15	9.15	6.87	6.34	5.44	5.44	4.66	3.34	4.31	3.34	1.69	3.34	2.21	0.30	2.48	1.21	-	1.69	0.30	-	0.97	-	-
18	8.17	8.17	8.17	5.95	5.45	4.59	4.59	3.85	2.60	3.52	2.60	1.04	2.60	1.52	-	1.78	0.58	-	1.04	-	-	0.36	-	-
19	7.33	7.33	7.33	5.18	4.70	3.88	3.88	3.18	1.99	2.86	1.99	0.51	1.99	0.97	-	1.21	0.07	-	0.51	-	-	-	-	-
20	6.61	6.61	6.61	4.53	4.07	3.29	3.29	2.62	1.48	2.31	1.48	0.07	1.48	0.51	-	0.74	-	-	0.07	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



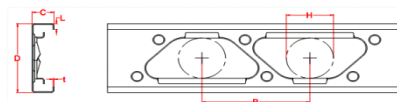
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 2-1/2" flange 33 mills - Section Identification: 600D250-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.65	5.65	5.65	5.35	5.25	5.05	5.05	4.85	4.46	4.75	4.46	3.88	4.46	4.07	3.30	4.17	3.68	2.74	3.88	3.30	2.18	3.59	2.92	1.63
9	5.57	5.57	5.57	5.19	5.07	4.82	4.82	4.56	4.07	4.44	4.07	3.34	4.07	3.58	2.64	3.70	3.11	1.95	3.34	2.64	1.27	2.99	2.18	0.61
10	5.49	5.49	5.49	5.02	4.86	4.55	4.55	4.25	3.65	4.09	3.65	2.78	3.65	3.06	1.94	3.21	2.49	1.12	2.78	1.94	0.33	2.35	1.39	-
11	5.41	5.41	5.41	4.83	4.64	4.27	4.27	3.90	3.20	3.72	3.20	2.18	3.20	2.51	1.21	2.68	1.85	0.29	2.18	1.21	-	1.69	0.59	-
12	5.31	5.31	5.31	4.62	4.40	3.96	3.96	3.54	2.72	3.33	2.72	1.57	2.72	1.95	0.49	2.14	1.20	-	1.57	0.49	-	1.02	-	-
13	5.21	5.21	5.21	4.40	4.14	3.64	3.64	3.16	2.24	2.92	2.24	0.96	2.24	1.38	-	1.59	0.56	-	0.96	-	-	0.36	-	-
14	5.10	5.10	5.10	4.17	3.87	3.31	3.31	2.77	1.75	2.51	1.75	0.37	1.75	0.82	-	1.04	-	-	0.37	-	-	-	-	-
15	4.99	4.99	4.99	3.92	3.59	2.96	2.96	2.37	1.28	2.09	1.28	-	1.28	0.28	-	0.52	-	-	-	-	-	-	-	-
16	4.87	4.87	4.87	3.67	3.30	2.62	2.62	1.98	0.82	1.68	0.82	-	0.82	-	-	0.02	-	-	-	-	-	-	-	-
17	4.75	4.75	4.75	3.41	3.01	2.28	2.28	1.60	0.38	1.28	0.38	-	0.38	-	-	-	-	-	-	-	-	-	-	-
18	4.61	4.61	4.61	3.14	2.72	1.94	1.94	1.23	-	0.90	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	4.47	4.47	4.47	2.87	2.43	1.62	1.62	0.89	-	0.55	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.33	4.33	4.33	2.61	2.14	1.31	1.31	0.56	-	0.22	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



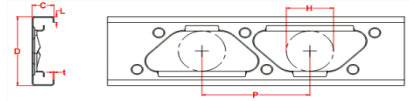
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 2-1/2" flange 43 mills - Section Identification: 600D250-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	8.40	8.40	8.40	8.08	7.97	7.76	7.76	7.55	7.13	7.44	7.13	6.51	7.13	6.71	5.88	6.82	6.29	5.28	6.51	5.88	4.68	6.19	5.48	4.09
9	8.29	8.29	8.29	7.89	7.75	7.48	7.48	7.21	6.68	7.07	6.68	5.90	6.68	6.15	5.13	6.28	5.64	4.38	5.90	5.13	3.65	5.51	4.63	2.93
10	8.18	8.18	8.18	7.67	7.50	7.16	7.16	6.83	6.18	6.67	6.18	5.24	6.18	5.55	4.32	5.71	4.93	3.42	5.24	4.32	2.56	4.77	3.72	1.72
11	8.05	8.05	8.05	7.43	7.22	6.82	6.82	6.42	5.65	6.23	5.65	4.53	5.65	4.90	3.46	5.08	4.17	2.43	4.53	3.46	1.45	3.99	2.77	0.49
12	7.92	7.92	7.92	7.17	6.92	6.44	6.44	5.98	5.08	5.75	5.08	3.80	5.08	4.22	2.59	4.43	3.39	1.45	3.80	2.59	0.34	3.18	1.82	-
13	7.77	7.77	7.77	6.88	6.60	6.04	6.04	5.51	4.49	5.25	4.49	3.05	4.49	3.52	1.72	3.75	2.60	0.47	3.05	1.72	-	2.37	0.88	-
14	7.62	7.62	7.62	6.58	6.25	5.62	5.62	5.02	3.88	4.73	3.88	2.31	3.88	2.82	0.88	3.08	1.82	-	2.31	0.88	-	1.58	-	-
15	7.45	7.45	7.45	6.26	5.89	5.18	5.18	4.52	3.27	4.19	3.27	1.59	3.27	2.13	0.07	2.41	1.07	-	1.59	0.07	-	0.81	-	-
16	7.28	7.28	7.28	5.93	5.51	4.74	4.74	4.01	2.68	3.66	2.68	0.90	2.68	1.47	-	1.76	0.36	-	0.90	-	-	0.09	-	-
17	7.10	7.10	7.10	5.58	5.13	4.28	4.28	3.51	2.10	3.14	2.10	0.26	2.10	0.84	-	1.15	-	-	0.26	-	-	-	-	-
18	6.91	6.91	6.91	5.22	4.73	3.84	3.84	3.02	1.56	2.63	1.56	-	1.56	0.26	-	0.58	-	-	-	-	-	-	-	-
19	6.72	6.72	6.72	4.86	4.34	3.40	3.40	2.55	1.05	2.15	1.05	-	1.05	-	-	0.04	-	-	-	-	-	-	-	-
20	6.52	6.52	6.52	4.51	3.96	2.97	2.97	2.10	0.57	1.70	0.57	-	0.57	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



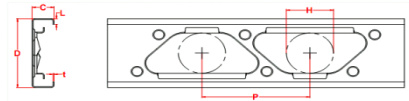
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 2-1/2" flange 54 mills - Section Identification: 600D250-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	14.39	14.39	14.39	14.07	13.97	13.75	13.75	13.54	13.11	13.44	13.11	12.48	13.11	12.69	11.85	12.79	12.27	11.23	12.48	11.85	10.62	12.16	11.44	10.02
9	14.12	14.12	14.12	13.71	13.58	13.29	13.29	13.02	12.48	12.88	12.48	11.69	12.48	11.95	10.90	12.08	11.42	10.14	11.69	10.90	9.38	11.29	10.39	8.64
10	13.82	13.82	13.82	13.30	13.13	12.79	12.79	12.45	11.79	12.28	11.79	10.82	11.79	11.14	9.87	11.30	10.50	8.95	10.82	9.87	8.05	10.34	9.26	7.18
11	13.49	13.49	13.49	12.85	12.65	12.23	12.23	11.83	11.03	11.63	11.03	9.88	11.03	10.26	8.78	10.45	9.51	7.71	9.88	8.78	6.68	9.32	8.06	5.68
12	13.14	13.14	13.14	12.37	12.12	11.63	11.63	11.16	10.23	10.92	10.23	8.90	10.23	9.33	7.64	9.55	8.47	6.44	8.90	7.64	5.30	8.26	6.84	4.19
13	12.77	12.77	12.77	11.86	11.57	11.00	11.00	10.44	9.38	10.17	9.38	7.89	9.38	8.38	6.50	8.62	7.42	5.19	7.89	6.50	3.94	7.19	5.62	2.76
14	12.37	12.37	12.37	11.31	10.98	10.32	10.32	9.70	8.52	9.40	8.52	6.88	8.52	7.41	5.38	7.68	6.37	3.98	6.88	5.38	2.66	6.11	4.43	1.41
15	11.96	11.96	11.96	10.74	10.36	9.63	9.63	8.94	7.64	8.60	7.64	5.89	7.64	6.45	4.30	6.74	5.34	2.83	5.89	4.30	1.46	5.08	3.31	0.17
16	11.53	11.53	11.53	10.15	9.72	8.92	8.92	8.16	6.78	7.80	6.78	4.93	6.78	5.52	3.28	5.83	4.36	1.77	4.93	3.28	0.37	4.09	2.26	-
17	11.19	11.19	11.19	9.61	9.14	8.28	8.28	7.47	6.00	7.08	6.00	4.06	6.00	4.68	2.36	4.99	3.47	0.82	4.06	2.36	-	3.19	1.31	-
18	10.86	10.86	10.86	9.09	8.58	7.63	7.63	6.78	5.24	6.37	5.24	3.25	5.24	3.87	1.51	4.20	2.64	-	3.25	1.51	-	2.35	0.45	-
19	10.48	10.48	10.48	8.52	7.98	6.98	6.98	6.09	4.51	5.67	4.51	2.48	4.51	3.12	0.73	3.45	1.87	-	2.48	0.73	-	1.58	-	-
20	10.00	10.00	10.00	7.89	7.32	6.30	6.30	5.39	3.80	4.97	3.80	1.77	3.80	2.41	0.03	2.74	1.16	-	1.77	0.03	-	0.87	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



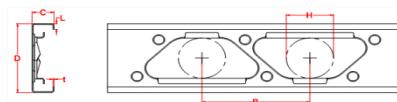
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 2-1/2" flange 68 mills - Section Identification: 600D250-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	20.56	20.56	20.56	20.20	20.08	19.84	19.84	19.61	19.11	19.49	19.11	18.40	19.11	18.64	17.70	18.76	18.16	17.00	18.40	17.70	16.31	18.05	17.23	15.63
9	20.21	20.21	20.21	19.76	19.60	19.28	19.28	18.97	18.35	18.81	18.35	17.45	18.35	17.75	16.56	17.90	17.15	15.69	17.45	16.56	14.83	17.00	15.98	13.98
10	19.84	19.84	19.84	19.24	19.04	18.65	18.65	18.27	17.51	18.08	17.51	16.39	17.51	16.76	15.30	16.94	16.03	14.25	16.39	15.30	13.21	15.84	14.60	12.20
11	19.43	19.43	19.43	18.69	18.45	17.97	17.97	17.50	16.57	17.26	16.57	15.24	16.57	15.68	13.95	15.90	14.80	12.71	15.24	13.95	11.51	14.59	13.12	10.35
12	18.99	18.99	18.99	18.09	17.80	17.22	17.22	16.66	15.56	16.38	15.56	14.00	15.56	14.51	12.52	14.77	13.50	11.11	14.00	12.52	9.76	13.25	11.58	8.47
13	18.46	18.46	18.46	17.38	17.03	16.35	16.35	15.69	14.44	15.37	14.44	12.67	14.44	13.24	11.02	13.53	12.11	9.47	12.67	11.02	8.00	11.83	9.98	6.60
14	17.80	17.80	17.80	16.53	16.13	15.35	15.35	14.61	13.20	14.24	13.20	11.26	13.20	11.89	9.48	12.21	10.65	7.83	11.26	9.48	6.27	10.35	8.37	4.81
15	17.13	17.13	17.13	15.66	15.20	14.33	14.33	13.50	11.98	13.10	11.98	9.90	11.98	10.57	8.01	10.91	9.25	6.28	9.90	8.01	4.67	8.92	6.84	3.15
16	16.46	16.46	16.46	14.78	14.27	13.32	13.32	12.42	10.78	12.00	10.78	8.59	10.78	9.29	6.62	9.65	7.91	4.84	8.59	6.62	3.20	7.59	5.42	1.66
17	15.80	15.80	15.80	13.90	13.35	12.31	12.31	11.35	9.62	10.90	9.62	7.35	9.62	8.08	5.34	8.45	6.66	3.54	7.35	5.34	1.87	6.33	4.12	0.32
18	15.16	15.16	15.16	13.05	12.44	11.33	11.33	10.33	8.53	9.85	8.53	6.21	8.53	6.95	4.18	7.33	5.51	2.36	6.21	4.18	0.69	5.17	2.95	-
19	14.50	14.50	14.50	12.17	11.53	10.37	10.37	9.33	7.50	8.84	7.50	5.15	7.50	5.89	3.11	6.28	4.44	1.30	5.15	3.11	-	4.09	1.89	-
20	13.64	13.64	13.64	11.18	10.53	9.35	9.35	8.30	6.47	7.81	6.47	4.15	6.47	4.88	2.14	5.26	3.44	0.36	4.15	2.14	-	3.11	0.93	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



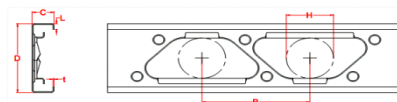
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 2-1/2" flange 33 mills - Section Identification: 800D250-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.79	5.79	5.79	5.57	5.49	5.34	5.34	5.19	4.89	5.12	4.89	4.45	4.89	4.59	4.00	4.67	4.30	3.56	4.45	4.00	3.12	4.22	3.71	2.68
9	5.75	5.75	5.75	5.46	5.37	5.18	5.18	4.99	4.61	4.89	4.61	4.05	4.61	4.24	3.49	4.33	3.86	2.94	4.05	3.49	2.39	3.77	3.12	1.85
10	5.70	5.70	5.70	5.35	5.23	4.99	4.99	4.76	4.30	4.64	4.30	3.61	4.30	3.84	2.93	3.95	3.38	2.27	3.61	2.93	1.61	3.27	2.49	0.96
11	5.65	5.65	5.65	5.22	5.08	4.79	4.79	4.51	3.96	4.37	3.96	3.14	3.96	3.41	2.33	3.55	2.87	1.55	3.14	2.33	0.78	2.73	1.81	0.02
12	5.59	5.59	5.59	5.08	4.91	4.57	4.57	4.24	3.58	4.07	3.58	2.63	3.58	2.95	1.70	3.10	2.32	0.80	2.63	1.70	-	2.16	1.10	-
13	5.52	5.52	5.52	4.93	4.72	4.33	4.33	3.95	3.19	3.76	3.19	2.10	3.19	2.46	1.05	2.64	1.75	0.03	2.10	1.05	-	1.57	0.37	-
14	5.46	5.46	5.46	4.76	4.53	4.08	4.08	3.64	2.79	3.42	2.79	1.56	2.79	1.96	0.38	2.16	1.16	-	1.56	0.38	-	0.97	-	-
15	5.38	5.38	5.38	4.58	4.32	3.82	3.82	3.32	2.36	3.08	2.36	1.01	2.36	1.45	-	1.67	0.57	-	1.01	-	-	0.35	-	-
16	5.31	5.31	5.31	4.40	4.11	3.54	3.54	2.99	1.93	2.72	1.93	0.45	1.93	0.94	-	1.18	-	-	0.45	-	-	-	-	-
17	5.23	5.23	5.23	4.21	3.88	3.25	3.25	2.65	1.50	2.35	1.50	-	1.50	0.43	-	0.69	-	-	-	-	-	-	-	-
18	5.15	5.15	5.15	4.01	3.65	2.96	2.96	2.30	1.07	1.99	1.07	-	1.07	-	-	0.21	-	-	-	-	-	-	-	-
19	5.06	5.06	5.06	3.80	3.41	2.67	2.67	1.96	0.65	1.62	0.65	-	0.65	-	-	-	-	-	-	-	-	-	-	-
20	4.97	4.97	4.97	3.59	3.17	2.37	2.37	1.62	0.24	1.26	0.24	-	0.24	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



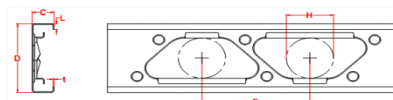
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 2-1/2" flange 43 mills - Section Identification: 800D250-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	8.63	8.63	8.63	8.41	8.34	8.19	8.19	8.04	7.75	7.97	7.75	7.31	7.75	7.46	6.88	7.53	7.17	6.44	7.31	6.88	6.01	7.09	6.59	5.58
9	8.57	8.57	8.57	8.29	8.20	8.01	8.01	7.82	7.45	7.73	7.45	6.90	7.45	7.08	6.35	7.17	6.71	5.80	6.90	6.35	5.26	6.62	5.98	4.72
10	8.50	8.50	8.50	8.15	8.04	7.81	7.81	7.58	7.12	7.46	7.12	6.44	7.12	6.66	5.77	6.78	6.21	5.10	6.44	5.77	4.44	6.10	5.32	3.79
11	8.42	8.42	8.42	8.00	7.86	7.58	7.58	7.30	6.75	7.17	6.75	5.93	6.75	6.20	5.13	6.34	5.66	4.34	5.93	5.13	3.56	5.53	4.60	2.79
12	8.34	8.34	8.34	7.83	7.67	7.34	7.34	7.00	6.35	6.83	6.35	5.38	6.35	5.70	4.45	5.86	5.07	3.54	5.38	4.45	2.63	4.91	3.83	1.75
13	8.25	8.25	8.25	7.66	7.45	7.06	7.06	6.68	5.92	6.48	5.92	4.81	5.92	5.17	3.73	5.36	4.45	2.70	4.81	3.73	1.68	4.27	3.03	0.69
14	8.15	8.15	8.15	7.45	7.23	6.77	6.77	6.33	5.46	6.11	5.46	4.20	5.46	4.62	3.00	4.83	3.80	1.84	4.20	3.00	0.71	3.59	2.22	-
15	8.05	8.05	8.05	7.25	6.98	6.47	6.47	5.96	4.99	5.72	4.99	3.58	4.99	4.04	2.25	4.27	3.13	0.97	3.58	2.25	-	2.91	1.39	-
16	7.94	7.94	7.94	7.02	6.73	6.15	6.15	5.58	4.49	5.30	4.49	2.95	4.49	3.45	1.50	3.71	2.45	0.12	2.95	1.50	-	2.21	0.57	-
17	7.82	7.82	7.82	6.79	6.46	5.81	5.81	5.18	3.99	4.88	3.99	2.31	3.99	2.86	0.76	3.14	1.78	-	2.31	0.76	-	1.52	-	-
18	7.70	7.70	7.70	6.54	6.18	5.46	5.46	4.78	3.48	4.44	3.48	1.68	3.48	2.27	0.03	2.56	1.13	-	1.68	0.03	-	0.85	-	-
19	7.58	7.58	7.58	6.29	5.88	5.10	5.10	4.36	2.97	4.00	2.97	1.08	2.97	1.68	-	2.00	0.48	-	1.08	-	-	0.19	-	-
20	7.45	7.45	7.45	6.02	5.58	4.74	4.74	3.94	2.47	3.56	2.47	0.48	2.47	1.12	-	1.44	-	-	0.48	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



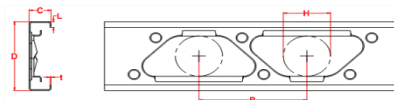
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 2-1/2" flange 54 mills - Section Identification: 800D250-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	14.97	14.97	14.97	14.75	14.68	14.53	14.53	14.38	14.09	14.31	14.09	13.65	14.09	13.79	13.21	13.87	13.50	12.77	13.65	13.21	12.33	13.43	12.92	11.90
9	14.80	14.80	14.80	14.52	14.43	14.24	14.24	14.05	13.68	13.96	13.68	13.13	13.68	13.31	12.57	13.40	12.94	12.02	13.13	12.57	11.47	12.85	12.20	10.93
10	14.62	14.62	14.62	14.27	14.15	13.92	13.92	13.69	13.23	13.58	13.23	12.53	13.23	12.77	11.85	12.89	12.30	11.17	12.53	11.85	10.51	12.19	11.40	9.84
11	14.41	14.41	14.41	13.99	13.85	13.57	13.57	13.28	12.72	13.14	12.72	11.89	12.72	12.16	11.07	12.30	11.62	10.27	11.89	11.07	9.47	11.48	10.54	8.69
12	14.19	14.19	14.19	13.69	13.52	13.17	13.17	12.84	12.17	12.67	12.17	11.20	12.17	11.52	10.24	11.68	10.88	9.30	11.20	10.24	8.38	10.72	9.61	7.47
13	13.96	13.96	13.96	13.36	13.15	12.75	12.75	12.36	11.59	12.17	11.59	10.46	11.59	10.83	9.36	11.02	10.09	8.28	10.46	9.36	7.24	9.90	8.64	6.21
14	13.70	13.70	13.70	13.00	12.77	12.31	12.31	11.86	10.97	11.63	10.97	9.68	10.97	10.11	8.44	10.32	9.26	7.24	9.68	8.44	6.07	9.06	7.63	4.94
15	13.44	13.44	13.44	12.62	12.36	11.84	11.84	11.32	10.32	11.07	10.32	8.88	10.32	9.35	7.50	9.59	8.41	6.17	8.88	7.50	4.90	8.18	6.61	3.67
16	13.16	13.16	13.16	12.23	11.93	11.34	11.34	10.76	9.64	10.48	9.64	8.05	9.64	8.57	6.54	8.84	7.54	5.11	8.05	6.54	3.74	7.29	5.58	2.43
17	12.86	12.86	12.86	11.82	11.48	10.82	10.82	10.18	8.95	9.86	8.95	7.22	8.95	7.78	5.60	8.07	6.67	4.07	7.22	5.60	2.62	6.39	4.57	1.24
18	12.56	12.56	12.56	11.38	11.01	10.28	10.28	9.58	8.24	9.24	8.24	6.39	8.24	6.99	4.67	7.30	5.80	3.06	6.39	4.67	1.55	5.51	3.58	0.11
19	12.24	12.24	12.24	10.94	10.52	9.73	9.73	8.97	7.54	8.60	7.54	5.57	7.54	6.20	3.77	6.53	4.95	2.09	5.57	3.77	0.53	4.65	2.64	-
20	11.92	11.92	11.92	10.48	10.03	9.17	9.17	8.35	6.83	7.96	6.83	4.77	6.83	5.44	2.91	5.78	4.13	1.19	4.77	2.91	-	3.82	1.74	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



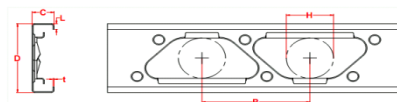
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 2-1/2" flange 68 mills - Section Identification: 800D250-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	21.41	21.41	21.41	21.18	21.10	20.94	20.94	20.78	20.47	20.70	20.47	19.99	20.47	20.15	19.52	20.23	19.84	19.06	19.99	19.52	18.59	19.76	19.21	18.12
9	21.20	21.20	21.20	20.90	20.79	20.59	20.59	20.39	19.99	20.29	19.99	19.40	19.99	19.59	18.80	19.69	19.20	18.20	19.40	18.80	17.60	19.10	18.40	17.02
10	20.96	20.96	20.96	20.59	20.46	20.21	20.21	19.96	19.46	19.84	19.46	18.73	19.46	18.97	17.97	19.10	18.46	17.23	18.73	17.97	16.51	18.33	17.48	15.79
11	20.70	20.70	20.70	20.25	20.10	19.79	19.79	19.49	18.86	19.32	18.86	17.96	18.86	18.26	17.07	18.41	17.66	16.19	17.96	17.07	15.32	17.51	16.48	14.46
12	20.43	20.43	20.43	19.88	19.70	19.31	19.31	18.94	18.22	18.76	18.22	17.14	18.22	17.50	16.09	17.68	16.79	15.05	17.14	16.09	14.04	16.61	15.40	13.03
13	20.13	20.13	20.13	19.46	19.24	18.81	18.81	18.37	17.52	18.16	17.52	16.27	17.52	16.68	15.04	16.89	15.86	13.85	16.27	15.04	12.68	15.65	14.24	11.54
14	19.82	19.82	19.82	19.03	18.78	18.27	18.27	17.76	16.77	17.51	16.77	15.33	16.77	15.81	13.94	16.05	14.86	12.59	15.33	13.94	11.28	14.63	13.04	10.00
15	19.49	19.49	19.49	18.58	18.28	17.69	17.69	17.11	15.98	16.82	15.98	14.35	15.98	14.89	12.79	15.16	13.82	11.29	14.35	12.79	9.85	13.56	11.78	8.45
16	19.14	19.14	19.14	18.09	17.75	17.08	17.08	16.42	15.15	16.09	15.15	13.33	15.15	13.92	11.61	14.23	12.75	9.97	13.33	11.61	8.41	12.46	10.51	6.90
17	18.68	18.68	18.68	17.49	17.10	16.35	16.35	15.61	14.21	15.26	14.21	12.23	14.21	12.87	10.37	13.20	11.59	8.62	12.23	10.37	6.95	11.28	9.19	5.37
18	18.17	18.17	18.17	16.83	16.40	15.56	15.56	14.76	13.23	14.37	13.23	11.10	13.23	11.79	9.13	12.14	10.43	7.28	11.10	9.13	5.55	10.10	7.89	3.89
19	17.65	17.65	17.65	16.16	15.68	14.77	14.77	13.90	12.25	13.47	12.25	9.99	12.25	10.72	7.93	11.10	9.29	6.01	9.99	7.93	4.21	8.94	6.63	2.51
20	17.14	17.14	17.14	15.48	14.96	13.97	13.97	13.03	11.29	12.58	11.29	8.92	11.29	9.68	6.77	10.07	8.18	4.80	8.92	6.77	2.95	7.82	5.44	1.23

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



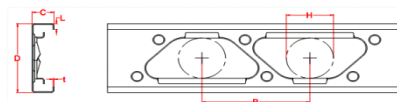
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 3" flange 33 mills - Section Identification: 362D300-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.33	5.33	5.33	4.75	4.57	4.20	4.20	3.85	3.17	3.68	3.17	2.22	3.17	2.53	1.33	2.69	1.92	0.48	2.22	1.33	-	1.77	0.76	-
9	5.16	5.16	5.16	4.43	4.20	3.75	3.75	3.32	2.52	3.12	2.52	1.42	2.52	1.77	0.41	1.95	1.07	-	1.42	0.41	-	0.90	-	-
10	4.98	4.98	4.98	4.08	3.80	3.27	3.27	2.78	1.87	2.54	1.87	0.65	1.87	1.04	-	1.24	0.27	-	0.65	-	-	0.09	-	-
11	4.79	4.79	4.79	3.70	3.38	2.78	2.78	2.24	1.25	1.98	1.25	-	1.25	0.37	-	0.58	-	-	-	-	-	-	-	-
12	4.57	4.57	4.57	3.31	2.95	2.30	2.30	1.72	0.68	1.44	0.68	-	0.68	-	-	-	-	-	-	-	-	-	-	-
13	4.35	4.35	4.35	2.92	2.54	1.85	1.85	1.24	0.18	0.96	0.18	-	0.18	-	-	-	-	-	-	-	-	-	-	-
14	4.12	4.12	4.12	2.55	2.15	1.43	1.43	0.81	-	0.53	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.89	3.89	3.89	2.19	1.78	1.06	1.06	0.44	-	0.15	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.65	3.65	3.65	1.86	1.45	0.73	0.73	0.11	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.42	3.42	3.42	1.56	1.15	0.44	0.44	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.18	3.18	3.18	1.29	0.89	0.20	0.20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.94	2.94	2.94	1.05	0.66	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.73	2.73	2.73	0.84	0.46	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

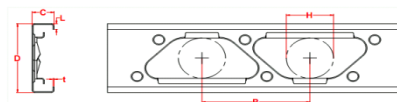


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 3" flange 43 mills - Section Identification: 362D300-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	7.71	7.71	7.71	7.08	6.87	6.47	6.47	6.08	5.32	5.89	5.32	4.25	5.32	4.60	3.25	4.78	3.91	2.29	4.25	3.25	1.38	3.74	2.61	0.51
9	7.48	7.48	7.48	6.66	6.40	5.90	5.90	5.42	4.51	5.19	4.51	3.26	4.51	3.66	2.10	3.87	2.86	1.02	3.26	2.10	0.01	2.67	1.37	-
10	7.23	7.23	7.23	6.21	5.89	5.29	5.29	4.73	3.68	4.46	3.68	2.28	3.68	2.73	1.01	2.96	1.84	-	2.28	1.01	-	1.63	0.23	-
11	6.96	6.96	6.96	5.71	5.34	4.66	4.66	4.02	2.87	3.72	2.87	1.36	2.87	1.84	0.03	2.09	0.90	-	1.36	0.03	-	0.68	-	-
12	6.67	6.67	6.67	5.20	4.79	4.03	4.03	3.34	2.11	3.01	2.11	0.54	2.11	1.04	-	1.30	0.07	-	0.54	-	-	-	-	-
13	6.37	6.37	6.37	4.68	4.22	3.41	3.41	2.69	1.42	2.35	1.42	-	1.42	0.33	-	0.59	-	-	-	-	-	-	-	-
14	6.06	6.06	6.06	4.16	3.68	2.83	2.83	2.09	0.81	1.75	0.81	-	0.81	-	-	-	-	-	-	-	-	-	-	-
15	5.74	5.74	5.74	3.66	3.17	2.30	2.30	1.56	0.28	1.21	0.28	-	0.28	-	-	-	-	-	-	-	-	-	-	-
16	5.41	5.41	5.41	3.19	2.69	1.83	1.83	1.09	-	0.75	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	5.07	5.07	5.07	2.76	2.26	1.41	1.41	0.68	-	0.35	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	4.73	4.73	4.73	2.36	1.87	1.04	1.04	0.33	-	0.00	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	4.30	4.30	4.30	1.98	1.51	0.71	0.71	0.03	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.92	3.92	3.92	1.65	1.20	0.43	0.43	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

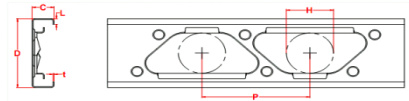
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 3" flange 54 mills - Section Identification: 362D300-54

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	12.80	12.80	12.80	12.14	11.92	11.50	11.50	11.09	10.30	10.89	10.30	9.17	10.30	9.53	8.10	9.72	8.80	7.09	9.17	8.10	6.12	8.62	7.42	5.19
9	12.21	12.21	12.21	11.35	11.07	10.54	10.54	10.03	9.07	9.79	9.07	7.74	9.07	8.17	6.51	8.39	7.32	5.36	7.74	6.51	4.29	7.11	5.74	3.27
10	11.57	11.57	11.57	10.49	10.15	9.52	9.52	8.93	7.82	8.64	7.82	6.33	7.82	6.81	4.98	7.05	5.87	3.76	6.33	4.98	2.62	5.64	4.15	1.55
11	10.90	10.90	10.90	9.58	9.20	8.47	8.47	7.80	6.59	7.49	6.59	5.00	6.59	5.51	3.59	5.77	4.52	2.33	5.00	3.59	1.16	4.28	2.74	0.07
12	10.20	10.20	10.20	8.66	8.22	7.43	7.43	6.71	5.44	6.37	5.44	3.80	5.44	4.32	2.37	4.59	3.31	1.10	3.80	2.37	-	3.06	1.51	-
13	9.48	9.48	9.48	7.73	7.26	6.43	6.43	5.68	4.39	5.34	4.39	2.75	4.39	3.26	1.33	3.53	2.25	0.07	2.75	1.33	-	2.01	0.48	-
14	8.74	8.74	8.74	6.83	6.35	5.49	5.49	4.75	3.46	4.40	3.46	1.84	3.46	2.35	0.46	2.62	1.36	-	1.84	0.46	-	1.13	-	-
15	8.14	8.14	8.14	6.06	5.56	4.70	4.70	3.96	2.68	3.62	2.68	1.09	2.68	1.59	-	1.85	0.62	-	1.09	-	-	0.40	-	-
16	7.44	7.44	7.44	5.29	4.80	3.96	3.96	3.23	2.00	2.90	2.00	0.46	2.00	0.94	-	1.19	0.01	-	0.46	-	-	-	-	-
17	6.80	6.80	6.80	4.61	4.13	3.31	3.31	2.61	1.42	2.29	1.42	-	1.42	0.40	-	0.65	-	-	-	-	-	-	-	-
18	6.09	6.09	6.09	3.96	3.51	2.72	2.72	2.06	0.92	1.75	0.92	-	0.92	-	-	0.19	-	-	-	-	-	-	-	-
19	5.46	5.46	5.46	3.41	2.97	2.23	2.23	1.60	0.52	1.31	0.52	-	0.52	-	-	-	-	-	-	-	-	-	-	-
20	4.93	4.93	4.93	2.94	2.53	1.82	1.82	1.21	0.19	0.94	0.19	-	0.19	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

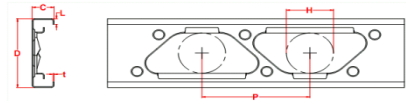


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

3-5/8" DeltaStud 3" flange 68 mills - Section Identification: 362D300-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	17.82	17.82	17.82	17.08	16.84	16.37	16.37	15.91	15.01	15.68	15.01	13.74	15.01	14.16	12.54	14.37	13.33	11.40	13.74	12.54	10.31	13.13	11.77	9.26
9	16.93	16.93	16.93	15.96	15.65	15.05	15.05	14.47	13.38	14.19	13.38	11.88	13.38	12.35	10.48	12.60	11.40	9.17	11.88	10.48	7.95	11.17	9.59	6.79
10	16.10	16.10	16.10	14.85	14.46	13.74	13.74	13.05	11.77	12.72	11.77	10.06	11.77	10.61	8.52	10.89	9.53	7.11	10.06	8.52	5.79	9.27	7.57	4.57
11	15.23	15.23	15.23	13.67	13.21	12.36	12.36	11.57	10.16	11.20	10.16	8.32	10.16	8.90	6.69	9.21	7.76	5.23	8.32	6.69	3.87	7.48	5.70	2.62
12	14.34	14.34	14.34	12.44	11.92	10.97	10.97	10.12	8.63	9.73	8.63	6.71	8.63	7.32	5.06	7.63	6.14	3.57	6.71	5.06	2.22	5.86	4.05	0.97
13	13.20	13.20	13.20	11.06	10.50	9.51	9.51	8.64	7.13	8.24	7.13	5.23	7.13	5.83	3.60	6.14	4.66	2.14	5.23	3.60	0.82	4.39	2.60	-
14	11.90	11.90	11.90	9.63	9.07	8.09	8.09	7.23	5.76	6.84	5.76	3.92	5.76	4.50	2.34	4.80	3.37	0.95	3.92	2.34	-	3.11	1.40	-
15	10.67	10.67	10.67	8.34	7.79	6.84	6.84	6.01	4.60	5.63	4.60	2.83	4.60	3.39	1.33	3.67	2.31	0.00	2.83	1.33	-	2.05	0.44	-
16	9.51	9.51	9.51	7.20	6.67	5.75	5.75	4.96	3.61	4.60	3.61	1.94	3.61	2.46	0.52	2.74	1.44	-	1.94	0.52	-	1.20	-	-
17	8.42	8.42	8.42	6.19	5.68	4.81	4.81	4.06	2.79	3.72	2.79	1.21	2.79	1.71	-	1.96	0.74	-	1.21	-	-	0.52	-	-
18	7.51	7.51	7.51	5.35	4.87	4.04	4.04	3.33	2.12	3.01	2.12	0.63	2.12	1.09	-	1.33	0.18	-	0.63	-	-	-	-	-
19	6.74	6.74	6.74	4.65	4.19	3.40	3.40	2.72	1.57	2.41	1.57	0.15	1.57	0.60	-	0.83	-	-	0.15	-	-	-	-	-
20	6.09	6.09	6.09	4.05	3.61	2.86	2.86	2.22	1.12	1.92	1.12	-	1.12	0.19	-	0.41	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

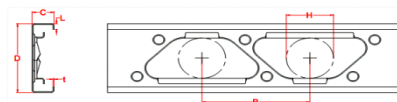


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 3" flange 33 mils - Section Identification: 400D300-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.46	5.46	5.46	4.95	4.79	4.47	4.47	4.15	3.54	4.00	3.54	2.66	3.54	2.95	1.83	3.10	2.38	1.03	2.66	1.83	0.27	2.24	1.29	-
9	5.32	5.32	5.32	4.68	4.47	4.07	4.07	3.68	2.94	3.49	2.94	1.90	2.94	2.24	0.94	2.41	1.57	0.02	1.90	0.94	-	1.41	0.32	-
10	5.16	5.16	5.16	4.37	4.12	3.64	3.64	3.18	2.32	2.96	2.32	1.15	2.32	1.53	0.07	1.72	0.78	-	1.15	0.07	-	0.60	-	-
11	5.00	5.00	5.00	4.04	3.75	3.19	3.19	2.67	1.71	2.42	1.71	0.43	1.71	0.84	-	1.05	0.03	-	0.43	-	-	-	-	-
12	4.82	4.82	4.82	3.69	3.36	2.74	2.74	2.17	1.13	1.90	1.13	-	1.13	0.21	-	0.43	-	-	-	-	-	-	-	-
13	4.63	4.63	4.63	3.33	2.96	2.29	2.29	1.68	0.60	1.40	0.60	-	0.60	-	-	-	-	-	-	-	-	-	-	-
14	4.42	4.42	4.42	2.97	2.58	1.86	1.86	1.23	0.12	0.94	0.12	-	0.12	-	-	-	-	-	-	-	-	-	-	-
15	4.22	4.22	4.22	2.62	2.21	1.47	1.47	0.82	-	0.52	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	4.01	4.01	4.01	2.29	1.86	1.11	1.11	0.46	-	0.16	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.79	3.79	3.79	1.97	1.54	0.79	0.79	0.14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.58	3.58	3.58	1.68	1.25	0.50	0.50	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.36	3.36	3.36	1.42	0.99	0.25	0.25	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.14	3.14	3.14	1.18	0.75	0.03	0.03	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

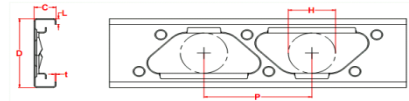
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 3" flange 43 mils - Section Identification: 400D300-43

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	7.91	7.91	7.91	7.36	7.18	6.82	6.82	6.48	5.80	6.30	5.80	4.82	5.80	5.14	3.89	5.30	4.50	2.99	4.82	3.89	2.13	4.35	3.29	1.30
9	7.71	7.71	7.71	7.01	6.78	6.33	6.33	5.90	5.07	5.69	5.07	3.89	5.07	4.27	2.79	4.47	3.52	1.75	3.89	2.79	0.77	3.33	2.09	-
10	7.50	7.50	7.50	6.61	6.33	5.79	5.79	5.27	4.29	5.02	4.29	2.95	4.29	3.38	1.71	3.61	2.53	0.57	2.95	1.71	-	2.32	0.95	-
11	7.27	7.27	7.27	6.19	5.85	5.22	5.22	4.62	3.52	4.33	3.52	2.03	3.52	2.51	0.70	2.75	1.57	-	2.03	0.70	-	1.35	-	-
12	7.03	7.03	7.03	5.73	5.34	4.62	4.62	3.96	2.76	3.65	2.76	1.18	2.76	1.68	-	1.94	0.69	-	1.18	-	-	0.46	-	-
13	6.77	6.77	6.77	5.25	4.83	4.04	4.04	3.32	2.04	2.98	2.04	0.40	2.04	0.92	-	1.19	-	-	0.40	-	-	-	-	-
14	6.50	6.50	6.50	4.77	4.30	3.46	3.46	2.71	1.39	2.36	1.39	-	1.39	0.24	-	0.52	-	-	-	-	-	-	-	-
15	6.21	6.21	6.21	4.29	3.80	2.92	2.92	2.14	0.80	1.78	0.80	-	0.80	-	-	-	-	-	-	-	-	-	-	-
16	5.92	5.92	5.92	3.82	3.31	2.41	2.41	1.62	0.29	1.27	0.29	-	0.29	-	-	-	-	-	-	-	-	-	-	-
17	5.62	5.62	5.62	3.38	2.85	1.95	1.95	1.16	-	0.81	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	5.32	5.32	5.32	2.96	2.43	1.53	1.53	0.76	-	0.41	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	5.01	5.01	5.01	2.57	2.05	1.16	1.16	0.40	-	0.06	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.68	4.68	4.68	2.21	1.70	0.83	0.83	0.10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



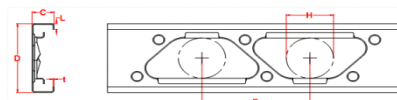
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 3" flange 54 mils - Section Identification: 400D300-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	13.29	13.29	13.29	12.72	12.53	12.17	12.17	11.80	11.09	11.62	11.09	10.07	11.09	10.41	9.09	10.58	9.74	8.14	10.07	9.09	7.23	9.57	8.45	6.35
9	12.78	12.78	12.78	12.05	11.81	11.34	11.34	10.88	10.01	10.66	10.01	8.77	10.01	9.17	7.60	9.38	8.37	6.50	8.77	7.60	5.46	8.18	6.86	4.46
10	12.24	12.24	12.24	11.31	11.01	10.44	10.44	9.89	8.86	9.62	8.86	7.43	8.86	7.89	6.12	8.13	6.98	4.90	7.43	6.12	3.76	6.76	5.30	2.68
11	11.65	11.65	11.65	10.51	10.16	9.49	9.49	8.85	7.70	8.56	7.70	6.12	7.70	6.63	4.71	6.89	5.63	3.42	6.12	4.71	2.22	5.40	3.84	1.09
12	11.04	11.04	11.04	9.67	9.28	8.52	8.52	7.82	6.56	7.49	6.56	4.89	6.56	5.42	3.42	5.70	4.38	2.08	4.89	3.42	0.86	4.13	2.52	-
13	10.41	10.41	10.41	8.83	8.38	7.56	7.56	6.81	5.48	6.46	5.48	3.77	5.48	4.31	2.27	4.59	3.24	0.93	3.77	2.27	-	2.99	1.36	-
14	9.75	9.75	9.75	7.97	7.49	6.62	6.62	5.85	4.49	5.49	4.49	2.76	4.49	3.31	1.27	3.59	2.24	-	2.76	1.27	-	1.99	0.37	-
15	9.08	9.08	9.08	7.14	6.64	5.75	5.75	4.96	3.60	4.60	3.60	1.89	3.60	2.43	0.43	2.71	1.38	-	1.89	0.43	-	1.14	-	-
16	8.49	8.49	8.49	6.39	5.87	4.97	4.97	4.19	2.84	3.83	2.84	1.15	2.84	1.68	-	1.95	0.65	-	1.15	-	-	0.42	-	-
17	7.90	7.90	7.90	5.68	5.17	4.27	4.27	3.49	2.17	3.14	2.17	0.53	2.17	1.04	-	1.31	0.04	-	0.53	-	-	-	-	-
18	7.27	7.27	7.27	5.01	4.50	3.62	3.62	2.87	1.58	2.52	1.58	-	1.58	0.49	-	0.75	-	-	-	-	-	-	-	-
19	6.67	6.67	6.67	4.40	3.90	3.05	3.05	2.32	1.08	1.99	1.08	-	1.08	0.03	-	0.29	-	-	-	-	-	-	-	-
20	6.08	6.08	6.08	3.85	3.37	2.55	2.55	1.85	0.66	1.53	0.66	-	0.66	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



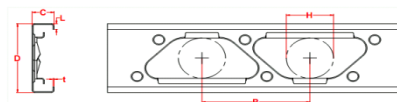
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

4" DeltaStud 3" flange 68 mils - Section Identification: 400D300-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	18.62	18.62	18.62	17.98	17.77	17.36	17.36	16.96	16.17	16.76	16.17	15.02	16.17	15.40	13.92	15.59	14.65	12.85	15.02	13.92	11.83	14.46	13.20	10.84
9	17.86	17.86	17.86	17.02	16.76	16.23	16.23	15.71	14.72	15.46	14.72	13.32	14.72	13.78	12.00	14.01	12.87	10.76	13.32	12.00	9.57	12.65	11.17	8.44
10	17.05	17.05	17.05	15.99	15.66	15.00	15.00	14.38	13.20	14.08	13.20	11.58	13.20	12.10	10.09	12.36	11.07	8.69	11.58	10.09	7.38	10.82	9.14	6.15
11	16.29	16.29	16.29	14.96	14.55	13.78	13.78	13.05	11.70	12.70	11.70	9.89	11.70	10.47	8.26	10.77	9.33	6.77	9.89	8.26	5.38	9.06	7.26	4.09
12	15.50	15.50	15.50	13.88	13.40	12.51	12.51	11.69	10.21	11.30	10.21	8.27	10.21	8.88	6.56	9.20	7.68	5.01	8.27	6.56	3.59	7.39	5.51	2.27
13	14.69	14.69	14.69	12.75	12.21	11.23	11.23	10.34	8.77	9.93	8.77	6.76	8.77	7.40	5.02	7.73	6.16	3.45	6.76	5.02	2.02	5.86	3.96	0.71
14	13.72	13.72	13.72	11.52	10.93	9.90	9.90	8.98	7.39	8.56	7.39	5.37	7.39	6.01	3.64	6.34	4.77	2.09	5.37	3.64	0.69	4.48	2.58	-
15	12.59	12.59	12.59	10.22	9.63	8.58	8.58	7.67	6.10	7.25	6.10	4.12	6.10	4.74	2.43	5.06	3.53	0.93	4.12	2.43	-	3.25	1.42	-
16	11.53	11.53	11.53	9.04	8.45	7.41	7.41	6.52	4.98	6.10	4.98	3.06	4.98	3.66	1.43	3.98	2.49	-	3.06	1.43	-	2.21	0.45	-
17	10.51	10.51	10.51	7.96	7.38	6.37	6.37	5.50	4.01	5.10	4.01	2.17	4.01	2.74	0.60	3.05	1.62	-	2.17	0.60	-	1.35	-	-
18	9.37	9.37	9.37	6.91	6.35	5.38	5.38	4.56	3.15	4.18	3.15	1.40	3.15	1.95	-	2.23	0.88	-	1.40	-	-	0.63	-	-
19	8.41	8.41	8.41	6.02	5.48	4.57	4.57	3.78	2.44	3.42	2.44	0.78	2.44	1.30	-	1.57	0.29	-	0.78	-	-	0.05	-	-
20	7.59	7.59	7.59	5.27	4.76	3.88	3.88	3.13	1.85	2.79	1.85	0.28	1.85	0.77	-	1.02	-	-	0.28	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

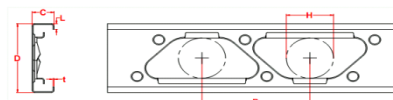


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 3" flange 33 mils - Section Identification: 600D300-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.81	5.81	5.81	5.51	5.41	5.21	5.21	5.01	4.62	4.92	4.62	4.04	4.62	4.23	3.46	4.33	3.84	2.89	4.04	3.46	2.32	3.75	3.08	1.77
9	5.74	5.74	5.74	5.36	5.23	4.98	4.98	4.73	4.23	4.61	4.23	3.50	4.23	3.74	2.79	3.87	3.26	2.09	3.50	2.79	1.41	3.15	2.33	0.74
10	5.67	5.67	5.67	5.19	5.04	4.72	4.72	4.41	3.81	4.26	3.81	2.93	3.81	3.22	2.08	3.37	2.65	1.26	2.93	2.08	0.45	2.50	1.53	-
11	5.58	5.58	5.58	5.00	4.81	4.44	4.44	4.07	3.36	3.89	3.36	2.33	3.36	2.67	1.35	2.84	2.00	0.40	2.33	1.35	-	1.83	0.71	-
12	5.49	5.49	5.49	4.80	4.58	4.14	4.14	3.71	2.89	3.50	2.89	1.71	2.89	2.09	0.60	2.29	1.34	-	1.71	0.60	-	1.15	-	-
13	5.39	5.39	5.39	4.58	4.32	3.82	3.82	3.33	2.40	3.09	2.40	1.09	2.40	1.51	-	1.73	0.67	-	1.09	-	-	0.47	-	-
14	5.29	5.29	5.29	4.35	4.06	3.48	3.48	2.94	1.90	2.67	1.90	0.48	1.90	0.94	-	1.17	0.03	-	0.48	-	-	-	-	-
15	5.18	5.18	5.18	4.11	3.78	3.14	3.14	2.54	1.41	2.25	1.41	-	1.41	0.38	-	0.63	-	-	-	-	-	-	-	-
16	5.06	5.06	5.06	3.86	3.49	2.79	2.79	2.14	0.94	1.83	0.94	-	0.94	-	-	0.11	-	-	-	-	-	-	-	-
17	4.94	4.94	4.94	3.60	3.20	2.44	2.44	1.75	0.48	1.42	0.48	-	0.48	-	-	-	-	-	-	-	-	-	-	-
18	4.81	4.81	4.81	3.33	2.90	2.10	2.10	1.37	0.05	1.02	0.05	-	0.05	-	-	-	-	-	-	-	-	-	-	-
19	4.67	4.67	4.67	3.06	2.60	1.77	1.77	1.01	-	0.65	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.54	4.54	4.54	2.80	2.32	1.45	1.45	0.67	-	0.30	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

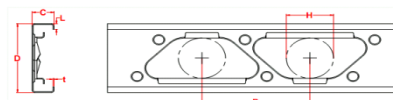
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 3" flange 43 mils - Section Identification: 600D300-43

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	8.46	8.46	8.46	8.15	8.05	7.84	7.84	7.64	7.23	7.54	7.23	6.63	7.23	6.83	6.02	6.93	6.42	5.43	6.63	6.02	4.84	6.32	5.63	4.26
9	8.37	8.37	8.37	7.97	7.84	7.58	7.58	7.32	6.80	7.19	6.80	6.03	6.80	6.29	5.29	6.41	5.78	4.55	6.03	5.29	3.83	5.66	4.79	3.12
10	8.26	8.26	8.26	7.77	7.61	7.28	7.28	6.95	6.32	6.80	6.32	5.40	6.32	5.70	4.49	5.86	5.09	3.61	5.40	4.49	2.75	4.94	3.90	1.92
11	8.14	8.14	8.14	7.54	7.34	6.95	6.95	6.56	5.81	6.37	5.81	4.71	5.81	5.07	3.65	5.25	4.35	2.64	4.71	3.65	1.65	4.18	2.97	0.69
12	8.01	8.01	8.01	7.29	7.06	6.59	6.59	6.14	5.25	5.91	5.25	3.99	5.25	4.40	2.79	4.61	3.58	1.65	3.99	2.79	0.54	3.38	2.02	-
13	7.88	7.88	7.88	7.02	6.75	6.21	6.21	5.68	4.68	5.43	4.68	3.25	4.68	3.72	1.92	3.95	2.80	0.66	3.25	1.92	-	2.58	1.08	-
14	7.73	7.73	7.73	6.74	6.42	5.80	5.80	5.21	4.08	4.92	4.08	2.51	4.08	3.02	1.07	3.28	2.02	-	2.51	1.07	-	1.78	0.15	-
15	7.58	7.58	7.58	6.43	6.07	5.38	5.38	4.72	3.48	4.40	3.48	1.79	3.48	2.33	0.24	2.61	1.26	-	1.79	0.24	-	1.00	-	-
16	7.42	7.42	7.42	6.12	5.71	4.95	4.95	4.22	2.89	3.88	2.89	1.09	2.89	1.67	-	1.96	0.53	-	1.09	-	-	0.26	-	-
17	7.25	7.25	7.25	5.78	5.34	4.51	4.51	3.73	2.31	3.36	2.31	0.42	2.31	1.03	-	1.34	-	-	0.42	-	-	-	-	-
18	7.08	7.08	7.08	5.44	4.96	4.06	4.06	3.24	1.75	2.85	1.75	-	1.75	0.42	-	0.74	-	-	-	-	-	-	-	-
19	6.90	6.90	6.90	5.09	4.58	3.63	3.63	2.77	1.23	2.36	1.23	-	1.23	-	-	0.19	-	-	-	-	-	-	-	-
20	6.71	6.71	6.71	4.74	4.20	3.20	3.20	2.31	0.73	1.89	0.73	-	0.73	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



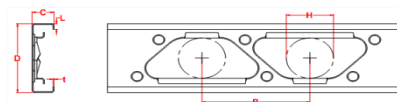
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 3" flange 54 mils - Section Identification: 600D300-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	14.66	14.66	14.66	14.35	14.25	14.04	14.04	13.84	13.43	13.73	13.43	12.80	13.43	13.00	12.19	13.12	12.59	11.58	12.80	12.19	10.98	12.49	11.78	10.39
9	14.41	14.41	14.41	14.01	13.88	13.61	13.61	13.34	12.81	13.21	12.81	12.04	12.81	12.29	11.27	12.42	11.78	10.52	12.04	11.27	9.78	11.65	10.77	9.04
10	14.12	14.12	14.12	13.63	13.45	13.12	13.12	12.79	12.15	12.63	12.15	11.20	12.15	11.51	10.27	11.67	10.89	9.36	11.20	10.27	8.48	10.73	9.66	7.61
11	13.81	13.81	13.81	13.20	12.99	12.59	12.59	12.20	11.42	12.00	11.42	10.29	11.42	10.67	9.20	10.85	9.93	8.15	10.29	9.20	7.12	9.74	8.49	6.12
12	13.48	13.48	13.48	12.74	12.50	12.02	12.02	11.56	10.65	11.33	10.65	9.34	10.65	9.77	8.09	9.98	8.92	6.89	9.34	8.09	5.74	8.71	7.29	4.63
13	13.13	13.13	13.13	12.25	11.97	11.41	11.41	10.87	9.83	10.61	9.83	8.35	9.83	8.83	6.96	9.08	7.88	5.64	8.35	6.96	4.38	7.64	6.07	3.18
14	12.75	12.75	12.75	11.73	11.41	10.77	10.77	10.15	8.98	9.86	8.98	7.35	8.98	7.88	5.83	8.15	6.83	4.41	7.35	5.83	3.07	6.58	4.87	1.79
15	12.36	12.36	12.36	11.19	10.82	10.10	10.10	9.41	8.13	9.08	8.13	6.36	8.13	6.93	4.74	7.22	5.80	3.24	6.36	4.74	1.83	5.53	3.73	0.50
16	11.96	11.96	11.96	10.62	10.20	9.41	9.41	8.66	7.27	8.30	7.27	5.39	7.27	6.00	3.70	6.31	4.81	2.14	5.39	3.70	0.69	4.53	2.64	-
17	11.54	11.54	11.54	10.03	9.57	8.71	8.71	7.91	6.43	7.52	6.43	4.47	6.43	5.10	2.72	5.42	3.87	1.12	4.47	2.72	-	3.57	1.64	-
18	11.11	11.11	11.11	9.43	8.94	8.02	8.02	7.17	5.63	6.77	5.63	3.61	5.63	4.25	1.82	4.58	2.99	0.20	3.61	1.82	-	2.69	0.73	-
19	10.66	10.66	10.66	8.83	8.31	7.34	7.34	6.45	4.87	6.04	4.87	2.80	4.87	3.46	1.00	3.79	2.18	-	2.80	1.00	-	1.88	-	-
20	10.21	10.21	10.21	8.24	7.69	6.67	6.67	5.76	4.15	5.34	4.15	2.07	4.15	2.72	0.27	3.06	1.44	-	2.07	0.27	-	1.14	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



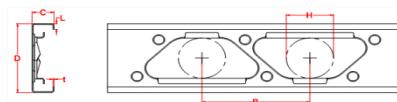
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

6" DeltaStud 3" flange 68 mils - Section Identification: 600D300-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	20.87	20.87	20.87	20.52	20.41	20.18	20.18	19.95	19.50	19.84	19.50	18.80	19.50	19.04	18.12	19.16	18.57	17.44	18.80	18.12	16.78	18.46	17.67	16.12
9	20.48	20.48	20.48	20.04	19.89	19.60	19.60	19.29	18.70	19.14	18.70	17.84	18.70	18.12	16.98	18.27	17.55	16.14	17.84	16.98	15.31	17.41	16.42	14.49
10	20.05	20.05	20.05	19.50	19.30	18.93	18.93	18.57	17.84	18.38	17.84	16.78	17.84	17.13	15.73	17.31	16.43	14.71	16.78	15.73	13.71	16.25	15.05	12.73
11	19.58	19.58	19.58	18.89	18.67	18.22	18.22	17.77	16.90	17.55	16.90	15.62	16.90	16.04	14.39	16.25	15.21	13.19	15.62	14.39	12.03	15.00	13.59	10.90
12	19.09	19.09	19.09	18.25	17.98	17.44	17.44	16.91	15.88	16.65	15.88	14.39	15.88	14.88	12.97	15.13	13.91	11.61	14.39	12.97	10.30	13.68	12.06	9.03
13	18.56	18.56	18.56	17.56	17.24	16.61	16.61	15.99	14.80	15.69	14.80	13.11	14.80	13.66	11.52	13.94	12.57	10.01	13.11	11.52	8.56	12.30	10.50	7.18
14	18.00	18.00	18.00	16.83	16.45	15.73	15.73	15.02	13.68	14.68	13.68	11.80	13.68	12.41	10.06	12.72	11.21	8.42	11.80	10.06	6.88	10.91	8.95	5.41
15	17.47	17.47	17.47	16.11	15.68	14.85	14.85	14.06	12.56	13.67	12.56	10.52	12.56	11.18	8.64	11.52	9.88	6.91	10.52	8.64	5.28	9.56	7.47	3.75
16	16.94	16.94	16.94	15.37	14.88	13.95	13.95	13.08	11.47	12.65	11.47	9.28	11.47	9.98	7.29	10.34	8.58	5.48	9.28	7.29	3.79	8.25	6.07	2.21
17	16.40	16.40	16.40	14.60	14.06	13.05	13.05	12.10	10.37	11.65	10.37	8.07	10.37	8.80	6.00	9.18	7.36	4.14	8.07	6.00	2.42	7.01	4.75	0.81
18	15.84	15.84	15.84	13.82	13.24	12.14	12.14	11.13	9.30	10.65	9.30	6.91	9.30	7.67	4.80	8.06	6.19	2.91	6.91	4.80	1.17	5.83	3.52	-
19	15.28	15.28	15.28	13.04	12.40	11.23	11.23	10.16	8.27	9.67	8.27	5.83	8.27	6.60	3.70	7.01	5.09	1.79	5.83	3.70	0.04	4.73	2.41	-
20	14.64	14.64	14.64	12.19	11.51	10.29	10.29	9.20	7.27	8.69	7.27	4.81	7.27	5.59	2.68	5.99	4.06	0.78	4.81	2.68	-	3.71	1.39	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
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- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

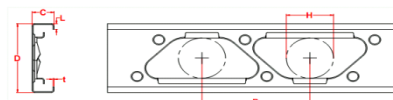
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 3" flange 33 mils - Section Identification: 800D300-33

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	5.96	5.96	5.96	5.73	5.66	5.51	5.51	5.36	5.06	5.28	5.06	4.61	5.06	4.76	4.17	4.83	4.46	3.72	4.61	4.17	3.29	4.39	3.87	2.85
9	5.91	5.91	5.91	5.63	5.53	5.34	5.34	5.15	4.78	5.06	4.78	4.21	4.78	4.40	3.66	4.50	4.03	3.11	4.21	3.66	2.56	3.94	3.29	2.02
10	5.87	5.87	5.87	5.51	5.40	5.16	5.16	4.93	4.47	4.81	4.47	3.78	4.47	4.01	3.10	4.12	3.55	2.43	3.78	3.10	1.77	3.44	2.65	1.11
11	5.81	5.81	5.81	5.39	5.24	4.96	4.96	4.68	4.13	4.54	4.13	3.31	4.13	3.58	2.50	3.71	3.03	1.71	3.31	2.50	0.93	2.90	1.97	0.16
12	5.76	5.76	5.76	5.25	5.08	4.75	4.75	4.41	3.76	4.25	3.76	2.80	3.76	3.12	1.86	3.28	2.48	0.95	2.80	1.86	0.06	2.33	1.25	-
13	5.70	5.70	5.70	5.10	4.90	4.51	4.51	4.12	3.36	3.93	3.36	2.27	3.36	2.63	1.20	2.81	1.91	0.17	2.27	1.20	-	1.73	0.51	-
14	5.63	5.63	5.63	4.93	4.71	4.26	4.26	3.82	2.96	3.60	2.96	1.72	2.96	2.12	0.53	2.33	1.31	-	1.72	0.53	-	1.12	-	-
15	5.56	5.56	5.56	4.76	4.50	3.99	3.99	3.50	2.53	3.25	2.53	1.16	2.53	1.61	-	1.83	0.71	-	1.16	-	-	0.49	-	-
16	5.49	5.49	5.49	4.58	4.29	3.72	3.72	3.16	2.10	2.89	2.10	0.59	2.10	1.08	-	1.33	0.11	-	0.59	-	-	-	-	-
17	5.41	5.41	5.41	4.39	4.07	3.43	3.43	2.82	1.66	2.52	1.66	0.03	1.66	0.56	-	0.83	-	-	0.03	-	-	-	-	-
18	5.33	5.33	5.33	4.19	3.83	3.14	3.14	2.47	1.22	2.15	1.22	-	1.22	0.05	-	0.34	-	-	-	-	-	-	-	-
19	5.24	5.24	5.24	3.99	3.60	2.84	2.84	2.13	0.79	1.78	0.79	-	0.79	-	-	-	-	-	-	-	-	-	-	-
20	5.16	5.16	5.16	3.78	3.35	2.54	2.54	1.78	0.37	1.41	0.37	-	0.37	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

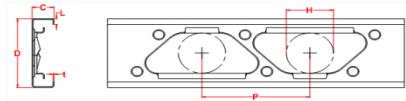
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 3" flange 43 mils - Section Identification: 800D300-43

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	8.69	8.69	8.69	8.47	8.39	8.25	8.25	8.10	7.80	8.02	7.80	7.36	7.80	7.51	6.93	7.58	7.22	6.49	7.36	6.93	6.05	7.14	6.63	5.62
9	8.63	8.63	8.63	8.35	8.25	8.07	8.07	7.88	7.51	7.79	7.51	6.95	7.51	7.14	6.40	7.23	6.77	5.85	6.95	6.40	5.31	6.68	6.03	4.76
10	8.56	8.56	8.56	8.22	8.10	7.87	7.87	7.64	7.18	7.52	7.18	6.50	7.18	6.72	5.82	6.84	6.27	5.15	6.50	5.82	4.49	6.16	5.37	3.83
11	8.49	8.49	8.49	8.07	7.93	7.65	7.65	7.37	6.82	7.23	6.82	6.00	6.82	6.27	5.19	6.41	5.73	4.39	6.00	5.19	3.61	5.59	4.66	2.83
12	8.41	8.41	8.41	7.91	7.74	7.41	7.41	7.08	6.42	6.92	6.42	5.45	6.42	5.77	4.51	5.94	5.13	3.58	5.45	4.51	2.67	4.98	3.89	1.78
13	8.33	8.33	8.33	7.74	7.54	7.14	7.14	6.76	5.99	6.56	5.99	4.88	5.99	5.25	3.79	5.43	4.51	2.74	4.88	3.79	1.70	4.33	3.09	0.70
14	8.24	8.24	8.24	7.55	7.32	6.86	6.86	6.42	5.54	6.20	5.54	4.27	5.54	4.69	3.05	4.90	3.86	1.87	4.27	3.05	0.72	3.66	2.26	-
15	8.14	8.14	8.14	7.34	7.08	6.56	6.56	6.06	5.07	5.81	5.07	3.65	5.07	4.12	2.29	4.35	3.19	0.99	3.65	2.29	-	2.96	1.42	-
16	8.04	8.04	8.04	7.13	6.83	6.25	6.25	5.68	4.58	5.40	4.58	3.01	4.58	3.53	1.53	3.79	2.51	0.12	3.01	1.53	-	2.26	0.58	-
17	7.94	7.94	7.94	6.90	6.57	5.92	5.92	5.29	4.08	4.98	4.08	2.37	4.08	2.93	0.77	3.21	1.83	-	2.37	0.77	-	1.56	-	-
18	7.82	7.82	7.82	6.67	6.30	5.58	5.58	4.89	3.57	4.55	3.57	1.73	3.57	2.33	0.03	2.63	1.15	-	1.73	0.03	-	0.87	-	-
19	7.71	7.71	7.71	6.42	6.01	5.22	5.22	4.47	3.05	4.11	3.05	1.11	3.05	1.73	-	2.06	0.49	-	1.11	-	-	0.19	-	-
20	7.58	7.58	7.58	6.16	5.72	4.86	4.86	4.06	2.55	3.67	2.55	0.49	2.55	1.16	-	1.49	-	-	0.49	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



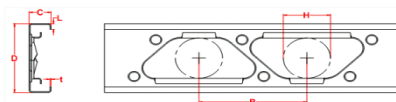
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 3" flange 54 mils - Section Identification: 800D300-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	15.22	15.22	15.22	15.00	14.92	14.77	14.77	14.63	14.33	14.55	14.33	13.89	14.33	14.03	13.45	14.11	13.74	13.00	13.89	13.45	12.56	13.67	13.15	12.13
9	15.06	15.06	15.06	14.78	14.68	14.50	14.50	14.31	13.93	14.21	13.93	13.37	13.93	13.56	12.82	13.65	13.19	12.26	13.37	12.82	11.71	13.09	12.44	11.16
10	14.88	14.88	14.88	14.53	14.42	14.19	14.19	13.95	13.49	13.84	13.49	12.80	13.49	13.03	12.10	13.15	12.57	11.42	12.80	12.10	10.75	12.46	11.65	10.08
11	14.69	14.69	14.69	14.27	14.13	13.84	13.84	13.57	12.99	13.43	12.99	12.16	12.99	12.44	11.34	12.58	11.88	10.53	12.16	11.34	9.72	11.75	10.79	8.93
12	14.48	14.48	14.48	13.98	13.81	13.48	13.48	13.13	12.46	12.96	12.46	11.48	12.46	11.80	10.51	11.97	11.15	9.56	11.48	10.51	8.63	10.99	9.88	7.71
13	14.26	14.26	14.26	13.67	13.47	13.06	13.06	12.67	11.89	12.47	11.89	10.75	11.89	11.13	9.64	11.32	10.37	8.55	10.75	9.64	7.49	10.19	8.91	6.44
14	14.02	14.02	14.02	13.32	13.09	12.63	12.63	12.17	11.28	11.95	11.28	9.98	11.28	10.41	8.72	10.63	9.56	7.50	9.98	8.72	6.31	9.35	7.90	5.15
15	13.77	13.77	13.77	12.96	12.69	12.17	12.17	11.65	10.64	11.40	10.64	9.18	10.64	9.66	7.78	9.90	8.71	6.43	9.18	7.78	5.12	8.47	6.87	3.86
16	13.51	13.51	13.51	12.58	12.28	11.68	11.68	11.10	9.97	10.82	9.97	8.36	9.97	8.89	6.82	9.16	7.84	5.35	8.36	6.82	3.94	7.58	5.83	2.59
17	13.23	13.23	13.23	12.18	11.84	11.18	11.18	10.53	9.29	10.22	9.29	7.52	9.29	8.10	5.86	8.39	6.96	4.29	7.52	5.86	2.79	6.68	4.80	1.36
18	12.94	12.94	12.94	11.77	11.39	10.66	10.66	9.95	8.59	9.60	8.59	6.69	8.59	7.30	4.91	7.62	6.08	3.25	6.69	4.91	1.68	5.79	3.79	0.18
19	12.64	12.64	12.64	11.34	10.92	10.12	10.12	9.34	7.88	8.97	7.88	5.86	7.88	6.51	3.99	6.85	5.22	2.25	5.86	3.99	0.62	4.91	2.82	-
20	12.33	12.33	12.33	10.89	10.44	9.57	9.57	8.74	7.18	8.33	7.18	5.05	7.18	5.73	3.10	6.09	4.38	1.31	5.05	3.10	-	4.05	1.89	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

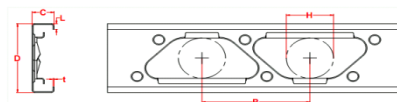
DeltaStud Braced Over Entire Height (Sheathed) ^{3,4}

8" DeltaStud 3" flange 68 mils - Section Identification: 800D300-68

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8	21.79	21.79	21.79	21.56	21.48	21.33	21.33	21.17	20.87	21.10	20.87	20.41	20.87	20.56	19.95	20.64	20.26	19.50	20.41	19.95	19.04	20.18	19.65	18.58
9	21.55	21.55	21.55	21.26	21.16	20.96	20.96	20.77	20.38	20.67	20.38	19.80	20.38	19.99	19.22	20.09	19.60	18.64	19.80	19.22	18.07	19.51	18.84	17.50
10	21.28	21.28	21.28	20.92	20.80	20.56	20.56	20.32	19.84	20.20	19.84	19.12	19.84	19.36	18.41	19.48	18.88	17.68	19.12	18.41	16.97	18.75	17.91	16.27
11	20.99	20.99	20.99	20.55	20.41	20.11	20.11	19.82	19.22	19.68	19.22	18.35	19.22	18.64	17.49	18.78	18.06	16.63	18.35	17.49	15.79	17.92	16.92	14.96
12	20.68	20.68	20.68	20.16	19.98	19.63	19.63	19.26	18.56	19.09	18.56	17.53	18.56	17.87	16.51	18.04	17.19	15.51	17.53	16.51	14.52	17.02	15.84	13.55
13	20.35	20.35	20.35	19.73	19.50	19.08	19.08	18.67	17.85	18.47	17.85	16.65	17.85	17.05	15.47	17.25	16.25	14.31	16.65	15.47	13.18	16.06	14.70	12.07
14	19.99	19.99	19.99	19.25	19.00	18.52	18.52	18.04	17.09	17.80	17.09	15.71	17.09	16.17	14.37	16.40	15.26	13.06	15.71	14.37	11.79	15.04	13.50	10.55
15	19.61	19.61	19.61	18.75	18.47	17.92	17.92	17.37	16.29	17.10	16.29	14.73	16.29	15.24	13.23	15.50	14.22	11.77	14.73	13.23	10.37	13.97	12.25	9.00
16	19.21	19.21	19.21	18.23	17.91	17.28	17.28	16.66	15.45	16.35	15.45	13.71	15.45	14.28	12.05	14.57	13.15	10.46	13.71	12.05	8.93	12.87	10.99	7.46
17	18.80	18.80	18.80	17.68	17.32	16.61	16.61	15.92	14.58	15.58	14.58	12.67	14.58	13.29	10.86	13.61	12.06	9.15	12.67	10.86	7.51	11.75	9.71	5.94
18	18.37	18.37	18.37	17.11	16.71	15.92	15.92	15.15	13.68	14.77	13.68	11.61	13.68	12.28	9.68	12.63	10.95	7.86	11.61	9.68	6.13	10.63	8.45	4.48
19	17.95	17.95	17.95	16.54	16.09	15.22	15.22	14.38	12.79	13.97	12.79	10.57	12.79	11.29	8.52	11.66	9.87	6.60	10.57	8.52	4.80	9.53	7.23	3.08
20	17.55	17.55	17.55	15.98	15.48	14.53	14.53	13.62	11.90	13.18	11.90	9.55	11.90	10.31	7.40	10.70	8.81	5.40	9.55	7.40	3.53	8.45	6.05	1.77

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



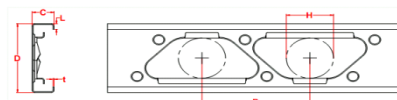
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 1-5/8" flange 33 mills - Section Identification: 362D162-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	3.42	3.42	3.42	2.91	2.75	2.43	2.43	2.13	1.55	1.98	1.55	0.75	1.55	1.01	0.02	1.15	0.50	-	0.75	0.02	-	0.38	-	-
9	3.31	3.31	3.31	2.67	2.47	2.08	2.08	1.72	1.06	1.55	1.06	0.15	1.06	0.44	-	0.59	-	-	0.15	-	-	-	-	-
10	3.19	3.19	3.19	2.40	2.16	1.72	1.72	1.31	0.58	1.12	0.58	-	0.58	-	-	0.07	-	-	-	-	-	-	-	-
11	3.04	3.04	3.04	2.11	1.85	1.36	1.36	0.92	0.14	0.72	0.14	-	0.14	-	-	-	-	-	-	-	-	-	-	-
12	2.88	2.88	2.88	1.83	1.54	1.02	1.02	0.56	-	0.35	-	-	-	-	-	-	-	-	-	-	-	-	-	-
13	2.71	2.71	2.71	1.55	1.25	0.71	0.71	0.25	-	0.03	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14	2.54	2.54	2.54	1.29	0.98	0.44	0.44	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	2.35	2.35	2.35	1.05	0.74	0.21	0.21	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	2.17	2.17	2.17	0.83	0.53	0.01	0.01	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.00	2.00	2.00	0.65	0.35	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	1.85	1.85	1.85	0.49	0.20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	1.72	1.72	1.72	0.36	0.07	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	1.59	1.59	1.59	0.24	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

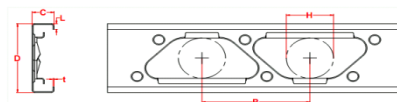
DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 1-5/8" flange 43 mills - Section Identification: 362D162-43

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.79	4.79	4.79	4.30	4.15	3.84	3.84	3.54	2.97	3.39	2.97	2.16	2.97	2.42	1.41	2.56	1.91	0.71	2.16	1.41	0.03	1.78	0.93	-
9	4.64	4.64	4.64	4.01	3.81	3.43	3.43	3.07	2.39	2.90	2.39	1.46	2.39	1.76	0.62	1.92	1.17	-	1.46	0.62	-	1.03	0.09	-
10	4.46	4.46	4.46	3.68	3.45	3.00	3.00	2.58	1.82	2.39	1.82	0.80	1.82	1.13	-	1.29	0.49	-	0.80	-	-	0.33	-	-
11	4.26	4.26	4.26	3.33	3.06	2.56	2.56	2.10	1.27	1.88	1.27	0.20	1.27	0.54	-	0.72	-	-	0.20	-	-	-	-	-
12	4.03	4.03	4.03	2.96	2.67	2.13	2.13	1.64	0.78	1.41	0.78	-	0.78	0.02	-	0.21	-	-	-	-	-	-	-	-
13	3.79	3.79	3.79	2.60	2.28	1.72	1.72	1.21	0.34	0.98	0.34	-	0.34	-	-	-	-	-	-	-	-	-	-	-
14	3.54	3.54	3.54	2.25	1.92	1.35	1.35	0.84	-	0.61	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.28	3.28	3.28	1.92	1.59	1.01	1.01	0.51	-	0.29	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.02	3.02	3.02	1.62	1.29	0.73	0.73	0.24	-	0.02	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.79	2.79	2.79	1.36	1.03	0.48	0.48	0.01	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.57	2.57	2.57	1.13	0.81	0.28	0.28	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.38	2.38	2.38	0.93	0.62	0.10	0.10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.19	2.19	2.19	0.76	0.46	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



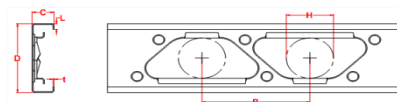
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 1-5/8" flange 54 mills - Section Identification: 362D162-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	7.30	7.30	7.30	6.84	6.70	6.41	6.41	6.12	5.58	5.99	5.58	4.81	5.58	5.06	4.08	5.19	4.56	3.39	4.81	4.08	2.73	4.44	3.62	2.10
9	6.94	6.94	6.94	6.36	6.17	5.81	5.81	5.47	4.82	5.30	4.82	3.92	4.82	4.21	3.09	4.36	3.64	2.32	3.92	3.09	1.60	3.50	2.57	0.91
10	6.52	6.52	6.52	5.81	5.59	5.17	5.17	4.77	4.04	4.58	4.04	3.05	4.04	3.37	2.16	3.53	2.74	1.34	3.05	2.16	0.58	2.59	1.61	-
11	6.06	6.06	6.06	5.22	4.97	4.50	4.50	4.07	3.28	3.86	3.28	2.24	3.28	2.57	1.33	2.74	1.93	0.49	2.24	1.33	-	1.77	0.76	-
12	5.58	5.58	5.58	4.63	4.36	3.86	3.86	3.41	2.59	3.19	2.59	1.54	2.59	1.87	0.62	2.04	1.22	-	1.54	0.62	-	1.06	0.05	-
13	5.13	5.13	5.13	4.09	3.80	3.28	3.28	2.81	1.99	2.59	1.99	0.94	1.99	1.27	0.03	1.44	0.62	-	0.94	0.03	-	0.47	-	-
14	4.71	4.71	4.71	3.59	3.29	2.76	2.76	2.29	1.47	2.07	1.47	0.44	1.47	0.76	-	0.93	0.13	-	0.44	-	-	-	-	-
15	4.28	4.28	4.28	3.11	2.81	2.29	2.29	1.82	1.03	1.61	1.03	0.03	1.03	0.34	-	0.50	-	-	0.03	-	-	-	-	-
16	3.89	3.89	3.89	2.69	2.40	1.88	1.88	1.43	0.66	1.22	0.66	-	0.66	-	-	0.16	-	-	-	-	-	-	-	-
17	3.54	3.54	3.54	2.33	2.04	1.53	1.53	1.10	0.35	0.90	0.35	-	0.35	-	-	-	-	-	-	-	-	-	-	-
18	3.23	3.23	3.23	2.01	1.73	1.24	1.24	0.82	0.10	0.63	0.10	-	0.10	-	-	-	-	-	-	-	-	-	-	-
19	2.95	2.95	2.95	1.74	1.46	0.99	0.99	0.58	-	0.40	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.71	2.71	2.71	1.50	1.24	0.78	0.78	0.39	-	0.21	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



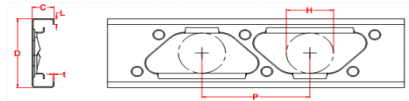
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 1-5/8" flange 68 mills - Section Identification: 362D162-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	9.80	9.80	9.80	9.30	9.14	8.82	8.82	8.51	7.91	8.36	7.91	7.06	7.91	7.34	6.25	7.48	6.78	5.48	7.06	6.25	4.75	6.65	5.74	4.05
9	9.25	9.25	9.25	8.61	8.41	8.01	8.01	7.64	6.92	7.45	6.92	5.92	6.92	6.24	5.00	6.40	5.60	4.14	5.92	5.00	3.33	5.45	4.42	2.56
10	8.60	8.60	8.60	7.82	7.58	7.12	7.12	6.68	5.88	6.47	5.88	4.78	5.88	5.13	3.79	5.32	4.44	2.88	4.78	3.79	2.03	4.27	3.17	1.24
11	7.88	7.88	7.88	6.97	6.70	6.20	6.20	5.72	4.86	5.50	4.86	3.72	4.86	4.08	2.70	4.27	3.37	1.78	3.72	2.70	0.92	3.19	2.07	0.13
12	7.16	7.16	7.16	6.15	5.86	5.32	5.32	4.82	3.94	4.59	3.94	2.78	3.94	3.15	1.77	3.34	2.43	0.86	2.78	1.77	0.02	2.26	1.15	-
13	6.48	6.48	6.48	5.39	5.08	4.53	4.53	4.03	3.14	3.79	3.14	1.99	3.14	2.35	1.00	2.54	1.65	0.11	1.99	1.00	-	1.48	0.40	-
14	5.87	5.87	5.87	4.71	4.40	3.83	3.83	3.33	2.46	3.10	2.46	1.34	2.46	1.69	0.38	1.87	1.00	-	1.34	0.38	-	0.84	-	-
15	5.31	5.31	5.31	4.11	3.80	3.24	3.24	2.74	1.88	2.51	1.88	0.80	1.88	1.14	-	1.32	0.48	-	0.80	-	-	0.32	-	-
16	4.82	4.82	4.82	3.58	3.27	2.72	2.72	2.24	1.41	2.02	1.41	0.36	1.41	0.69	-	0.86	0.05	-	0.36	-	-	-	-	-
17	4.38	4.38	4.38	3.13	2.82	2.28	2.28	1.81	1.01	1.60	1.01	0.00	1.01	0.32	-	0.48	-	-	0.00	-	-	-	-	-
18	3.99	3.99	3.99	2.73	2.43	1.91	1.91	1.45	0.67	1.24	0.67	-	0.67	0.01	-	0.17	-	-	-	-	-	-	-	-
19	3.65	3.65	3.65	2.39	2.09	1.58	1.58	1.14	0.40	0.94	0.40	-	0.40	-	-	-	-	-	-	-	-	-	-	-
20	3.34	3.34	3.34	2.09	1.80	1.31	1.31	0.88	0.17	0.69	0.17	-	0.17	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



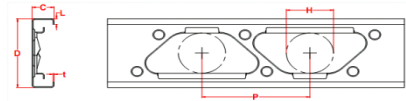
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 1-5/8" flange 33 mills - Section Identification: 400D162-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	3.63	3.63	3.63	3.16	3.01	2.71	2.71	2.42	1.86	2.28	1.86	1.08	1.86	1.34	0.35	1.47	0.83	-	1.08	0.35	-	0.71	-	-
9	3.54	3.54	3.54	2.94	2.75	2.39	2.39	2.04	1.37	1.87	1.37	0.47	1.37	0.76	-	0.91	0.18	-	0.47	-	-	0.04	-	-
10	3.44	3.44	3.44	2.70	2.47	2.04	2.04	1.63	0.89	1.44	0.89	-	0.89	0.20	-	0.37	-	-	-	-	-	-	-	-
11	3.32	3.32	3.32	2.43	2.17	1.68	1.68	1.23	0.42	1.02	0.42	-	0.42	-	-	-	-	-	-	-	-	-	-	-
12	3.18	3.18	3.18	2.16	1.87	1.33	1.33	0.85	-	0.63	-	-	-	-	-	-	-	-	-	-	-	-	-	-
13	3.03	3.03	3.03	1.88	1.57	1.00	1.00	0.50	-	0.27	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14	2.87	2.87	2.87	1.61	1.28	0.70	0.70	0.19	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	2.70	2.70	2.70	1.35	1.02	0.43	0.43	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	2.53	2.53	2.53	1.12	0.78	0.19	0.19	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.36	2.36	2.36	0.90	0.57	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.18	2.18	2.18	0.71	0.38	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.03	2.03	2.03	0.54	0.23	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	1.88	1.88	1.88	0.40	0.09	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



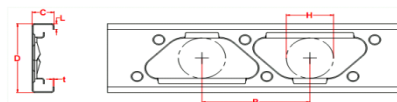
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 1-5/8" flange 43 mills - Section Identification: 400D162-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	5.10	5.10	5.10	4.65	4.50	4.21	4.21	3.93	3.39	3.79	3.39	2.61	3.39	2.86	1.87	2.99	2.36	1.17	2.61	1.87	0.50	2.24	1.40	-
9	4.98	4.98	4.98	4.40	4.21	3.85	3.85	3.50	2.84	3.34	2.84	1.92	2.84	2.22	1.07	2.37	1.63	0.27	1.92	1.07	-	1.49	0.53	-
10	4.83	4.83	4.83	4.11	3.88	3.45	3.45	3.04	2.28	2.84	2.28	1.24	2.28	1.57	0.30	1.75	0.92	-	1.24	0.30	-	0.76	-	-
11	4.66	4.66	4.66	3.79	3.52	3.03	3.03	2.56	1.72	2.34	1.72	0.60	1.72	0.95	-	1.14	0.25	-	0.60	-	-	0.08	-	-
12	4.47	4.47	4.47	3.44	3.14	2.59	2.59	2.09	1.19	1.85	1.19	0.01	1.19	0.39	-	0.58	-	-	0.01	-	-	-	-	-
13	4.26	4.26	4.26	3.09	2.76	2.17	2.17	1.64	0.70	1.39	0.70	-	0.70	-	-	0.08	-	-	-	-	-	-	-	-
14	4.03	4.03	4.03	2.73	2.39	1.77	1.77	1.23	0.28	0.97	0.28	-	0.28	-	-	-	-	-	-	-	-	-	-	-
15	3.79	3.79	3.79	2.39	2.04	1.41	1.41	0.85	-	0.60	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.55	3.55	3.55	2.07	1.71	1.07	1.07	0.53	-	0.28	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.30	3.30	3.30	1.77	1.41	0.78	0.78	0.25	-	0.00	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.06	3.06	3.06	1.50	1.14	0.53	0.53	0.01	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.84	2.84	2.84	1.26	0.91	0.32	0.32	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.63	2.63	2.63	1.05	0.71	0.13	0.13	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



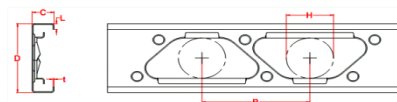
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 1-5/8" flange 54 mills - Section Identification: 400D162-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	7.98	7.98	7.98	7.56	7.42	7.15	7.15	6.88	6.36	6.75	6.36	5.62	6.36	5.86	4.90	5.98	5.37	4.22	5.62	4.90	3.56	5.26	4.44	2.93
9	7.69	7.69	7.69	7.14	6.97	6.62	6.62	6.29	5.66	6.13	5.66	4.76	5.66	5.05	3.93	5.20	4.48	3.14	4.76	3.93	2.39	4.34	3.40	1.68
10	7.34	7.34	7.34	6.66	6.45	6.03	6.03	5.64	4.90	5.45	4.90	3.88	4.90	4.21	2.96	4.38	3.57	2.10	3.88	2.96	1.29	3.41	2.38	0.53
11	6.94	6.94	6.94	6.13	5.88	5.40	5.40	4.95	4.13	4.74	4.13	3.03	4.13	3.39	2.05	3.57	2.70	1.15	3.03	2.05	0.31	2.53	1.44	-
12	6.51	6.51	6.51	5.56	5.27	4.76	4.76	4.27	3.40	4.04	3.40	2.25	3.40	2.61	1.24	2.80	1.90	0.32	2.25	1.24	-	1.73	0.62	-
13	6.03	6.03	6.03	4.97	4.67	4.11	4.11	3.61	2.71	3.37	2.71	1.55	2.71	1.91	0.53	2.10	1.20	-	1.55	0.53	-	1.03	-	-
14	5.57	5.57	5.57	4.41	4.09	3.52	3.52	3.01	2.10	2.77	2.10	0.95	2.10	1.31	-	1.50	0.60	-	0.95	-	-	0.43	-	-
15	5.14	5.14	5.14	3.90	3.57	2.99	2.99	2.48	1.57	2.24	1.57	0.44	1.57	0.80	-	0.98	0.10	-	0.44	-	-	-	-	-
16	4.74	4.74	4.74	3.44	3.11	2.53	2.53	2.01	1.13	1.78	1.13	0.02	1.13	0.37	-	0.55	-	-	0.02	-	-	-	-	-
17	4.37	4.37	4.37	3.03	2.69	2.12	2.12	1.61	0.75	1.38	0.75	-	0.75	0.01	-	0.19	-	-	-	-	-	-	-	-
18	4.04	4.04	4.04	2.66	2.33	1.76	1.76	1.27	0.43	1.04	0.43	-	0.43	-	-	-	-	-	-	-	-	-	-	-
19	3.71	3.71	3.71	2.32	2.00	1.45	1.45	0.97	0.16	0.75	0.16	-	0.16	-	-	-	-	-	-	-	-	-	-	-
20	3.40	3.40	3.40	2.02	1.71	1.17	1.17	0.71	-	0.50	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



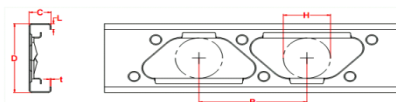
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 1-5/8" flange 68 mills - Section Identification: 400D162-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	10.77	10.77	10.77	10.31	10.16	9.86	9.86	9.57	9.00	9.42	9.00	8.17	9.00	8.44	7.38	8.58	7.90	6.62	8.17	7.38	5.89	7.77	6.87	5.18
9	10.36	10.36	10.36	9.76	9.57	9.19	9.19	8.82	8.11	8.64	8.11	7.11	8.11	7.44	6.18	7.60	6.79	5.29	7.11	6.18	4.46	6.64	5.58	3.66
10	9.88	9.88	9.88	9.12	8.88	8.42	8.42	7.98	7.15	7.76	7.15	6.01	7.15	6.37	4.96	6.56	5.65	3.99	6.01	4.96	3.08	5.47	4.31	2.22
11	9.32	9.32	9.32	8.40	8.12	7.59	7.59	7.09	6.16	6.85	6.16	4.92	6.16	5.32	3.80	5.52	4.54	2.78	4.92	3.80	1.83	4.34	3.11	0.94
12	8.71	8.71	8.71	7.63	7.32	6.73	6.73	6.18	5.19	5.93	5.19	3.89	5.19	4.31	2.74	4.52	3.50	1.70	3.89	2.74	0.75	3.30	2.04	-
13	8.03	8.03	8.03	6.83	6.49	5.86	5.86	5.29	4.27	5.02	4.27	2.96	4.27	3.37	1.81	3.59	2.56	0.78	2.96	1.81	-	2.36	1.11	-
14	7.30	7.30	7.30	6.02	5.66	5.02	5.02	4.45	3.43	4.18	3.43	2.14	3.43	2.55	1.02	2.76	1.75	0.02	2.14	1.02	-	1.56	0.34	-
15	6.63	6.63	6.63	5.29	4.93	4.28	4.28	3.71	2.72	3.45	2.72	1.46	2.72	1.85	0.38	2.06	1.08	-	1.46	0.38	-	0.90	-	-
16	6.03	6.03	6.03	4.64	4.28	3.64	3.64	3.08	2.11	2.82	2.11	0.89	2.11	1.27	-	1.47	0.53	-	0.89	-	-	0.35	-	-
17	5.49	5.49	5.49	4.07	3.72	3.09	3.09	2.54	1.60	2.29	1.60	0.42	1.60	0.79	-	0.98	0.07	-	0.42	-	-	-	-	-
18	5.01	5.01	5.01	3.57	3.22	2.61	2.61	2.08	1.17	1.84	1.17	0.04	1.17	0.39	-	0.58	-	-	0.04	-	-	-	-	-
19	4.58	4.58	4.58	3.14	2.80	2.21	2.21	1.69	0.81	1.46	0.81	-	0.81	0.06	-	0.24	-	-	-	-	-	-	-	-
20	4.20	4.20	4.20	2.76	2.43	1.85	1.85	1.36	0.51	1.13	0.51	-	0.51	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



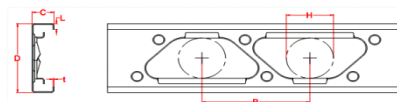
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 1-5/8" flange 33 mills - Section Identification: 600D162-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.15	4.15	4.15	3.83	3.73	3.52	3.52	3.31	2.91	3.21	2.91	2.30	2.91	2.50	1.71	2.60	2.10	1.13	2.30	1.71	0.56	2.00	1.32	0.00
9	4.15	4.15	4.15	3.74	3.61	3.34	3.34	3.07	2.56	2.94	2.56	1.81	2.56	2.05	1.08	2.18	1.56	0.37	1.81	1.08	-	1.44	0.60	-
10	4.12	4.12	4.12	3.61	3.44	3.11	3.11	2.79	2.16	2.63	2.16	1.27	2.16	1.56	0.40	1.71	0.97	-	1.27	0.40	-	0.83	-	-
11	4.08	4.08	4.08	3.46	3.26	2.86	2.86	2.48	1.74	2.29	1.74	0.70	1.74	1.04	-	1.21	0.36	-	0.70	-	-	0.20	-	-
12	4.03	4.03	4.03	3.29	3.05	2.59	2.59	2.15	1.30	1.93	1.30	0.13	1.30	0.51	-	0.70	-	-	0.13	-	-	-	-	-
13	3.98	3.98	3.98	3.10	2.83	2.30	2.30	1.80	0.85	1.55	0.85	-	0.85	-	-	0.19	-	-	-	-	-	-	-	-
14	3.91	3.91	3.91	2.89	2.58	1.99	1.99	1.44	0.41	1.17	0.41	-	0.41	-	-	-	-	-	-	-	-	-	-	-
15	3.82	3.82	3.82	2.67	2.33	1.68	1.68	1.08	-	0.80	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.73	3.73	3.73	2.44	2.07	1.37	1.37	0.73	-	0.44	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.62	3.62	3.62	2.21	1.80	1.07	1.07	0.41	-	0.09	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.51	3.51	3.51	1.97	1.55	0.78	0.78	0.10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.39	3.39	3.39	1.74	1.30	0.51	0.51	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.26	3.26	3.26	1.51	1.06	0.27	0.27	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



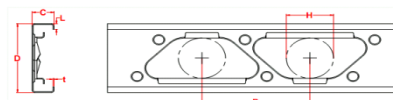
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 1-5/8" flange 43 mills - Section Identification: 600D162-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	5.84	5.84	5.84	5.55	5.45	5.25	5.25	5.06	4.68	4.97	4.68	4.11	4.68	4.30	3.55	4.39	3.92	3.00	4.11	3.55	2.45	3.83	3.18	1.92
9	5.84	5.84	5.84	5.46	5.34	5.08	5.08	4.83	4.35	4.71	4.35	3.63	4.35	3.87	2.93	3.98	3.39	2.25	3.63	2.93	1.58	3.28	2.47	0.93
10	5.82	5.82	5.82	5.34	5.18	4.87	4.87	4.56	3.96	4.41	3.96	3.09	3.96	3.38	2.25	3.52	2.80	1.44	3.09	2.25	0.65	2.66	1.70	-
11	5.77	5.77	5.77	5.18	4.99	4.61	4.61	4.24	3.52	4.06	3.52	2.50	3.52	2.83	1.52	3.00	2.17	0.59	2.50	1.52	-	2.00	0.90	-
12	5.70	5.70	5.70	4.99	4.76	4.32	4.32	3.89	3.05	3.67	3.05	1.88	3.05	2.26	0.79	2.46	1.51	-	1.88	0.79	-	1.33	0.09	-
13	5.62	5.62	5.62	4.78	4.52	4.00	4.00	3.50	2.56	3.26	2.56	1.26	2.56	1.68	0.06	1.90	0.85	-	1.26	0.06	-	0.65	-	-
14	5.52	5.52	5.52	4.55	4.24	3.66	3.66	3.10	2.06	2.83	2.06	0.65	2.06	1.10	-	1.34	0.21	-	0.65	-	-	-	-	-
15	5.41	5.41	5.41	4.29	3.95	3.30	3.30	2.69	1.57	2.40	1.57	0.06	1.57	0.55	-	0.79	-	-	0.06	-	-	-	-	-
16	5.28	5.28	5.28	4.01	3.63	2.93	2.93	2.27	1.08	1.96	1.08	-	1.08	0.01	-	0.27	-	-	-	-	-	-	-	-
17	5.13	5.13	5.13	3.72	3.31	2.56	2.56	1.87	0.63	1.54	0.63	-	0.63	-	-	-	-	-	-	-	-	-	-	-
18	4.97	4.97	4.97	3.43	2.99	2.20	2.20	1.48	0.20	1.14	0.20	-	0.20	-	-	-	-	-	-	-	-	-	-	-
19	4.80	4.80	4.80	3.13	2.67	1.85	1.85	1.11	-	0.77	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.62	4.62	4.62	2.84	2.36	1.52	1.52	0.77	-	0.42	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



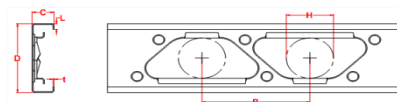
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 1-5/8" flange 54 mills - Section Identification: 600D162-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	9.58	9.58	9.58	9.31	9.21	9.03	9.03	8.85	8.48	8.76	8.48	7.94	8.48	8.12	7.40	8.21	7.76	6.88	7.94	7.40	6.35	7.67	7.05	5.84
9	9.58	9.58	9.58	9.22	9.11	8.86	8.86	8.62	8.15	8.50	8.15	7.46	8.15	7.69	6.78	7.81	7.23	6.12	7.46	6.78	5.47	7.12	6.34	4.83
10	9.58	9.58	9.58	9.12	8.97	8.66	8.66	8.36	7.78	8.22	7.78	6.92	7.78	7.20	6.09	7.34	6.64	5.28	6.92	6.09	4.50	6.50	5.55	3.73
11	9.48	9.48	9.48	8.91	8.72	8.35	8.35	7.98	7.27	7.80	7.27	6.25	7.27	6.59	5.27	6.76	5.92	4.34	6.25	5.27	3.43	5.76	4.64	2.56
12	9.32	9.32	9.32	8.63	8.40	7.96	7.96	7.53	6.69	7.31	6.69	5.51	6.69	5.90	4.40	6.09	5.14	3.35	5.51	4.40	2.35	4.95	3.69	1.39
13	9.13	9.13	9.13	8.30	8.03	7.51	7.51	7.02	6.07	6.77	6.07	4.74	6.07	5.17	3.52	5.39	4.33	2.37	4.74	3.52	1.28	4.12	2.74	0.26
14	8.89	8.89	8.89	7.92	7.61	7.02	7.02	6.46	5.41	6.19	5.41	3.96	5.41	4.43	2.64	4.67	3.51	1.42	3.96	2.64	0.28	3.29	1.82	-
15	8.61	8.61	8.61	7.49	7.15	6.49	6.49	5.88	4.73	5.58	4.73	3.20	4.73	3.69	1.81	3.94	2.72	0.54	3.20	1.81	-	2.49	0.95	-
16	8.29	8.29	8.29	7.03	6.65	5.94	5.94	5.28	4.07	4.96	4.07	2.46	4.07	2.97	1.03	3.24	1.97	-	2.46	1.03	-	1.73	0.16	-
17	7.95	7.95	7.95	6.55	6.14	5.38	5.38	4.69	3.42	4.35	3.42	1.78	3.42	2.30	0.33	2.57	1.28	-	1.78	0.33	-	1.03	-	-
18	7.57	7.57	7.57	6.06	5.62	4.83	4.83	4.11	2.82	3.77	2.82	1.15	2.82	1.68	-	1.95	0.65	-	1.15	-	-	0.41	-	-
19	7.18	7.18	7.18	5.56	5.11	4.29	4.29	3.56	2.26	3.21	2.26	0.59	2.26	1.12	-	1.39	0.09	-	0.59	-	-	-	-	-
20	6.78	6.78	6.78	5.07	4.61	3.78	3.78	3.04	1.75	2.70	1.75	0.10	1.75	0.62	-	0.89	-	-	0.10	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



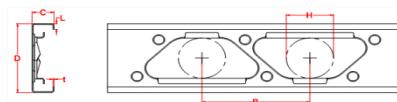
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 1-5/8" flange 68 mills - Section Identification: 600D162-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	12.88	12.88	12.88	12.61	12.52	12.34	12.34	12.15	11.79	12.06	11.79	11.24	11.79	11.42	10.70	11.51	11.06	10.17	11.24	10.70	9.65	10.97	10.35	9.13
9	12.88	12.88	12.88	12.53	12.41	12.16	12.16	11.92	11.45	11.80	11.45	10.75	11.45	10.98	10.07	11.10	10.52	9.39	10.75	10.07	8.73	10.41	9.62	8.08
10	12.88	12.88	12.88	12.42	12.26	11.96	11.96	11.66	11.06	11.51	11.06	10.19	11.06	10.48	9.34	10.62	9.90	8.51	10.19	9.34	7.70	9.76	8.78	6.91
11	12.88	12.88	12.88	12.30	12.10	11.72	11.72	11.34	10.61	11.16	10.61	9.55	10.61	9.90	8.53	10.07	9.20	7.54	9.55	8.53	6.59	9.03	7.86	5.67
12	12.71	12.71	12.71	11.99	11.75	11.29	11.29	10.84	9.97	10.62	9.97	8.72	9.97	9.13	7.54	9.34	8.32	6.42	8.72	7.54	5.35	8.13	6.79	4.32
13	12.44	12.44	12.44	11.57	11.29	10.75	10.75	10.23	9.22	9.97	9.22	7.81	9.22	8.27	6.50	8.50	7.36	5.26	7.81	6.50	4.09	7.14	5.66	2.98
14	12.11	12.11	12.11	11.09	10.77	10.14	10.14	9.54	8.42	9.25	8.42	6.86	8.42	7.36	5.44	7.62	6.37	4.11	6.86	5.44	2.87	6.13	4.54	1.69
15	11.73	11.73	11.73	10.55	10.18	9.48	9.48	8.81	7.58	8.49	7.58	5.91	7.58	6.44	4.40	6.72	5.39	3.01	5.91	4.40	1.72	5.14	3.46	0.51
16	11.30	11.30	11.30	9.95	9.54	8.76	8.76	8.05	6.74	7.71	6.74	4.97	6.74	5.53	3.41	5.83	4.44	1.99	4.97	3.41	0.67	4.17	2.45	-
17	10.82	10.82	10.82	9.31	8.87	8.05	8.05	7.28	5.90	6.92	5.90	4.09	5.90	4.67	2.50	4.97	3.54	1.06	4.09	2.50	-	3.27	1.52	-
18	10.30	10.30	10.30	8.66	8.19	7.32	7.32	6.52	5.11	6.15	5.11	3.27	5.11	3.85	1.67	4.15	2.71	0.22	3.27	1.67	-	2.44	0.70	-
19	9.77	9.77	9.77	8.00	7.50	6.60	6.60	5.79	4.36	5.41	4.36	2.52	4.36	3.09	0.92	3.40	1.96	-	2.52	0.92	-	1.70	-	-
20	9.21	9.21	9.21	7.34	6.83	5.92	5.92	5.10	3.67	4.72	3.67	1.84	3.67	2.41	0.27	2.71	1.29	-	1.84	0.27	-	1.03	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



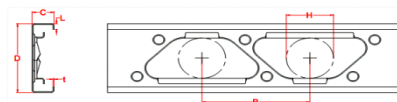
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 1-5/8" flange 33 mills - Section Identification: 800D162-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	3.96	3.96	3.96	3.74	3.67	3.52	3.52	3.37	3.09	3.30	3.09	2.65	3.09	2.80	2.23	2.87	2.51	1.80	2.65	2.23	1.38	2.44	1.94	0.96
9	3.96	3.96	3.96	3.68	3.58	3.40	3.40	3.22	2.85	3.12	2.85	2.30	2.85	2.48	1.77	2.57	2.12	1.23	2.30	1.77	0.71	2.03	1.41	0.19
10	3.96	3.96	3.96	3.61	3.49	3.26	3.26	3.04	2.58	2.92	2.58	1.91	2.58	2.14	1.26	2.25	1.69	0.61	1.91	1.26	-	1.58	0.83	-
11	3.96	3.96	3.96	3.53	3.39	3.11	3.11	2.84	2.29	2.70	2.29	1.49	2.29	1.75	0.71	1.89	1.23	-	1.49	0.71	-	1.10	0.20	-
12	3.96	3.96	3.96	3.45	3.28	2.95	2.95	2.62	1.97	2.45	1.97	1.03	1.97	1.34	0.13	1.50	0.73	-	1.03	0.13	-	0.58	-	-
13	3.96	3.96	3.96	3.35	3.15	2.76	2.76	2.37	1.63	2.19	1.63	0.55	1.63	0.90	-	1.08	0.20	-	0.55	-	-	0.03	-	-
14	3.96	3.96	3.96	3.24	3.01	2.56	2.56	2.12	1.27	1.90	1.27	0.05	1.27	0.45	-	0.65	-	-	0.05	-	-	-	-	-
15	3.96	3.96	3.96	3.13	2.86	2.35	2.35	1.85	0.89	1.60	0.89	-	0.89	-	-	0.20	-	-	-	-	-	-	-	-
16	3.96	3.96	3.96	3.01	2.71	2.12	2.12	1.56	0.50	1.29	0.50	-	0.50	-	-	-	-	-	-	-	-	-	-	-
17	3.96	3.96	3.96	2.87	2.54	1.88	1.88	1.26	0.11	0.97	0.11	-	0.11	-	-	-	-	-	-	-	-	-	-	-
18	3.96	3.96	3.96	2.73	2.36	1.64	1.64	0.96	-	0.64	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.96	3.96	3.96	2.58	2.17	1.38	1.38	0.66	-	0.31	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.96	3.96	3.96	2.42	1.97	1.13	1.13	0.35	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



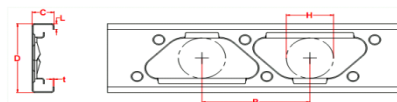
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 1-5/8" flange 43 mills - Section Identification: 800D162-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	5.74	5.74	5.74	5.52	5.44	5.30	5.30	5.15	4.86	5.08	4.86	4.43	4.86	4.57	4.00	4.64	4.28	3.57	4.43	4.00	3.14	4.21	3.71	2.72
9	5.74	5.74	5.74	5.46	5.36	5.18	5.18	4.99	4.62	4.90	4.62	4.07	4.62	4.25	3.53	4.35	3.89	2.99	4.07	3.53	2.45	3.80	3.17	1.92
10	5.74	5.74	5.74	5.39	5.27	5.04	5.04	4.81	4.35	4.69	4.35	3.67	4.35	3.90	3.01	4.01	3.45	2.35	3.67	3.01	1.70	3.34	2.57	1.05
11	5.74	5.74	5.74	5.31	5.17	4.89	4.89	4.61	4.05	4.47	4.05	3.23	4.05	3.50	2.43	3.64	2.97	1.65	3.23	2.43	0.88	2.83	1.91	0.12
12	5.74	5.74	5.74	5.22	5.05	4.71	4.71	4.37	3.72	4.21	3.72	2.76	3.72	3.07	1.82	3.23	2.44	0.91	2.76	1.82	0.02	2.29	1.21	-
13	5.74	5.74	5.74	5.12	4.92	4.52	4.52	4.13	3.36	3.93	3.36	2.24	3.36	2.61	1.17	2.79	1.88	0.13	2.24	1.17	-	1.70	0.48	-
14	5.74	5.74	5.74	5.01	4.78	4.31	4.31	3.86	2.97	3.63	2.97	1.71	2.97	2.12	0.50	2.33	1.30	-	1.71	0.50	-	1.09	-	-
15	5.74	5.74	5.74	4.89	4.62	4.09	4.09	3.57	2.56	3.31	2.56	1.15	2.56	1.61	-	1.84	0.69	-	1.15	-	-	0.47	-	-
16	5.74	5.74	5.74	4.76	4.45	3.84	3.84	3.26	2.14	2.97	2.14	0.58	2.14	1.09	-	1.34	0.08	-	0.58	-	-	-	-	-
17	5.74	5.74	5.74	4.62	4.27	3.59	3.59	2.93	1.70	2.62	1.70	0.00	1.70	0.55	-	0.83	-	-	0.00	-	-	-	-	-
18	5.74	5.74	5.74	4.47	4.07	3.32	3.32	2.60	1.26	2.25	1.26	-	1.26	0.02	-	0.32	-	-	-	-	-	-	-	-
19	5.73	5.73	5.73	4.30	3.86	3.03	3.03	2.25	0.81	1.87	0.81	-	0.81	-	-	-	-	-	-	-	-	-	-	-
20	5.63	5.63	5.63	4.06	3.59	2.69	2.69	1.87	0.36	1.47	0.36	-	0.36	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



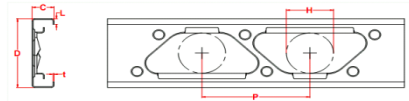
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 1-5/8" flange 54 mills - Section Identification: 800D162-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	9.28	9.28	9.28	9.08	9.01	8.88	8.88	8.74	8.48	8.68	8.48	8.08	8.48	8.21	7.68	8.28	7.95	7.29	8.08	7.68	6.89	7.88	7.42	6.50
9	9.28	9.28	9.28	9.02	8.93	8.76	8.76	8.59	8.25	8.51	8.25	7.75	8.25	7.92	7.24	8.00	7.58	6.74	7.75	7.24	6.25	7.49	6.91	5.75
10	9.28	9.28	9.28	8.96	8.85	8.64	8.64	8.42	8.00	8.32	8.00	7.37	8.00	7.58	6.75	7.68	7.16	6.13	7.37	6.75	5.52	7.06	6.34	4.91
11	9.28	9.28	9.28	8.88	8.75	8.49	8.49	8.23	7.70	8.10	7.70	6.94	7.70	7.19	6.18	7.32	6.69	5.44	6.94	6.18	4.71	6.56	5.69	3.99
12	9.28	9.28	9.28	8.80	8.64	8.32	8.32	8.00	7.38	7.85	7.38	6.47	7.38	6.77	5.58	6.92	6.17	4.71	6.47	5.58	3.85	6.02	5.00	3.02
13	9.28	9.28	9.28	8.70	8.51	8.13	8.13	7.76	7.03	7.57	7.03	5.96	7.03	6.31	4.92	6.49	5.61	3.92	5.96	4.92	2.94	5.44	4.25	2.00
14	9.28	9.28	9.28	8.59	8.37	7.92	7.92	7.49	6.64	7.27	6.64	5.41	6.64	5.81	4.23	6.02	5.01	3.10	5.41	4.23	2.01	4.81	3.47	0.95
15	9.28	9.28	9.28	8.47	8.21	7.70	7.70	7.19	6.22	6.95	6.22	4.83	6.22	5.28	3.51	5.51	4.38	2.25	4.83	3.51	1.05	4.16	2.66	-
16	9.28	9.28	9.28	8.34	8.04	7.45	7.45	6.87	5.77	6.59	5.77	4.22	5.77	4.72	2.76	4.98	3.72	1.39	4.22	2.76	0.08	3.48	1.83	-
17	9.28	9.28	9.28	8.19	7.85	7.17	7.17	6.53	5.30	6.21	5.30	3.59	5.30	4.14	2.00	4.42	3.05	0.53	3.59	2.00	-	2.78	1.01	-
18	9.28	9.28	9.28	8.03	7.64	6.88	6.88	6.16	4.80	5.81	4.80	2.95	4.80	3.54	1.25	3.85	2.36	-	2.95	1.25	-	2.08	0.19	-
19	9.28	9.28	9.28	7.85	7.41	6.56	6.56	5.77	4.29	5.39	4.29	2.30	4.29	2.94	0.51	3.27	1.69	-	2.30	0.51	-	1.38	-	-
20	9.10	9.10	9.10	7.51	7.03	6.12	6.12	5.27	3.72	4.87	3.72	1.64	3.72	2.30	-	2.65	1.00	-	1.64	-	-	0.69	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



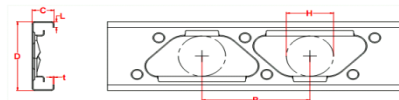
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 1-5/8" flange 68 mills - Section Identification: 800D162-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	12.56	12.56	12.56	12.36	12.29	12.16	12.16	12.03	11.77	11.96	11.77	11.37	11.77	11.50	10.98	11.57	11.24	10.59	11.37	10.98	10.20	11.17	10.72	9.81
9	12.56	12.56	12.56	12.30	12.22	12.05	12.05	11.88	11.54	11.80	11.54	11.04	11.54	11.21	10.54	11.29	10.87	10.04	11.04	10.54	9.54	10.79	10.20	9.05
10	12.56	12.56	12.56	12.24	12.13	11.92	11.92	11.71	11.29	11.60	11.29	10.66	11.29	10.87	10.03	10.97	10.45	9.41	10.66	10.03	8.79	10.35	9.61	8.19
11	12.56	12.56	12.56	12.17	12.04	11.78	11.78	11.52	10.99	11.39	10.99	10.22	10.99	10.47	9.46	10.60	9.97	8.71	10.22	9.46	7.98	9.84	8.96	7.25
12	12.56	12.56	12.56	12.08	11.93	11.61	11.61	11.28	10.66	11.13	10.66	9.74	10.66	10.04	8.84	10.20	9.44	7.95	9.74	8.84	7.08	9.29	8.24	6.23
13	12.56	12.56	12.56	11.99	11.79	11.41	11.41	11.03	10.29	10.85	10.29	9.21	10.29	9.57	8.15	9.75	8.85	7.13	9.21	8.15	6.12	8.68	7.47	5.14
14	12.56	12.56	12.56	11.87	11.64	11.20	11.20	10.75	9.89	10.54	9.89	8.63	9.89	9.05	7.42	9.26	8.22	6.25	8.63	7.42	5.11	8.02	6.63	4.01
15	12.56	12.56	12.56	11.75	11.48	10.96	10.96	10.45	9.45	10.19	9.45	8.01	9.45	8.48	6.64	8.72	7.55	5.33	8.01	6.64	4.07	7.32	5.76	2.85
16	12.56	12.56	12.56	11.61	11.30	10.70	10.70	10.11	8.97	9.82	8.97	7.35	8.97	7.88	5.83	8.15	6.83	4.38	7.35	5.83	3.00	6.58	4.85	1.69
17	12.56	12.56	12.56	11.45	11.10	10.40	10.40	9.73	8.45	9.41	8.45	6.66	8.45	7.24	4.99	7.54	6.09	3.42	6.66	4.99	1.93	5.81	3.93	0.52
18	12.56	12.56	12.56	11.28	10.87	10.09	10.09	9.33	7.91	8.97	7.91	5.94	7.91	6.58	4.14	6.90	5.33	2.46	5.94	4.14	0.88	5.02	3.01	-
19	12.56	12.56	12.56	11.08	10.62	9.74	9.74	8.90	7.34	8.50	7.34	5.21	7.34	5.90	3.29	6.25	4.55	1.51	5.21	3.29	-	4.23	2.09	-
20	12.47	12.47	12.47	10.79	10.28	9.30	9.30	8.39	6.71	7.95	6.71	4.45	6.71	5.17	2.43	5.55	3.76	0.58	4.45	2.43	-	3.42	1.19	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



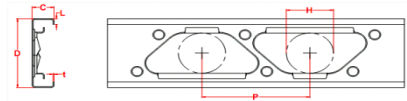
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 2" flange 33 mills - Section Identification: 362D200-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.03	4.03	4.03	3.50	3.33	3.00	3.00	2.68	2.07	2.52	2.07	1.22	2.07	1.50	0.44	1.64	0.96	-	1.22	0.44	-	0.82	-	-
9	3.90	3.90	3.90	3.23	3.02	2.62	2.62	2.24	1.52	2.05	1.52	0.56	1.52	0.87	-	1.03	0.26	-	0.56	-	-	0.11	-	-
10	3.76	3.76	3.76	2.94	2.69	2.22	2.22	1.78	0.99	1.58	0.99	-	0.99	0.27	-	0.44	-	-	-	-	-	-	-	-
11	3.61	3.61	3.61	2.62	2.34	1.82	1.82	1.34	0.49	1.12	0.49	-	0.49	-	-	-	-	-	-	-	-	-	-	-
12	3.44	3.44	3.44	2.30	1.99	1.43	1.43	0.93	0.05	0.70	0.05	-	0.05	-	-	-	-	-	-	-	-	-	-	-
13	3.24	3.24	3.24	1.99	1.66	1.07	1.07	0.56	-	0.32	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14	3.03	3.03	3.03	1.68	1.34	0.75	0.75	0.23	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	2.81	2.81	2.81	1.39	1.05	0.47	0.47	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	2.58	2.58	2.58	1.14	0.80	0.23	0.23	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.38	2.38	2.38	0.91	0.58	0.02	0.02	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.19	2.19	2.19	0.72	0.40	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.03	2.03	2.03	0.55	0.24	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	1.88	1.88	1.88	0.41	0.11	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

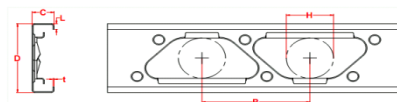


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 2" flange 43 mills - Section Identification: 362D200-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	5.77	5.77	5.77	5.21	5.03	4.68	4.68	4.34	3.69	4.18	3.69	2.77	3.69	3.07	1.91	3.22	2.48	1.10	2.77	1.91	0.33	2.33	1.37	-
9	5.56	5.56	5.56	4.85	4.62	4.19	4.19	3.78	3.01	3.58	3.01	1.95	3.01	2.29	0.98	2.47	1.62	0.08	1.95	0.98	-	1.45	0.38	-
10	5.32	5.32	5.32	4.44	4.18	3.67	3.67	3.20	2.33	2.97	2.33	1.17	2.33	1.54	0.13	1.73	0.81	-	1.17	0.13	-	0.63	-	-
11	5.06	5.06	5.06	4.02	3.71	3.14	3.14	2.62	1.69	2.38	1.69	0.46	1.69	0.85	-	1.05	0.09	-	0.46	-	-	-	-	-
12	4.78	4.78	4.78	3.58	3.24	2.63	2.63	2.08	1.10	1.82	1.10	-	1.10	0.24	-	0.45	-	-	-	-	-	-	-	-
13	4.48	4.48	4.48	3.15	2.79	2.15	2.15	1.58	0.58	1.31	0.58	-	0.58	-	-	-	-	-	-	-	-	-	-	-
14	4.18	4.18	4.18	2.73	2.36	1.71	1.71	1.13	0.14	0.87	0.14	-	0.14	-	-	-	-	-	-	-	-	-	-	-
15	3.88	3.88	3.88	2.34	1.97	1.31	1.31	0.75	-	0.49	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.56	3.56	3.56	1.98	1.61	0.97	0.97	0.42	-	0.17	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.27	3.27	3.27	1.67	1.31	0.68	0.68	0.14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.02	3.02	3.02	1.40	1.04	0.43	0.43	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.79	2.79	2.79	1.16	0.82	0.23	0.23	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.57	2.57	2.57	0.96	0.62	0.05	0.05	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



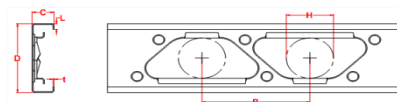
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 2" flange 54 mills - Section Identification: 362D200-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	8.90	8.90	8.90	8.38	8.21	7.88	7.88	7.56	6.93	7.40	6.93	6.04	6.93	6.33	5.21	6.48	5.76	4.41	6.04	5.21	3.66	5.62	4.67	2.93
9	8.41	8.41	8.41	7.74	7.53	7.12	7.12	6.73	5.99	6.54	5.99	4.96	5.99	5.29	4.01	5.46	4.63	3.13	4.96	4.01	2.30	4.48	3.42	1.51
10	7.86	7.86	7.86	7.05	6.80	6.32	6.32	5.87	5.03	5.65	5.03	3.90	5.03	4.27	2.89	4.45	3.55	1.95	3.90	2.89	1.09	3.38	2.26	0.27
11	7.27	7.27	7.27	6.32	6.04	5.51	5.51	5.02	4.12	4.78	4.12	2.93	4.12	3.31	1.89	3.51	2.57	0.94	2.93	1.89	0.06	2.40	1.24	-
12	6.65	6.65	6.65	5.59	5.28	4.72	4.72	4.20	3.28	3.96	3.28	2.08	3.28	2.46	1.03	2.66	1.72	0.09	2.08	1.03	-	1.54	0.40	-
13	6.07	6.07	6.07	4.91	4.59	4.00	4.00	3.48	2.55	3.23	2.55	1.36	2.55	1.73	0.33	1.93	1.00	-	1.36	0.33	-	0.83	-	-
14	5.55	5.55	5.55	4.30	3.97	3.38	3.38	2.85	1.93	2.61	1.93	0.76	1.93	1.13	-	1.32	0.41	-	0.76	-	-	0.25	-	-
15	5.05	5.05	5.05	3.75	3.41	2.82	2.82	2.30	1.40	2.06	1.40	0.27	1.40	0.62	-	0.81	-	-	0.27	-	-	-	-	-
16	4.57	4.57	4.57	3.25	2.92	2.34	2.34	1.83	0.96	1.60	0.96	-	0.96	0.21	-	0.39	-	-	-	-	-	-	-	-
17	4.16	4.16	4.16	2.82	2.49	1.93	1.93	1.43	0.59	1.21	0.59	-	0.59	-	-	0.04	-	-	-	-	-	-	-	-
18	3.79	3.79	3.79	2.44	2.13	1.58	1.58	1.10	0.29	0.88	0.29	-	0.29	-	-	-	-	-	-	-	-	-	-	-
19	3.47	3.47	3.47	2.12	1.81	1.28	1.28	0.82	0.04	0.61	0.04	-	0.04	-	-	-	-	-	-	-	-	-	-	-
20	3.18	3.18	3.18	1.84	1.54	1.02	1.02	0.58	-	0.38	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

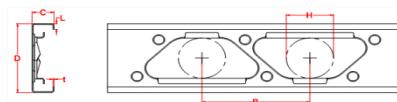


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 2" flange 68 mills - Section Identification: 362D200-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	11.78	11.78	11.78	11.28	11.11	10.79	10.79	10.48	9.87	10.32	9.87	8.99	9.87	9.28	8.16	9.43	8.71	7.37	8.99	8.16	6.61	8.57	7.63	5.88
9	11.10	11.10	11.10	10.46	10.25	9.85	9.85	9.47	8.73	9.28	8.73	7.69	8.73	8.03	6.74	8.20	7.37	5.84	7.69	6.74	4.99	7.21	6.13	4.18
10	10.29	10.29	10.29	9.51	9.26	8.79	8.79	8.34	7.51	8.12	7.51	6.37	7.51	6.74	5.32	6.93	6.01	4.37	6.37	5.32	3.47	5.84	4.68	2.63
11	9.41	9.41	9.41	8.50	8.22	7.70	7.70	7.21	6.31	6.98	6.31	5.11	6.31	5.50	4.04	5.70	4.75	3.06	5.11	4.04	2.16	4.57	3.38	1.31
12	8.49	8.49	8.49	7.48	7.19	6.63	6.63	6.12	5.20	5.88	5.20	3.98	5.20	4.37	2.91	4.57	3.61	1.94	3.98	2.91	1.06	3.43	2.26	0.23
13	7.66	7.66	7.66	6.57	6.26	5.68	5.68	5.16	4.23	4.92	4.23	3.03	4.23	3.41	1.98	3.61	2.67	1.04	3.03	1.98	0.17	2.49	1.34	-
14	6.91	6.91	6.91	5.75	5.43	4.85	4.85	4.33	3.41	4.09	3.41	2.24	3.41	2.61	1.21	2.80	1.88	0.30	2.24	1.21	-	1.71	0.60	-
15	6.25	6.25	6.25	5.04	4.72	4.14	4.14	3.62	2.72	3.38	2.72	1.58	2.72	1.94	0.59	2.13	1.23	-	1.58	0.59	-	1.07	0.00	-
16	5.66	5.66	5.66	4.42	4.10	3.53	3.53	3.02	2.14	2.79	2.14	1.04	2.14	1.38	0.09	1.57	0.71	-	1.04	0.09	-	0.55	-	-
17	5.14	5.14	5.14	3.88	3.56	3.00	3.00	2.51	1.66	2.28	1.66	0.59	1.66	0.92	-	1.10	0.27	-	0.59	-	-	0.12	-	-
18	4.68	4.68	4.68	3.41	3.10	2.55	2.55	2.07	1.25	1.85	1.25	0.22	1.25	0.54	-	0.71	-	-	0.22	-	-	-	-	-
19	4.28	4.28	4.28	3.00	2.69	2.16	2.16	1.70	0.90	1.48	0.90	-	0.90	0.23	-	0.39	-	-	-	-	-	-	-	-
20	3.92	3.92	3.92	2.65	2.35	1.83	1.83	1.38	0.61	1.17	0.61	-	0.61	-	-	0.12	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



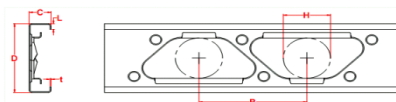
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 2" flange 33 mils - Section Identification: 400D200-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.24	4.24	4.24	3.75	3.60	3.29	3.29	2.99	2.41	2.84	2.41	1.59	2.41	1.86	0.82	1.99	1.33	0.08	1.59	0.82	-	1.20	0.32	-
9	4.14	4.14	4.14	3.52	3.32	2.94	2.94	2.57	1.88	2.40	1.88	0.92	1.88	1.23	0.03	1.39	0.62	-	0.92	0.03	-	0.47	-	-
10	4.02	4.02	4.02	3.25	3.02	2.56	2.56	2.14	1.34	1.93	1.34	0.27	1.34	0.62	-	0.79	-	-	0.27	-	-	-	-	-
11	3.89	3.89	3.89	2.97	2.69	2.17	2.17	1.70	0.83	1.47	0.83	-	0.83	0.04	-	0.23	-	-	-	-	-	-	-	-
12	3.74	3.74	3.74	2.67	2.36	1.79	1.79	1.27	0.35	1.03	0.35	-	0.35	-	-	-	-	-	-	-	-	-	-	-
13	3.58	3.58	3.58	2.36	2.02	1.42	1.42	0.87	-	0.62	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14	3.41	3.41	3.41	2.06	1.70	1.07	1.07	0.51	-	0.25	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.22	3.22	3.22	1.76	1.40	0.75	0.75	0.19	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.01	3.01	3.01	1.48	1.11	0.47	0.47	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.80	2.80	2.80	1.23	0.86	0.23	0.23	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.59	2.59	2.59	1.00	0.63	0.02	0.02	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.40	2.40	2.40	0.80	0.44	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.23	2.23	2.23	0.62	0.28	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



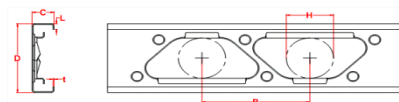
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 2" flange 43 mils - Section Identification: 400D200-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	6.15	6.15	6.15	5.63	5.47	5.14	5.14	4.82	4.19	4.66	4.19	3.31	4.19	3.60	2.46	3.74	3.02	1.66	3.31	2.46	0.90	2.88	1.92	0.15
9	5.97	5.97	5.97	5.31	5.10	4.69	4.69	4.30	3.55	4.11	3.55	2.50	3.55	2.84	1.52	3.01	2.16	0.61	2.50	1.52	-	2.00	0.91	-
10	5.77	5.77	5.77	4.95	4.70	4.21	4.21	3.74	2.88	3.52	2.88	1.69	2.88	2.07	0.62	2.27	1.33	-	1.69	0.62	-	1.15	-	-
11	5.54	5.54	5.54	4.56	4.26	3.70	3.70	3.17	2.22	2.92	2.22	0.94	2.22	1.35	-	1.56	0.55	-	0.94	-	-	0.36	-	-
12	5.29	5.29	5.29	4.14	3.81	3.18	3.18	2.61	1.59	2.35	1.59	0.26	1.59	0.68	-	0.90	-	-	0.26	-	-	-	-	-
13	5.03	5.03	5.03	3.72	3.35	2.69	2.69	2.08	1.02	1.80	1.02	-	1.02	0.09	-	0.31	-	-	-	-	-	-	-	-
14	4.75	4.75	4.75	3.30	2.91	2.22	2.22	1.59	0.52	1.31	0.52	-	0.52	-	-	-	-	-	-	-	-	-	-	-
15	4.47	4.47	4.47	2.90	2.49	1.78	1.78	1.16	0.08	0.87	0.08	-	0.08	-	-	-	-	-	-	-	-	-	-	-
16	4.18	4.18	4.18	2.52	2.11	1.39	1.39	0.77	-	0.49	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.89	3.89	3.89	2.16	1.76	1.05	1.05	0.44	-	0.16	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.59	3.59	3.59	1.84	1.44	0.75	0.75	0.15	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.33	3.33	3.33	1.56	1.16	0.49	0.49	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.08	3.08	3.08	1.31	0.93	0.27	0.27	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



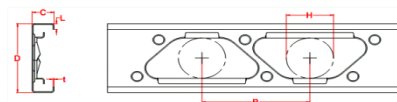
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 2" flange 54 mils - Section Identification: 400D200-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	9.77	9.77	9.77	9.28	9.12	8.80	8.80	8.49	7.89	8.34	7.89	7.01	7.89	7.30	6.18	7.45	6.73	5.39	7.01	6.18	4.62	6.59	5.65	3.88
9	9.35	9.35	9.35	8.72	8.52	8.12	8.12	7.73	6.99	7.54	6.99	5.96	6.99	6.29	4.99	6.47	5.63	4.07	5.96	4.99	3.21	5.46	4.37	2.38
10	8.87	8.87	8.87	8.09	7.84	7.36	7.36	6.91	6.05	6.69	6.05	4.88	6.05	5.26	3.81	5.45	4.52	2.82	4.88	3.81	1.89	4.34	3.14	1.01
11	8.34	8.34	8.34	7.41	7.12	6.57	6.57	6.06	5.12	5.81	5.12	3.85	5.12	4.26	2.72	4.46	3.46	1.68	3.85	2.72	0.72	3.27	2.02	-
12	7.78	7.78	7.78	6.70	6.37	5.78	5.78	5.23	4.23	4.96	4.23	2.90	4.23	3.32	1.74	3.54	2.50	0.69	2.90	1.74	-	2.31	1.03	-
13	7.19	7.19	7.19	5.98	5.64	5.00	5.00	4.42	3.40	4.15	3.40	2.06	3.40	2.48	0.90	2.71	1.66	-	2.06	0.90	-	1.47	0.20	-
14	6.59	6.59	6.59	5.29	4.93	4.28	4.28	3.69	2.66	3.42	2.66	1.34	2.66	1.76	0.20	1.97	0.95	-	1.34	0.20	-	0.76	-	-
15	6.05	6.05	6.05	4.67	4.30	3.64	3.64	3.05	2.03	2.78	2.03	0.74	2.03	1.15	-	1.36	0.36	-	0.74	-	-	0.17	-	-
16	5.56	5.56	5.56	4.11	3.74	3.08	3.08	2.50	1.49	2.23	1.49	0.24	1.49	0.63	-	0.84	-	-	0.24	-	-	-	-	-
17	5.12	5.12	5.12	3.62	3.24	2.59	2.59	2.02	1.04	1.76	1.04	-	1.04	0.21	-	0.41	-	-	-	-	-	-	-	-
18	4.72	4.72	4.72	3.18	2.81	2.17	2.17	1.61	0.66	1.36	0.66	-	0.66	-	-	0.04	-	-	-	-	-	-	-	-
19	4.34	4.34	4.34	2.79	2.43	1.80	1.80	1.26	0.34	1.01	0.34	-	0.34	-	-	-	-	-	-	-	-	-	-	-
20	3.98	3.98	3.98	2.44	2.08	1.48	1.48	0.95	0.07	0.71	0.07	-	0.07	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



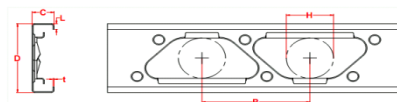
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 2" flange 68 mils - Section Identification: 400D200-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	13.01	13.01	13.01	12.54	12.38	12.08	12.08	11.77	11.19	11.63	11.19	10.33	11.19	10.61	9.51	10.76	10.06	8.72	10.33	9.51	7.96	9.92	8.98	7.22
9	12.43	12.43	12.43	11.82	11.62	11.24	11.24	10.86	10.13	10.67	10.13	9.10	10.13	9.43	8.12	9.60	8.76	7.20	9.10	8.12	6.32	8.60	7.50	5.48
10	11.78	11.78	11.78	11.01	10.77	10.30	10.30	9.85	8.99	9.63	8.99	7.80	8.99	8.19	6.71	8.38	7.43	5.69	7.80	6.71	4.73	7.25	6.02	3.82
11	11.06	11.06	11.06	10.14	9.85	9.30	9.30	8.78	7.83	8.54	7.83	6.53	7.83	6.95	5.36	7.17	6.13	4.27	6.53	5.36	3.27	5.93	4.62	2.33
12	10.29	10.29	10.29	9.21	8.89	8.29	8.29	7.72	6.69	7.45	6.69	5.33	6.69	5.76	4.11	5.99	4.91	3.01	5.33	4.11	1.99	4.71	3.36	1.04
13	9.48	9.48	9.48	8.27	7.92	7.27	7.27	6.68	5.62	6.40	5.62	4.23	5.62	4.67	3.00	4.90	3.81	1.91	4.23	3.00	0.90	3.60	2.26	-
14	8.61	8.61	8.61	7.31	6.95	6.28	6.28	5.68	4.62	5.40	4.62	3.25	4.62	3.68	2.05	3.90	2.83	0.98	3.25	2.05	0.00	2.63	1.33	-
15	7.80	7.80	7.80	6.44	6.07	5.40	5.40	4.80	3.75	4.52	3.75	2.41	3.75	2.84	1.26	3.06	2.01	0.22	2.41	1.26	-	1.81	0.56	-
16	7.08	7.08	7.08	5.67	5.30	4.64	4.64	4.05	3.02	3.77	3.02	1.72	3.02	2.13	0.61	2.34	1.33	-	1.72	0.61	-	1.15	-	-
17	6.44	6.44	6.44	5.00	4.63	3.98	3.98	3.40	2.40	3.13	2.40	1.15	2.40	1.54	0.07	1.75	0.77	-	1.15	0.07	-	0.59	-	-
18	5.87	5.87	5.87	4.41	4.05	3.41	3.41	2.85	1.88	2.59	1.88	0.67	1.88	1.05	-	1.24	0.31	-	0.67	-	-	0.14	-	-
19	5.36	5.36	5.36	3.90	3.54	2.92	2.92	2.37	1.44	2.12	1.44	0.27	1.44	0.64	-	0.83	-	-	0.27	-	-	-	-	-
20	4.92	4.92	4.92	3.45	3.10	2.49	2.49	1.97	1.06	1.72	1.06	-	1.06	0.29	-	0.48	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



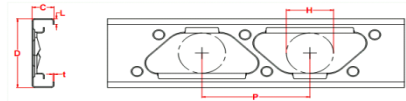
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 2" flange 33 mils - Section Identification: 600D200-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.80	4.80	4.80	4.48	4.37	4.16	4.16	3.95	3.53	3.85	3.53	2.91	3.53	3.12	2.31	3.22	2.71	1.71	2.91	2.31	1.12	2.61	1.91	0.55
9	4.77	4.77	4.77	4.36	4.22	3.96	3.96	3.68	3.16	3.55	3.16	2.39	3.16	2.64	1.64	2.77	2.14	0.92	2.39	1.64	0.21	2.02	1.16	-
10	4.73	4.73	4.73	4.22	4.05	3.71	3.71	3.39	2.75	3.22	2.75	1.82	2.75	2.13	0.94	2.28	1.53	0.08	1.82	0.94	-	1.38	0.36	-
11	4.68	4.68	4.68	4.05	3.85	3.45	3.45	3.06	2.30	2.87	2.30	1.23	2.30	1.58	0.21	1.76	0.89	-	1.23	0.21	-	0.71	-	-
12	4.62	4.62	4.62	3.87	3.63	3.16	3.16	2.70	1.84	2.48	1.84	0.62	1.84	1.02	-	1.22	0.24	-	0.62	-	-	0.05	-	-
13	4.55	4.55	4.55	3.67	3.39	2.85	2.85	2.33	1.36	2.08	1.36	0.02	1.36	0.46	-	0.68	-	-	0.02	-	-	-	-	-
14	4.47	4.47	4.47	3.45	3.13	2.52	2.52	1.95	0.89	1.68	0.89	-	0.89	-	-	0.15	-	-	-	-	-	-	-	-
15	4.38	4.38	4.38	3.21	2.86	2.19	2.19	1.57	0.43	1.27	0.43	-	0.43	-	-	-	-	-	-	-	-	-	-	-
16	4.28	4.28	4.28	2.96	2.58	1.86	1.86	1.19	-	0.88	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	4.16	4.16	4.16	2.71	2.30	1.53	1.53	0.83	-	0.50	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	4.05	4.05	4.05	2.46	2.02	1.21	1.21	0.49	-	0.15	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.92	3.92	3.92	2.21	1.74	0.91	0.91	0.17	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.79	3.79	3.79	1.96	1.48	0.63	0.63	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

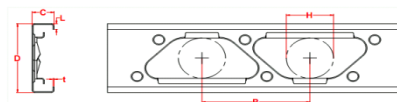


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 2" flange 43 mils - Section Identification: 600D200-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	7.19	7.19	7.19	6.85	6.74	6.51	6.51	6.29	5.84	6.18	5.84	5.18	5.84	5.40	4.52	5.50	4.96	3.89	5.18	4.52	3.26	4.86	4.10	2.64
9	7.13	7.13	7.13	6.70	6.55	6.26	6.26	5.97	5.41	5.83	5.41	4.58	5.41	4.85	3.77	4.99	4.31	2.99	4.58	3.77	2.22	4.17	3.25	1.46
10	7.06	7.06	7.06	6.51	6.33	5.97	5.97	5.62	4.93	5.44	4.93	3.93	4.93	4.26	2.96	4.42	3.60	2.03	3.93	2.96	1.13	3.44	2.34	0.25
11	6.97	6.97	6.97	6.29	6.07	5.64	5.64	5.22	4.40	5.01	4.40	3.23	4.40	3.61	2.11	3.81	2.85	1.05	3.23	2.11	0.03	2.66	1.40	-
12	6.86	6.86	6.86	6.05	5.79	5.28	5.28	4.79	3.84	4.55	3.84	2.50	3.84	2.94	1.26	3.16	2.08	0.07	2.50	1.26	-	1.87	0.46	-
13	6.73	6.73	6.73	5.78	5.47	4.89	4.89	4.32	3.26	4.05	3.26	1.77	3.26	2.25	0.41	2.50	1.31	-	1.77	0.41	-	1.08	-	-
14	6.58	6.58	6.58	5.48	5.13	4.47	4.47	3.84	2.66	3.54	2.66	1.06	2.66	1.58	-	1.84	0.56	-	1.06	-	-	0.32	-	-
15	6.41	6.41	6.41	5.15	4.77	4.04	4.04	3.35	2.08	3.02	2.08	0.38	2.08	0.92	-	1.20	-	-	0.38	-	-	-	-	-
16	6.24	6.24	6.24	4.82	4.39	3.60	3.60	2.86	1.52	2.51	1.52	-	1.52	0.31	-	0.61	-	-	-	-	-	-	-	-
17	6.04	6.04	6.04	4.47	4.01	3.16	3.16	2.38	0.99	2.02	0.99	-	0.99	-	-	0.04	-	-	-	-	-	-	-	-
18	5.84	5.84	5.84	4.12	3.63	2.74	2.74	1.93	0.50	1.55	0.50	-	0.50	-	-	-	-	-	-	-	-	-	-	-
19	5.63	5.63	5.63	3.77	3.25	2.33	2.33	1.50	0.04	1.11	0.04	-	0.04	-	-	-	-	-	-	-	-	-	-	-
20	5.42	5.42	5.42	3.43	2.89	1.94	1.94	1.10	-	0.71	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
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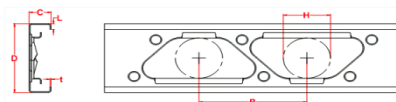
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 2" flange 54 mils - Section Identification: 600D200-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	12.29	12.29	12.29	11.95	11.84	11.62	11.62	11.39	10.94	11.27	10.94	10.27	10.94	10.49	9.62	10.60	10.06	8.98	10.27	9.62	8.34	9.95	9.19	7.71
9	12.14	12.14	12.14	11.70	11.56	11.26	11.26	10.97	10.40	10.83	10.40	9.57	10.40	9.84	8.75	9.98	9.29	7.95	9.57	8.75	7.16	9.15	8.21	6.39
10	11.94	11.94	11.94	11.39	11.21	10.85	10.85	10.49	9.79	10.31	9.79	8.77	9.79	9.10	7.78	9.27	8.43	6.82	8.77	7.78	5.89	8.27	7.14	4.98
11	11.71	11.71	11.71	11.03	10.81	10.37	10.37	9.94	9.10	9.72	9.10	7.89	9.10	8.28	6.74	8.49	7.50	5.63	7.89	6.74	4.57	7.31	5.99	3.54
12	11.43	11.43	11.43	10.61	10.34	9.82	9.82	9.31	8.34	9.07	8.34	6.95	8.34	7.41	5.65	7.64	6.51	4.42	6.95	5.65	3.25	6.29	4.82	2.12
13	11.10	11.10	11.10	10.13	9.82	9.22	9.22	8.64	7.53	8.35	7.53	5.99	7.53	6.49	4.57	6.75	5.50	3.23	5.99	4.57	1.97	5.27	3.67	0.78
14	10.74	10.74	10.74	9.61	9.25	8.57	8.57	7.92	6.70	7.61	6.70	5.03	6.70	5.57	3.51	5.84	4.51	2.10	5.03	3.51	0.78	4.25	2.56	-
15	10.33	10.33	10.33	9.05	8.65	7.89	7.89	7.18	5.87	6.84	5.87	4.10	5.87	4.66	2.51	4.96	3.55	1.05	4.10	2.51	-	3.28	1.52	-
16	9.90	9.90	9.90	8.46	8.02	7.20	7.20	6.44	5.05	6.08	5.05	3.22	5.05	3.80	1.58	4.10	2.65	0.10	3.22	1.58	-	2.38	0.58	-
17	9.44	9.44	9.44	7.85	7.38	6.52	6.52	5.72	4.28	5.35	4.28	2.40	4.28	3.00	0.76	3.31	1.83	-	2.40	0.76	-	1.55	-	-
18	8.97	8.97	8.97	7.25	6.76	5.85	5.85	5.03	3.55	4.64	3.55	1.66	3.55	2.26	0.00	2.57	1.09	-	1.66	0.00	-	0.81	-	-
19	8.49	8.49	8.49	6.65	6.14	5.21	5.21	4.37	2.89	3.98	2.89	1.00	2.89	1.60	-	1.91	0.43	-	1.00	-	-	0.15	-	-
20	8.00	8.00	8.00	6.07	5.55	4.60	4.60	3.76	2.29	3.37	2.29	0.42	2.29	1.00	-	1.31	-	-	0.42	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



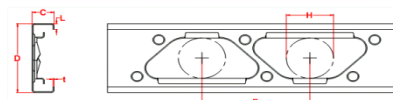
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 2" flange 68 mils - Section Identification: 600D200-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	16.55	16.55	16.55	16.23	16.13	15.92	15.92	15.71	15.27	15.60	15.27	14.65	15.27	14.85	14.02	14.96	14.44	13.41	14.65	14.02	12.80	14.33	13.61	12.20
9	16.34	16.34	16.34	15.94	15.80	15.51	15.51	15.24	14.70	15.11	14.70	13.90	14.70	14.16	13.11	14.30	13.64	12.34	13.90	13.11	11.58	13.51	12.60	10.83
10	16.09	16.09	16.09	15.56	15.39	15.04	15.04	14.70	14.03	14.53	14.03	13.04	14.03	13.37	12.08	13.53	12.72	11.14	13.04	12.08	10.23	12.56	11.45	9.34
11	15.77	15.77	15.77	15.12	14.91	14.49	14.49	14.07	13.25	13.86	13.25	12.07	13.25	12.46	10.94	12.66	11.69	9.84	12.07	10.94	8.78	11.50	10.20	7.76
12	15.40	15.40	15.40	14.61	14.35	13.84	13.84	13.35	12.39	13.11	12.39	11.02	12.39	11.47	9.72	11.69	10.58	8.48	11.02	9.72	7.29	10.36	8.89	6.15
13	14.96	14.96	14.96	14.02	13.71	13.12	13.12	12.55	11.45	12.27	11.45	9.90	11.45	10.40	8.46	10.66	9.41	7.10	9.90	8.46	5.81	9.17	7.54	4.58
14	14.47	14.47	14.47	13.36	13.01	12.33	12.33	11.68	10.45	11.37	10.45	8.75	10.45	9.30	7.19	9.58	8.22	5.74	8.75	7.19	4.37	7.96	6.21	3.08
15	13.92	13.92	13.92	12.65	12.25	11.49	11.49	10.77	9.43	10.43	9.43	7.61	9.43	8.20	5.96	8.50	7.05	4.44	7.61	5.96	3.03	6.77	4.94	1.70
16	13.34	13.34	13.34	11.90	11.46	10.62	10.62	9.86	8.43	9.49	8.43	6.51	8.43	7.13	4.80	7.44	5.92	3.24	6.51	4.80	1.80	5.63	3.75	0.45
17	12.72	12.72	12.72	11.12	10.64	9.76	9.76	8.94	7.44	8.54	7.44	5.47	7.44	6.10	3.73	6.42	4.86	2.15	5.47	3.73	0.70	4.57	2.66	-
18	12.09	12.09	12.09	10.34	9.83	8.89	8.89	8.04	6.50	7.63	6.50	4.50	6.50	5.14	2.75	5.46	3.89	1.17	4.50	2.75	-	3.60	1.68	-
19	11.43	11.43	11.43	9.55	9.02	8.05	8.05	7.18	5.62	6.77	5.62	3.61	5.62	4.25	1.87	4.58	3.01	0.31	3.61	1.87	-	2.71	0.81	-
20	10.78	10.78	10.78	8.79	8.24	7.25	7.25	6.36	4.81	5.95	4.81	2.81	4.81	3.44	1.09	3.77	2.22	-	2.81	1.09	-	1.93	0.05	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

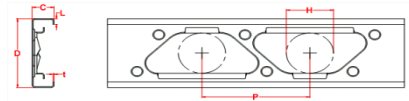


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 2" flange 33 mils - Section Identification: 800D200-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.93	4.93	4.93	4.69	4.61	4.46	4.46	4.30	3.98	4.22	3.98	3.52	3.98	3.67	3.05	3.75	3.36	2.59	3.52	3.05	2.13	3.28	2.74	1.68
9	4.93	4.93	4.93	4.63	4.53	4.33	4.33	4.13	3.73	4.03	3.73	3.14	3.73	3.33	2.55	3.43	2.94	1.97	3.14	2.55	1.40	2.84	2.16	0.83
10	4.93	4.93	4.93	4.55	4.43	4.18	4.18	3.93	3.44	3.81	3.44	2.71	3.44	2.95	1.99	3.07	2.47	1.29	2.71	1.99	0.60	2.35	1.52	-
11	4.93	4.93	4.93	4.47	4.32	4.01	4.01	3.71	3.12	3.56	3.12	2.24	3.12	2.53	1.39	2.68	1.96	0.56	2.24	1.39	-	1.81	0.83	-
12	4.92	4.92	4.92	4.37	4.18	3.82	3.82	3.46	2.76	3.28	2.76	1.74	2.76	2.07	0.75	2.24	1.40	-	1.74	0.75	-	1.24	0.10	-
13	4.89	4.89	4.89	4.24	4.02	3.60	3.60	3.18	2.37	2.97	2.37	1.20	2.37	1.58	0.08	1.78	0.82	-	1.20	0.08	-	0.63	-	-
14	4.86	4.86	4.86	4.09	3.85	3.36	3.36	2.88	1.96	2.64	1.96	0.64	1.96	1.07	-	1.29	0.22	-	0.64	-	-	0.01	-	-
15	4.82	4.82	4.82	3.94	3.65	3.10	3.10	2.56	1.53	2.30	1.53	0.08	1.53	0.55	-	0.79	-	-	0.08	-	-	-	-	-
16	4.77	4.77	4.77	3.77	3.45	2.83	2.83	2.23	1.10	1.94	1.10	-	1.10	0.03	-	0.29	-	-	-	-	-	-	-	-
17	4.72	4.72	4.72	3.58	3.23	2.54	2.54	1.89	0.66	1.57	0.66	-	0.66	-	-	-	-	-	-	-	-	-	-	-
18	4.65	4.65	4.65	3.39	3.00	2.25	2.25	1.54	0.23	1.20	0.23	-	0.23	-	-	-	-	-	-	-	-	-	-	-
19	4.58	4.58	4.58	3.18	2.76	1.95	1.95	1.20	-	0.84	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.50	4.50	4.50	2.97	2.51	1.65	1.65	0.86	-	0.48	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

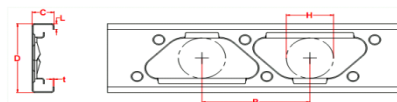


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 2" flange 43 mils - Section Identification: 800D200-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	7.44	7.44	7.44	7.19	7.10	6.94	6.94	6.77	6.44	6.69	6.44	5.95	6.44	6.11	5.46	6.20	5.78	4.97	5.95	5.46	4.49	5.70	5.13	4.00
9	7.44	7.44	7.44	7.12	7.01	6.80	6.80	6.59	6.17	6.48	6.17	5.54	6.17	5.75	4.92	5.85	5.33	4.31	5.54	4.92	3.70	5.23	4.51	3.09
10	7.44	7.44	7.44	7.04	6.91	6.64	6.64	6.38	5.86	6.25	5.86	5.09	5.86	5.34	4.32	5.47	4.83	3.57	5.09	4.32	2.83	4.70	3.82	2.10
11	7.44	7.44	7.44	6.95	6.79	6.47	6.47	6.15	5.51	5.99	5.51	4.57	5.51	4.88	3.66	5.04	4.27	2.77	4.57	3.66	1.89	4.12	3.06	1.02
12	7.43	7.43	7.43	6.84	6.65	6.25	6.25	5.87	5.12	5.68	5.12	4.01	5.12	4.38	2.94	4.56	3.65	1.91	4.01	2.94	0.89	3.48	2.25	-
13	7.38	7.38	7.38	6.68	6.45	5.99	5.99	5.54	4.67	5.32	4.67	3.40	4.67	3.82	2.19	4.03	2.99	1.00	3.40	2.19	-	2.78	1.39	-
14	7.32	7.32	7.32	6.50	6.23	5.70	5.70	5.19	4.19	4.94	4.19	2.76	4.19	3.23	1.40	3.46	2.29	0.09	2.76	1.40	-	2.07	0.52	-
15	7.24	7.24	7.24	6.29	5.99	5.39	5.39	4.81	3.69	4.52	3.69	2.09	3.69	2.61	0.60	2.88	1.59	-	2.09	0.60	-	1.34	-	-
16	7.15	7.15	7.15	6.07	5.72	5.05	5.05	4.40	3.16	4.09	3.16	1.43	3.16	1.99	-	2.28	0.88	-	1.43	-	-	0.61	-	-
17	7.04	7.04	7.04	5.82	5.44	4.69	4.69	3.98	2.63	3.63	2.63	0.77	2.63	1.37	-	1.68	0.18	-	0.77	-	-	-	-	-
18	6.92	6.92	6.92	5.56	5.14	4.32	4.32	3.55	2.10	3.17	2.10	0.12	2.10	0.76	-	1.09	-	-	0.12	-	-	-	-	-
19	6.79	6.79	6.79	5.29	4.82	3.94	3.94	3.11	1.58	2.71	1.58	-	1.58	0.17	-	0.52	-	-	-	-	-	-	-	-
20	6.64	6.64	6.64	5.00	4.50	3.56	3.56	2.68	1.07	2.26	1.07	-	1.07	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



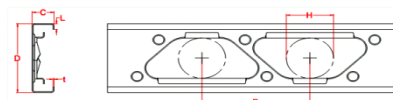
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 2" flange 54 mils - Section Identification: 800D200-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	12.83	12.83	12.83	12.58	12.50	12.33	12.33	12.17	11.84	12.09	11.84	11.36	11.84	11.52	10.87	11.60	11.19	10.39	11.36	10.87	9.91	11.12	10.55	9.43
9	12.83	12.83	12.83	12.51	12.40	12.19	12.19	11.99	11.57	11.88	11.57	10.95	11.57	11.15	10.33	11.26	10.74	9.72	10.95	10.33	9.10	10.64	9.92	8.50
10	12.83	12.83	12.83	12.43	12.30	12.03	12.03	11.77	11.25	11.64	11.25	10.46	11.25	10.72	9.70	10.85	10.21	8.94	10.46	9.70	8.19	10.08	9.19	7.45
11	12.83	12.83	12.83	12.34	12.18	11.85	11.85	11.52	10.88	11.36	10.88	9.93	10.88	10.25	9.00	10.40	9.62	8.09	9.93	9.00	7.19	9.47	8.39	6.31
12	12.83	12.83	12.83	12.23	12.03	11.63	11.63	11.24	10.47	11.05	10.47	9.34	10.47	9.71	8.24	9.90	8.97	7.17	9.34	8.24	6.12	8.79	7.52	5.09
13	12.72	12.72	12.72	12.00	11.76	11.30	11.30	10.84	9.93	10.61	9.93	8.62	9.93	9.05	7.36	9.27	8.20	6.13	8.62	7.36	4.94	7.98	6.53	3.78
14	12.55	12.55	12.55	11.71	11.44	10.90	10.90	10.36	9.33	10.10	9.33	7.84	9.33	8.32	6.41	8.57	7.35	5.04	7.84	6.41	3.72	7.11	5.49	2.45
15	12.36	12.36	12.36	11.38	11.07	10.45	10.45	9.84	8.67	9.54	8.67	7.01	8.67	7.55	5.43	7.83	6.47	3.94	7.01	5.43	2.51	6.21	4.43	1.14
16	12.12	12.12	12.12	11.01	10.65	9.95	9.95	9.27	7.97	8.94	7.97	6.15	7.97	6.74	4.45	7.05	5.57	2.84	6.15	4.45	1.33	5.28	3.37	-
17	11.85	11.85	11.85	10.59	10.19	9.41	9.41	8.67	7.25	8.30	7.25	5.29	7.25	5.92	3.47	6.25	4.67	1.78	5.29	3.47	0.19	4.36	2.33	-
18	11.54	11.54	11.54	10.14	9.69	8.84	8.84	8.03	6.51	7.64	6.51	4.43	6.51	5.10	2.53	5.45	3.78	0.77	4.43	2.53	-	3.46	1.35	-
19	11.21	11.21	11.21	9.66	9.18	8.26	8.26	7.39	5.78	6.97	5.78	3.60	5.78	4.30	1.63	4.66	2.93	-	3.60	1.63	-	2.60	0.41	-
20	10.85	10.85	10.85	9.16	8.64	7.66	7.66	6.74	5.06	6.31	5.06	2.81	5.06	3.53	0.80	3.90	2.12	-	2.81	0.80	-	1.78	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
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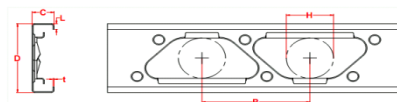
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 2" flange 68 mils - Section Identification: 800D200-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	17.26	17.26	17.26	17.04	16.96	16.81	16.81	16.66	16.36	16.58	16.36	15.90	16.36	16.05	15.45	16.13	15.75	15.01	15.90	15.45	14.56	15.68	15.15	14.11
9	17.26	17.26	17.26	16.97	16.87	16.68	16.68	16.49	16.10	16.39	16.10	15.52	16.10	15.71	14.94	15.81	15.33	14.37	15.52	14.94	13.80	15.23	14.56	13.22
10	17.26	17.26	17.26	16.90	16.77	16.53	16.53	16.29	15.80	16.16	15.80	15.08	15.80	15.32	14.34	15.44	14.82	13.63	15.08	14.34	12.92	14.70	13.86	12.22
11	17.26	17.26	17.26	16.81	16.66	16.36	16.36	16.04	15.44	15.89	15.44	14.55	15.44	14.85	13.67	15.00	14.26	12.81	14.55	13.67	11.96	14.11	13.10	11.11
12	17.26	17.26	17.26	16.71	16.53	16.15	16.15	15.78	15.05	15.60	15.05	13.98	15.05	14.34	12.93	14.51	13.63	11.90	13.98	12.93	10.89	13.45	12.24	9.90
13	17.20	17.20	17.20	16.53	16.30	15.86	15.86	15.42	14.56	15.21	14.56	13.30	14.56	13.72	12.07	13.93	12.89	10.88	13.30	12.07	9.72	12.68	11.27	8.58
14	16.99	16.99	16.99	16.19	15.93	15.41	15.41	14.90	13.90	14.65	13.90	12.46	13.90	12.93	11.06	13.17	11.98	9.71	12.46	11.06	8.41	11.75	10.16	7.15
15	16.72	16.72	16.72	15.80	15.49	14.90	14.90	14.31	13.18	14.02	13.18	11.54	13.18	12.08	9.99	12.35	11.02	8.50	11.54	9.99	7.07	10.76	8.99	5.69
16	16.41	16.41	16.41	15.34	15.00	14.32	14.32	13.66	12.39	13.34	12.39	10.58	12.39	11.17	8.88	11.47	10.00	7.26	10.58	8.88	5.72	9.72	7.79	4.25
17	16.05	16.05	16.05	14.83	14.44	13.69	13.69	12.95	11.55	12.59	11.55	9.58	11.55	10.22	7.75	10.55	8.96	6.03	9.58	7.75	4.40	8.65	6.59	2.85
18	15.64	15.64	15.64	14.27	13.84	13.00	13.00	12.20	10.68	11.81	10.68	8.57	10.68	9.25	6.63	9.60	7.91	4.82	8.57	6.63	3.12	7.58	5.41	1.52
19	15.19	15.19	15.19	13.68	13.20	12.29	12.29	11.42	9.79	11.00	9.79	7.56	9.79	8.28	5.54	8.65	6.87	3.67	7.56	5.54	1.92	6.53	4.28	0.27
20	14.71	14.71	14.71	13.05	12.53	11.55	11.55	10.62	8.90	10.17	8.90	6.59	8.90	7.33	4.50	7.71	5.87	2.58	6.59	4.50	0.80	5.52	3.21	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



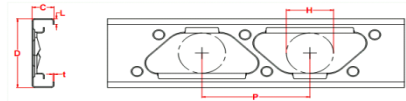
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 2-1/2" flange 33 mills - Section Identification: 362D250-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.32	4.32	4.32	3.79	3.63	3.30	3.30	2.97	2.36	2.82	2.36	1.49	2.36	1.78	0.69	1.92	1.22	-	1.49	0.69	-	1.08	0.17	-
9	4.19	4.19	4.19	3.52	3.31	2.91	2.91	2.52	1.79	2.33	1.79	0.80	1.79	1.12	-	1.28	0.48	-	0.80	-	-	0.33	-	-
10	4.04	4.04	4.04	3.23	2.98	2.50	2.50	2.06	1.23	1.84	1.23	0.13	1.23	0.49	-	0.67	-	-	0.13	-	-	-	-	-
11	3.89	3.89	3.89	2.91	2.62	2.09	2.09	1.59	0.71	1.36	0.71	-	0.71	-	-	0.10	-	-	-	-	-	-	-	-
12	3.71	3.71	3.71	2.58	2.27	1.68	1.68	1.16	0.23	0.91	0.23	-	0.23	-	-	-	-	-	-	-	-	-	-	-
13	3.53	3.53	3.53	2.26	1.92	1.30	1.30	0.76	-	0.50	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14	3.34	3.34	3.34	1.95	1.59	0.96	0.96	0.40	-	0.15	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.14	3.14	3.14	1.65	1.29	0.65	0.65	0.09	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	2.94	2.94	2.94	1.38	1.01	0.37	0.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.74	2.74	2.74	1.13	0.77	0.14	0.14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.54	2.54	2.54	0.91	0.55	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.35	2.35	2.35	0.72	0.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.18	2.18	2.18	0.55	0.21	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



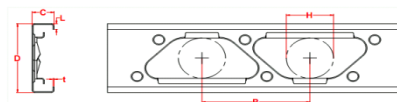
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 2-1/2" flange 43 mils - Section Identification: 362D250-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	6.43	6.43	6.43	5.86	5.68	5.31	5.31	4.96	4.28	4.79	4.28	3.32	4.28	3.63	2.41	3.79	3.01	1.56	3.32	2.41	0.74	2.86	1.84	-
9	6.25	6.25	6.25	5.51	5.28	4.83	4.83	4.40	3.58	4.19	3.58	2.45	3.58	2.82	1.41	3.00	2.10	0.45	2.45	1.41	-	1.92	0.77	-
10	6.04	6.04	6.04	5.13	4.84	4.30	4.30	3.80	2.86	3.56	2.86	1.60	2.86	2.01	0.47	2.22	1.22	-	1.60	0.47	-	1.03	-	-
11	5.78	5.78	5.78	4.68	4.35	3.74	3.74	3.18	2.16	2.91	2.16	0.81	2.16	1.24	-	1.46	0.41	-	0.81	-	-	0.20	-	-
12	5.46	5.46	5.46	4.19	3.83	3.17	3.17	2.56	1.49	2.28	1.49	0.11	1.49	0.55	-	0.77	-	-	0.11	-	-	-	-	-
13	5.13	5.13	5.13	3.71	3.32	2.62	2.62	1.99	0.90	1.71	0.90	-	0.90	-	-	0.18	-	-	-	-	-	-	-	-
14	4.79	4.79	4.79	3.24	2.84	2.12	2.12	1.49	0.39	1.20	0.39	-	0.39	-	-	-	-	-	-	-	-	-	-	-
15	4.45	4.45	4.45	2.80	2.39	1.67	1.67	1.04	-	0.75	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	4.10	4.10	4.10	2.39	1.98	1.27	1.27	0.66	-	0.38	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.77	3.77	3.77	2.03	1.63	0.93	0.93	0.33	-	0.06	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.47	3.47	3.47	1.71	1.32	0.64	0.64	0.06	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.21	3.21	3.21	1.44	1.06	0.40	0.40	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.97	2.97	2.97	1.20	0.83	0.19	0.19	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



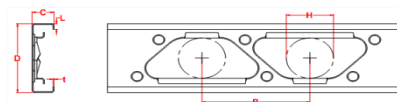
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 2-1/2" flange 54 mills - Section Identification: 362D250-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	9.88	9.88	9.88	9.35	9.17	8.83	8.83	8.50	7.85	8.34	7.85	6.93	7.85	7.23	6.06	7.39	6.64	5.22	6.93	6.06	4.42	6.49	5.50	3.66
9	9.44	9.44	9.44	8.76	8.54	8.11	8.11	7.70	6.92	7.50	6.92	5.83	6.92	6.18	4.81	6.36	5.48	3.87	5.83	4.81	2.97	5.31	4.18	2.12
10	8.99	8.99	8.99	8.13	7.86	7.35	7.35	6.87	5.96	6.63	5.96	4.73	5.96	5.13	3.62	5.33	4.35	2.60	4.73	3.62	1.64	4.16	2.93	0.75
11	8.32	8.32	8.32	7.32	7.01	6.44	6.44	5.91	4.94	5.66	4.94	3.63	4.94	4.05	2.48	4.27	3.24	1.43	3.63	2.48	0.47	3.04	1.77	-
12	7.62	7.62	7.62	6.48	6.16	5.55	5.55	4.99	3.98	4.72	3.98	2.65	3.98	3.07	1.50	3.29	2.25	0.46	2.65	1.50	-	2.06	0.79	-
13	6.95	6.95	6.95	5.72	5.37	4.73	4.73	4.16	3.14	3.89	3.14	1.82	3.14	2.24	0.68	2.45	1.43	-	1.82	0.68	-	1.24	-	-
14	6.35	6.35	6.35	5.03	4.67	4.02	4.02	3.44	2.42	3.17	2.42	1.13	2.42	1.54	0.01	1.75	0.74	-	1.13	0.01	-	0.56	-	-
15	5.79	5.79	5.79	4.40	4.04	3.39	3.39	2.81	1.82	2.55	1.82	0.56	1.82	0.95	-	1.16	0.18	-	0.56	-	-	0.00	-	-
16	5.26	5.26	5.26	3.83	3.47	2.83	2.83	2.27	1.30	2.01	1.30	0.08	1.30	0.46	-	0.66	-	-	0.08	-	-	-	-	-
17	4.79	4.79	4.79	3.34	2.98	2.35	2.35	1.81	0.87	1.56	0.87	-	0.87	0.06	-	0.26	-	-	-	-	-	-	-	-
18	4.37	4.37	4.37	2.91	2.56	1.95	1.95	1.42	0.51	1.17	0.51	-	0.51	-	-	-	-	-	-	-	-	-	-	-
19	4.00	4.00	4.00	2.54	2.19	1.60	1.60	1.09	0.21	0.85	0.21	-	0.21	-	-	-	-	-	-	-	-	-	-	-
20	3.67	3.67	3.67	2.21	1.88	1.30	1.30	0.80	-	0.58	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

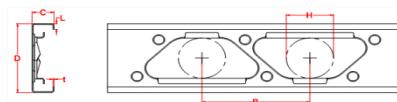
DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 2-1/2" flange 68 mills - Section Identification: 362D250-68

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	13.38	13.38	13.38	12.82	12.64	12.28	12.28	11.93	11.24	11.76	11.24	10.26	11.24	10.58	9.33	10.75	9.95	8.43	10.26	9.33	7.57	9.79	8.73	6.74
9	12.60	12.60	12.60	11.89	11.66	11.21	11.21	10.78	9.95	10.57	9.95	8.79	9.95	9.17	7.71	9.36	8.42	6.69	8.79	7.71	5.73	8.24	7.03	4.82
10	11.70	11.70	11.70	10.83	10.56	10.03	10.03	9.53	8.59	9.29	8.59	7.30	8.59	7.71	6.12	7.93	6.90	5.04	7.30	6.12	4.02	6.70	5.39	3.06
11	10.72	10.72	10.72	9.71	9.40	8.81	8.81	8.27	7.25	8.00	7.25	5.89	7.25	6.33	4.66	6.55	5.47	3.55	5.89	4.66	2.51	5.26	3.91	1.54
12	9.70	9.70	9.70	8.57	8.24	7.62	7.62	7.04	5.99	6.77	5.99	4.61	5.99	5.05	3.38	5.28	4.18	2.27	4.61	3.38	1.25	3.98	2.63	0.30
13	8.76	8.76	8.76	7.54	7.19	6.54	6.54	5.95	4.89	5.67	4.89	3.52	4.89	3.95	2.31	4.18	3.10	1.22	3.52	2.31	0.23	2.89	1.57	-
14	7.92	7.92	7.92	6.63	6.26	5.60	5.60	5.01	3.96	4.73	3.96	2.61	3.96	3.03	1.43	3.25	2.19	0.38	2.61	1.43	-	2.00	0.72	-
15	7.17	7.17	7.17	5.82	5.45	4.79	4.79	4.20	3.17	3.93	3.17	1.84	3.17	2.26	0.71	2.48	1.45	-	1.84	0.71	-	1.26	0.02	-
16	6.51	6.51	6.51	5.11	4.75	4.09	4.09	3.51	2.50	3.24	2.50	1.22	2.50	1.62	0.12	1.83	0.84	-	1.22	0.12	-	0.65	-	-
17	5.92	5.92	5.92	4.50	4.13	3.49	3.49	2.92	1.94	2.66	1.94	0.70	1.94	1.09	-	1.29	0.34	-	0.70	-	-	0.16	-	-
18	5.41	5.41	5.41	3.96	3.60	2.97	2.97	2.42	1.46	2.16	1.46	0.28	1.46	0.65	-	0.84	-	-	0.28	-	-	-	-	-
19	4.95	4.95	4.95	3.49	3.14	2.52	2.52	1.99	1.06	1.74	1.06	-	1.06	0.28	-	0.46	-	-	-	-	-	-	-	-
20	4.54	4.54	4.54	3.08	2.74	2.14	2.14	1.62	0.73	1.38	0.73	-	0.73	-	-	0.15	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

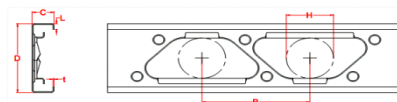


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 2-1/2" flange 33 mills - Section Identification: 400D250-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.55	4.55	4.55	4.06	3.91	3.60	3.60	3.29	2.71	3.15	2.71	1.87	2.71	2.14	1.07	2.28	1.60	0.32	1.87	1.07	-	1.47	0.57	-
9	4.44	4.44	4.44	3.82	3.63	3.24	3.24	2.87	2.16	2.69	2.16	1.17	2.16	1.49	0.26	1.66	0.86	-	1.17	0.26	-	0.71	-	-
10	4.32	4.32	4.32	3.56	3.32	2.86	2.86	2.42	1.61	2.21	1.61	0.49	1.61	0.85	-	1.03	0.15	-	0.49	-	-	-	-	-
11	4.18	4.18	4.18	3.26	2.98	2.46	2.46	1.96	1.06	1.73	1.06	-	1.06	0.24	-	0.44	-	-	-	-	-	-	-	-
12	4.03	4.03	4.03	2.96	2.64	2.05	2.05	1.52	0.55	1.26	0.55	-	0.55	-	-	-	-	-	-	-	-	-	-	-
13	3.87	3.87	3.87	2.64	2.29	1.66	1.66	1.09	0.08	0.83	0.08	-	0.08	-	-	-	-	-	-	-	-	-	-	-
14	3.70	3.70	3.70	2.33	1.96	1.29	1.29	0.70	-	0.43	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.52	3.52	3.52	2.03	1.64	0.95	0.95	0.35	-	0.07	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.34	3.34	3.34	1.74	1.34	0.65	0.65	0.04	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.15	3.15	3.15	1.47	1.07	0.37	0.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.96	2.96	2.96	1.22	0.82	0.14	0.14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.77	2.77	2.77	1.00	0.60	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.58	2.58	2.58	0.80	0.41	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
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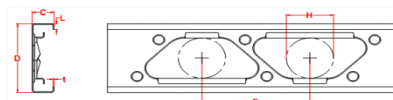
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 2-1/2" flange 43 mills - Section Identification: 400D250-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	6.77	6.77	6.77	6.25	6.08	5.74	5.74	5.41	4.77	5.25	4.77	3.85	4.77	4.15	2.98	4.31	3.56	2.14	3.85	2.98	1.33	3.41	2.41	0.55
9	6.62	6.62	6.62	5.95	5.73	5.31	5.31	4.90	4.11	4.70	4.11	3.01	4.11	3.37	1.98	3.55	2.66	1.02	3.01	1.98	0.10	2.49	1.33	-
10	6.45	6.45	6.45	5.61	5.34	4.83	4.83	4.34	3.42	4.10	3.42	2.16	3.42	2.57	1.01	2.78	1.77	-	2.16	1.01	-	1.58	0.29	-
11	6.25	6.25	6.25	5.23	4.91	4.31	4.31	3.75	2.72	3.49	2.72	1.34	2.72	1.78	0.10	2.01	0.92	-	1.34	0.10	-	0.71	-	-
12	6.04	6.04	6.04	4.82	4.45	3.78	3.78	3.16	2.05	2.87	2.05	0.58	2.05	1.05	-	1.29	0.13	-	0.58	-	-	-	-	-
13	5.76	5.76	5.76	4.35	3.96	3.23	3.23	2.57	1.41	2.27	1.41	-	1.41	0.38	-	0.62	-	-	-	-	-	-	-	-
14	5.45	5.45	5.45	3.89	3.46	2.70	2.70	2.02	0.83	1.70	0.83	-	0.83	-	-	0.04	-	-	-	-	-	-	-	-
15	5.13	5.13	5.13	3.43	2.99	2.21	2.21	1.52	0.33	1.20	0.33	-	0.33	-	-	-	-	-	-	-	-	-	-	-
16	4.80	4.80	4.80	3.00	2.55	1.76	1.76	1.07	-	0.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	4.48	4.48	4.48	2.60	2.15	1.36	1.36	0.69	-	0.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	4.14	4.14	4.14	2.23	1.78	1.01	1.01	0.35	-	0.05	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.83	3.83	3.83	1.90	1.46	0.71	0.71	0.07	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.55	3.55	3.55	1.61	1.18	0.45	0.45	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



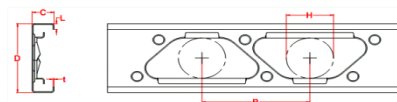
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 2-1/2" flange 54 mills - Section Identification: 400D250-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	10.57	10.57	10.57	10.08	9.92	9.61	9.61	9.30	8.69	9.14	8.69	7.80	8.69	8.09	6.96	8.24	7.52	6.14	7.80	6.96	5.35	7.38	6.41	4.59
9	10.31	10.31	10.31	9.66	9.46	9.05	9.05	8.66	7.89	8.46	7.89	6.81	7.89	7.17	5.80	7.35	6.47	4.84	6.81	5.80	3.92	6.30	5.15	3.05
10	9.89	9.89	9.89	9.09	8.83	8.33	8.33	7.86	6.96	7.63	6.96	5.72	6.96	6.12	4.57	6.33	5.33	3.51	5.72	4.57	2.51	5.14	3.86	1.56
11	9.43	9.43	9.43	8.45	8.15	7.57	7.57	7.02	6.01	6.76	6.01	4.64	6.01	5.08	3.41	5.30	4.21	2.27	4.64	3.41	1.22	4.01	2.64	0.23
12	8.93	8.93	8.93	7.77	7.42	6.77	6.77	6.17	5.07	5.88	5.07	3.61	5.07	4.08	2.33	4.32	3.17	1.16	3.61	2.33	0.08	2.96	1.54	-
13	8.26	8.26	8.26	6.96	6.59	5.90	5.90	5.26	4.13	4.97	4.13	2.65	4.13	3.12	1.36	3.37	2.21	0.20	2.65	1.36	-	1.99	0.58	-
14	7.57	7.57	7.57	6.17	5.78	5.06	5.06	4.42	3.28	4.12	3.28	1.82	3.28	2.28	0.55	2.51	1.38	-	1.82	0.55	-	1.16	-	-
15	6.95	6.95	6.95	5.45	5.05	4.33	4.33	3.68	2.55	3.38	2.55	1.11	2.55	1.57	-	1.80	0.68	-	1.11	-	-	0.47	-	-
16	6.38	6.38	6.38	4.82	4.41	3.68	3.68	3.04	1.93	2.75	1.93	0.53	1.93	0.97	-	1.20	0.11	-	0.53	-	-	-	-	-
17	5.87	5.87	5.87	4.25	3.84	3.12	3.12	2.49	1.40	2.20	1.40	0.04	1.40	0.47	-	0.69	-	-	0.04	-	-	-	-	-
18	5.41	5.41	5.41	3.75	3.34	2.63	2.63	2.02	0.96	1.73	0.96	-	0.96	0.05	-	0.27	-	-	-	-	-	-	-	-
19	5.00	5.00	5.00	3.31	2.91	2.21	2.21	1.61	0.58	1.33	0.58	-	0.58	-	-	-	-	-	-	-	-	-	-	-
20	4.59	4.59	4.59	2.90	2.51	1.83	1.83	1.25	0.25	0.98	0.25	-	0.25	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



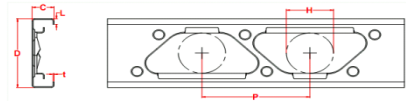
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 2-1/2" flange 68 mills - Section Identification: 400D250-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	14.61	14.61	14.61	14.09	13.92	13.58	13.58	13.25	12.59	13.08	12.59	11.64	12.59	11.96	10.73	12.11	11.33	9.84	11.64	10.73	8.99	11.18	10.14	8.16
9	14.12	14.12	14.12	13.44	13.22	12.78	12.78	12.36	11.54	12.15	11.54	10.37	11.54	10.75	9.27	10.94	9.99	8.22	10.37	9.27	7.22	9.81	8.56	6.26
10	13.41	13.41	13.41	12.56	12.28	11.75	11.75	11.24	10.27	10.99	10.27	8.93	10.27	9.36	7.68	9.59	8.50	6.52	8.93	7.68	5.43	8.29	6.90	4.39
11	12.59	12.59	12.59	11.56	11.24	10.62	10.62	10.04	8.96	9.75	8.96	7.48	8.96	7.96	6.14	8.20	7.02	4.91	7.48	6.14	3.76	6.80	5.31	2.68
12	11.72	11.72	11.72	10.52	10.15	9.48	9.48	8.84	7.67	8.53	7.67	6.11	7.67	6.61	4.72	6.87	5.63	3.46	6.11	4.72	2.29	5.40	3.87	1.20
13	10.82	10.82	10.82	9.47	9.08	8.34	8.34	7.67	6.45	7.35	6.45	4.87	6.45	5.37	3.46	5.63	4.38	2.20	4.87	3.46	1.05	4.15	2.61	-
14	9.86	9.86	9.86	8.40	7.99	7.23	7.23	6.54	5.32	6.22	5.32	3.75	5.32	4.25	2.37	4.50	3.27	1.14	3.75	2.37	0.01	3.03	1.54	-
15	8.94	8.94	8.94	7.41	6.99	6.22	6.22	5.54	4.33	5.22	4.33	2.79	4.33	3.28	1.46	3.53	2.32	0.27	2.79	1.46	-	2.10	0.65	-
16	8.12	8.12	8.12	6.54	6.11	5.35	5.35	4.67	3.49	4.36	3.49	1.99	3.49	2.47	0.70	2.71	1.55	-	1.99	0.70	-	1.33	-	-
17	7.40	7.40	7.40	5.77	5.35	4.60	4.60	3.93	2.78	3.63	2.78	1.33	2.78	1.78	0.09	2.03	0.90	-	1.33	0.09	-	0.69	-	-
18	6.75	6.75	6.75	5.10	4.68	3.94	3.94	3.30	2.18	3.00	2.18	0.78	2.18	1.22	-	1.45	0.37	-	0.78	-	-	0.16	-	-
19	6.18	6.18	6.18	4.51	4.10	3.38	3.38	2.75	1.67	2.46	1.67	0.32	1.67	0.74	-	0.96	-	-	0.32	-	-	-	-	-
20	5.67	5.67	5.67	4.00	3.60	2.89	2.89	2.28	1.23	2.00	1.23	-	1.23	0.34	-	0.55	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



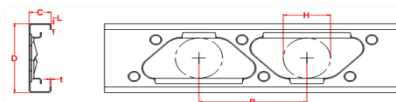
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 2-1/2" flange 33 mills - Section Identification: 600D250-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	5.17	5.17	5.17	4.84	4.74	4.52	4.52	4.30	3.87	4.19	3.87	3.24	3.87	3.45	2.61	3.55	3.03	2.00	3.24	2.61	1.39	2.92	2.20	0.79
9	5.13	5.13	5.13	4.71	4.58	4.30	4.30	4.03	3.48	3.89	3.48	2.69	3.48	2.95	1.92	3.08	2.44	1.17	2.69	1.92	0.43	2.31	1.42	-
10	5.08	5.08	5.08	4.56	4.39	4.05	4.05	3.71	3.05	3.55	3.05	2.10	3.05	2.42	1.19	2.57	1.80	0.30	2.10	1.19	-	1.64	0.59	-
11	5.03	5.03	5.03	4.39	4.18	3.77	3.77	3.37	2.59	3.17	2.59	1.49	2.59	1.85	0.43	2.03	1.13	-	1.49	0.43	-	0.95	-	-
12	4.96	4.96	4.96	4.20	3.95	3.47	3.47	3.00	2.11	2.78	2.11	0.85	2.11	1.26	-	1.47	0.45	-	0.85	-	-	0.25	-	-
13	4.88	4.88	4.88	3.99	3.70	3.15	3.15	2.62	1.61	2.36	1.61	0.22	1.61	0.67	-	0.90	-	-	0.22	-	-	-	-	-
14	4.80	4.80	4.80	3.76	3.43	2.81	2.81	2.22	1.11	1.93	1.11	-	1.11	0.09	-	0.34	-	-	-	-	-	-	-	-
15	4.70	4.70	4.70	3.52	3.15	2.46	2.46	1.82	0.63	1.51	0.63	-	0.63	-	-	-	-	-	-	-	-	-	-	-
16	4.59	4.59	4.59	3.26	2.86	2.11	2.11	1.42	0.16	1.09	0.16	-	0.16	-	-	-	-	-	-	-	-	-	-	-
17	4.48	4.48	4.48	3.00	2.57	1.77	1.77	1.04	-	0.69	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	4.36	4.36	4.36	2.74	2.28	1.44	1.44	0.67	-	0.31	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	4.23	4.23	4.23	2.48	1.99	1.11	1.11	0.33	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.10	4.10	4.10	2.22	1.72	0.81	0.81	0.01	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



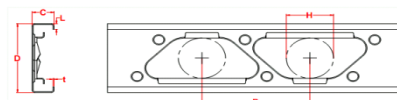
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 2-1/2" flange 43 mills - Section Identification: 600D250-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	7.72	7.72	7.72	7.37	7.26	7.03	7.03	6.80	6.35	6.69	6.35	5.68	6.35	5.90	5.01	6.01	5.45	4.36	5.68	5.01	3.72	5.35	4.58	3.08
9	7.66	7.66	7.66	7.22	7.07	6.78	6.78	6.49	5.91	6.34	5.91	5.07	5.91	5.35	4.24	5.49	4.79	3.44	5.07	4.24	2.65	4.65	3.70	1.87
10	7.59	7.59	7.59	7.04	6.85	6.49	6.49	6.13	5.43	5.95	5.43	4.40	5.43	4.74	3.41	4.91	4.07	2.45	4.40	3.41	1.52	3.90	2.77	0.61
11	7.51	7.51	7.51	6.83	6.60	6.16	6.16	5.73	4.90	5.52	4.90	3.69	4.90	4.09	2.54	4.29	3.30	1.44	3.69	2.54	0.37	3.11	1.80	-
12	7.41	7.41	7.41	6.59	6.33	5.81	5.81	5.30	4.33	5.05	4.33	2.94	4.33	3.39	1.64	3.62	2.50	0.41	2.94	1.64	-	2.28	0.81	-
13	7.30	7.30	7.30	6.33	6.02	5.42	5.42	4.84	3.73	4.55	3.73	2.18	3.73	2.68	0.75	2.94	1.69	-	2.18	0.75	-	1.45	-	-
14	7.17	7.17	7.17	6.04	5.69	5.00	5.00	4.35	3.12	4.03	3.12	1.43	3.12	1.97	-	2.25	0.91	-	1.43	-	-	0.64	-	-
15	7.03	7.03	7.03	5.74	5.33	4.57	4.57	3.85	2.51	3.50	2.51	0.70	2.51	1.28	-	1.58	0.14	-	0.70	-	-	-	-	-
16	6.89	6.89	6.89	5.41	4.96	4.12	4.12	3.34	1.91	2.97	1.91	0.01	1.91	0.62	-	0.93	-	-	0.01	-	-	-	-	-
17	6.73	6.73	6.73	5.07	4.58	3.68	3.68	2.84	1.34	2.45	1.34	-	1.34	-	-	0.31	-	-	-	-	-	-	-	-
18	6.56	6.56	6.56	4.72	4.20	3.23	3.23	2.36	0.79	1.95	0.79	-	0.79	-	-	-	-	-	-	-	-	-	-	-
19	6.38	6.38	6.38	4.37	3.81	2.80	2.80	1.89	0.29	1.47	0.29	-	0.29	-	-	-	-	-	-	-	-	-	-	-
20	6.20	6.20	6.20	4.02	3.44	2.39	2.39	1.46	-	1.02	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



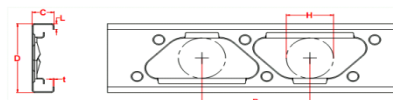
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 2-1/2" flange 54 mills - Section Identification: 600D250-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	12.64	12.64	12.64	12.31	12.20	11.99	11.99	11.77	11.34	11.66	11.34	10.68	11.34	10.90	10.05	11.00	10.47	9.42	10.68	10.05	8.80	10.37	9.63	8.18
9	12.49	12.49	12.49	12.07	11.93	11.65	11.65	11.37	10.82	11.23	10.82	10.00	10.82	10.27	9.20	10.41	9.73	8.42	10.00	9.20	7.65	9.60	8.68	6.89
10	12.31	12.31	12.31	11.78	11.60	11.26	11.26	10.91	10.23	10.74	10.23	9.23	10.23	9.56	8.27	9.73	8.91	7.32	9.23	8.27	6.40	8.75	7.63	5.50
11	12.10	12.10	12.10	11.45	11.23	10.81	10.81	10.39	9.57	10.18	9.57	8.39	9.57	8.78	7.25	8.98	8.01	6.16	8.39	7.25	5.10	7.82	6.52	4.07
12	11.86	11.86	11.86	11.07	10.81	10.31	10.31	9.81	8.86	9.57	8.86	7.49	8.86	7.94	6.19	8.16	7.05	4.95	7.49	6.19	3.77	6.83	5.36	2.63
13	11.58	11.58	11.58	10.64	10.34	9.75	9.75	9.18	8.09	8.90	8.09	6.55	8.09	7.05	5.11	7.30	6.06	3.76	6.55	5.11	2.47	5.82	4.20	1.25
14	11.33	11.33	11.33	10.23	9.88	9.20	9.20	8.55	7.32	8.24	7.32	5.63	7.32	6.17	4.06	6.46	5.09	2.61	5.63	4.06	1.24	4.83	3.08	-
15	11.08	11.08	11.08	9.80	9.40	8.63	8.63	7.90	6.55	7.55	6.55	4.71	6.55	5.30	3.05	5.61	4.14	1.51	4.71	3.05	0.08	3.86	2.01	-
16	10.81	10.81	10.81	9.34	8.89	8.03	8.03	7.24	5.78	6.86	5.78	3.83	5.78	4.45	2.08	4.77	3.23	0.49	3.83	2.08	-	2.93	1.01	-
17	10.50	10.50	10.50	8.83	8.33	7.41	7.41	6.56	5.01	6.15	5.01	2.98	5.01	3.63	1.18	3.96	2.36	-	2.98	1.18	-	2.06	0.08	-
18	10.09	10.09	10.09	8.25	7.72	6.74	6.74	5.85	4.24	5.43	4.24	2.17	4.24	2.83	0.35	3.17	1.54	-	2.17	0.35	-	1.23	-	-
19	9.68	9.68	9.68	7.68	7.12	6.09	6.09	5.17	3.53	4.74	3.53	1.43	3.53	2.10	-	2.44	0.80	-	1.43	-	-	0.50	-	-
20	9.24	9.24	9.24	7.09	6.51	5.46	5.46	4.52	2.86	4.08	2.86	0.77	2.86	1.43	-	1.77	0.14	-	0.77	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



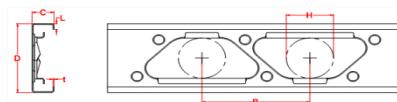
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 2-1/2" flange 68 mills - Section Identification: 600D250-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	18.26	18.26	18.26	17.90	17.79	17.55	17.55	17.32	16.84	17.20	16.84	16.14	16.84	16.37	15.45	16.49	15.91	14.76	16.14	15.45	14.08	15.79	14.99	13.41
9	18.01	18.01	18.01	17.56	17.41	17.09	17.09	16.79	16.19	16.64	16.19	15.30	16.19	15.59	14.43	15.74	15.01	13.57	15.30	14.43	12.72	14.86	13.85	11.89
10	17.72	17.72	17.72	17.13	16.94	16.56	16.56	16.18	15.44	16.00	15.44	14.34	15.44	14.71	13.28	14.89	13.99	12.24	14.34	13.28	11.22	13.81	12.58	10.23
11	17.37	17.37	17.37	16.65	16.42	15.95	15.95	15.49	14.59	15.26	14.59	13.28	14.59	13.71	12.02	13.93	12.86	10.81	13.28	12.02	9.63	12.65	11.21	8.48
12	16.98	16.98	16.98	16.11	15.83	15.27	15.27	14.72	13.66	14.45	13.66	12.14	13.66	12.64	10.69	12.89	11.65	9.31	12.14	10.69	7.99	11.41	9.76	6.71
13	16.55	16.55	16.55	15.51	15.17	14.52	14.52	13.88	12.66	13.57	12.66	10.93	12.66	11.50	9.32	11.78	10.39	7.80	10.93	9.32	6.35	10.12	8.30	4.97
14	16.08	16.08	16.08	14.86	14.47	13.71	13.71	12.99	11.61	12.63	11.61	9.71	11.61	10.32	7.95	10.64	9.11	6.31	9.71	7.95	4.77	8.81	6.85	3.32
15	15.59	15.59	15.59	14.17	13.72	12.87	12.87	12.06	10.55	11.67	10.55	8.49	10.55	9.15	6.62	9.49	7.85	4.90	8.49	6.62	3.29	7.53	5.46	1.77
16	15.09	15.09	15.09	13.45	12.95	12.00	12.00	11.13	9.50	10.71	9.50	7.31	9.50	8.02	5.35	8.37	6.63	3.57	7.31	5.35	1.92	6.31	4.15	0.37
17	14.55	14.55	14.55	12.70	12.15	11.13	11.13	10.18	8.46	9.73	8.46	6.18	8.46	6.91	4.17	7.28	5.48	2.35	6.18	4.17	0.67	5.14	2.94	-
18	13.82	13.82	13.82	11.81	11.22	10.14	10.14	9.15	7.38	8.69	7.38	5.06	7.38	5.81	3.05	6.18	4.37	1.23	5.06	3.05	-	4.03	1.81	-
19	13.09	13.09	13.09	10.92	10.31	9.19	9.19	8.17	6.38	7.70	6.38	4.05	6.38	4.79	2.04	5.17	3.36	0.24	4.05	2.04	-	3.02	0.83	-
20	12.35	12.35	12.35	10.05	9.42	8.27	8.27	7.25	5.45	6.77	5.45	3.14	5.45	3.87	1.15	4.25	2.45	-	3.14	1.15	-	2.12	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



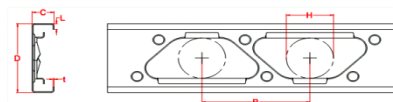
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 2-1/2" flange 33 mills - Section Identification: 800D250-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	5.39	5.39	5.39	5.15	5.06	4.90	4.90	4.74	4.41	4.65	4.41	3.92	4.41	4.08	3.44	4.16	3.76	2.95	3.92	3.44	2.48	3.68	3.11	2.00
9	5.38	5.38	5.38	5.06	4.96	4.75	4.75	4.54	4.13	4.44	4.13	3.51	4.13	3.72	2.90	3.82	3.31	2.30	3.51	2.90	1.70	3.21	2.50	1.11
10	5.36	5.36	5.36	4.97	4.84	4.58	4.58	4.32	3.81	4.20	3.81	3.06	3.81	3.31	2.31	3.43	2.81	1.58	3.06	2.31	0.86	2.69	1.83	0.15
11	5.33	5.33	5.33	4.86	4.70	4.39	4.39	4.08	3.47	3.92	3.47	2.56	3.47	2.86	1.68	3.01	2.27	0.81	2.56	1.68	-	2.12	1.10	-
12	5.30	5.30	5.30	4.74	4.55	4.18	4.18	3.81	3.09	3.62	3.09	2.03	3.09	2.38	1.00	2.55	1.68	0.01	2.03	1.00	-	1.51	0.34	-
13	5.27	5.27	5.27	4.60	4.38	3.94	3.94	3.51	2.67	3.30	2.67	1.47	2.67	1.87	0.31	2.07	1.08	-	1.47	0.31	-	0.88	-	-
14	5.22	5.22	5.22	4.44	4.19	3.69	3.69	3.20	2.25	2.95	2.25	0.89	2.25	1.33	-	1.56	0.45	-	0.89	-	-	0.23	-	-
15	5.17	5.17	5.17	4.27	3.98	3.41	3.41	2.86	1.80	2.59	1.80	0.30	1.80	0.79	-	1.04	-	-	0.30	-	-	-	-	-
16	5.11	5.11	5.11	4.09	3.76	3.13	3.13	2.52	1.35	2.22	1.35	-	1.35	0.24	-	0.51	-	-	-	-	-	-	-	-
17	5.05	5.05	5.05	3.90	3.53	2.83	2.83	2.16	0.89	1.83	0.89	-	0.89	-	-	-	-	-	-	-	-	-	-	-
18	4.98	4.98	4.98	3.70	3.30	2.53	2.53	1.80	0.44	1.45	0.44	-	0.44	-	-	-	-	-	-	-	-	-	-	-
19	4.90	4.90	4.90	3.48	3.05	2.22	2.22	1.44	0.00	1.07	0.00	-	0.00	-	-	-	-	-	-	-	-	-	-	-
20	4.82	4.82	4.82	3.27	2.80	1.91	1.91	1.08	-	0.69	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



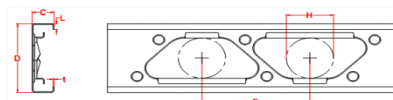
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 2-1/2" flange 43 mills - Section Identification: 800D250-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	8.06	8.06	8.06	7.80	7.72	7.55	7.55	7.38	7.04	7.29	7.04	6.53	7.04	6.70	6.03	6.78	6.36	5.52	6.53	6.03	5.02	6.28	5.69	4.53
9	8.04	8.04	8.04	7.71	7.60	7.39	7.39	7.17	6.74	7.06	6.74	6.10	6.74	6.31	5.46	6.42	5.88	4.83	6.10	5.46	4.20	5.78	5.04	3.58
10	8.01	8.01	8.01	7.60	7.47	7.20	7.20	6.93	6.40	6.80	6.40	5.61	6.40	5.87	4.83	6.00	5.35	4.05	5.61	4.83	3.29	5.21	4.31	2.54
11	7.98	7.98	7.98	7.48	7.32	6.99	6.99	6.66	6.02	6.50	6.02	5.07	6.02	5.38	4.13	5.54	4.75	3.21	5.07	4.13	2.31	4.60	3.52	1.42
12	7.93	7.93	7.93	7.34	7.14	6.75	6.75	6.36	5.59	6.16	5.59	4.47	5.59	4.84	3.39	5.03	4.11	2.32	4.47	3.39	1.28	3.92	2.67	0.26
13	7.88	7.88	7.88	7.18	6.94	6.48	6.48	6.03	5.14	5.80	5.14	3.84	5.14	4.27	2.60	4.48	3.42	1.39	3.84	2.60	0.22	3.21	1.79	-
14	7.81	7.81	7.81	6.99	6.72	6.19	6.19	5.67	4.65	5.41	4.65	3.18	4.65	3.66	1.79	3.91	2.71	0.44	3.18	1.79	-	2.48	0.88	-
15	7.74	7.74	7.74	6.79	6.48	5.87	5.87	5.28	4.13	4.99	4.13	2.50	4.13	3.04	0.96	3.31	1.99	-	2.50	0.96	-	1.73	-	-
16	7.66	7.66	7.66	6.57	6.22	5.54	5.54	4.87	3.60	4.55	3.60	1.81	3.60	2.39	0.14	2.69	1.25	-	1.81	0.14	-	0.96	-	-
17	7.56	7.56	7.56	6.33	5.94	5.18	5.18	4.45	3.06	4.09	3.06	1.13	3.06	1.75	-	2.07	0.51	-	1.13	-	-	0.21	-	-
18	7.46	7.46	7.46	6.08	5.65	4.81	4.81	4.01	2.51	3.62	2.51	0.44	2.51	1.11	-	1.45	-	-	0.44	-	-	-	-	-
19	7.35	7.35	7.35	5.82	5.34	4.43	4.43	3.57	1.96	3.15	1.96	-	1.96	0.49	-	0.85	-	-	-	-	-	-	-	-
20	7.23	7.23	7.23	5.54	5.02	4.04	4.04	3.12	1.43	2.68	1.43	-	1.43	-	-	0.26	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



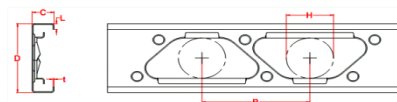
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 2-1/2" flange 54 mills - Section Identification: 800D250-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	13.47	13.47	13.47	13.22	13.14	12.98	12.98	12.81	12.49	12.73	12.49	12.00	12.49	12.17	11.52	12.25	11.84	11.04	12.00	11.52	10.56	11.76	11.20	10.08
9	13.41	13.41	13.41	13.10	12.99	12.79	12.79	12.58	12.16	12.47	12.16	11.54	12.16	11.75	10.93	11.86	11.34	10.32	11.54	10.93	9.71	11.24	10.53	9.11
10	13.34	13.34	13.34	12.95	12.82	12.56	12.56	12.30	11.79	12.17	11.79	11.01	11.79	11.28	10.25	11.41	10.77	9.52	11.01	10.25	8.76	10.63	9.76	8.02
11	13.25	13.25	13.25	12.77	12.61	12.30	12.30	11.99	11.35	11.82	11.35	10.42	11.35	10.73	9.51	10.88	10.11	8.61	10.42	9.51	7.72	9.96	8.91	6.85
12	13.14	13.14	13.14	12.57	12.38	11.99	11.99	11.61	10.86	11.42	10.86	9.76	10.86	10.13	8.69	10.31	9.40	7.64	9.76	8.69	6.61	9.22	7.99	5.60
13	13.00	13.00	13.00	12.31	12.09	11.64	11.64	11.20	10.32	10.98	10.32	9.05	10.32	9.47	7.81	9.68	8.63	6.61	9.05	7.81	5.44	8.42	7.00	4.29
14	12.84	12.84	12.84	12.04	11.77	11.25	11.25	10.74	9.73	10.48	9.73	8.28	9.73	8.76	6.88	9.00	7.81	5.53	8.28	6.88	4.23	7.57	5.98	2.97
15	12.65	12.65	12.65	11.72	11.42	10.82	10.82	10.24	9.10	9.95	9.10	7.47	9.10	8.01	5.92	8.28	6.95	4.44	7.47	5.92	3.01	6.69	4.92	1.65
16	12.44	12.44	12.44	11.38	11.03	10.36	10.36	9.70	8.43	9.38	8.43	6.64	8.43	7.22	4.95	7.52	6.06	3.34	6.64	4.95	1.82	5.78	3.87	0.35
17	12.20	12.20	12.20	11.00	10.61	9.86	9.86	9.13	7.74	8.78	7.74	5.79	7.74	6.42	3.97	6.75	5.17	2.27	5.79	3.97	0.66	4.87	2.83	-
18	11.95	11.95	11.95	10.60	10.17	9.34	9.34	8.54	7.03	8.15	7.03	4.94	7.03	5.62	3.02	5.96	4.29	1.24	4.94	3.02	-	3.97	1.82	-
19	11.67	11.67	11.67	10.17	9.70	8.80	8.80	7.93	6.32	7.52	6.32	4.12	6.32	4.83	2.11	5.19	3.43	0.25	4.12	2.11	-	3.09	0.86	-
20	11.45	11.45	11.45	9.79	9.27	8.29	8.29	7.36	5.64	6.92	5.64	3.33	5.64	4.07	1.25	4.45	2.61	-	3.33	1.25	-	2.26	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



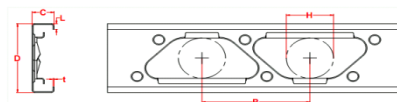
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 2-1/2" flange 68 mills - Section Identification: 800D250-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	19.52	19.52	19.52	19.26	19.17	19.00	19.00	18.83	18.48	18.74	18.48	17.96	18.48	18.14	17.45	18.22	17.79	16.94	17.96	17.45	16.42	17.70	17.10	15.91
9	19.45	19.45	19.45	19.12	19.01	18.78	18.78	18.56	18.12	18.45	18.12	17.46	18.12	17.68	16.80	17.79	17.24	16.15	17.46	16.80	15.50	17.13	16.36	14.85
10	19.36	19.36	19.36	18.95	18.81	18.53	18.53	18.25	17.70	18.12	17.70	16.86	17.70	17.15	16.04	17.29	16.59	15.23	16.86	16.04	14.43	16.45	15.50	13.63
11	19.25	19.25	19.25	18.74	18.57	18.23	18.23	17.89	17.20	17.71	17.20	16.19	17.20	16.53	15.20	16.69	15.86	14.22	16.19	15.20	13.26	15.69	14.55	12.30
12	19.12	19.12	19.12	18.50	18.30	17.87	17.87	17.45	16.64	17.25	16.64	15.44	16.64	15.84	14.26	16.04	15.05	13.11	15.44	14.26	11.98	14.85	13.49	10.87
13	18.92	18.92	18.92	18.17	17.92	17.43	17.43	16.94	15.98	16.70	15.98	14.58	15.98	15.05	13.22	15.28	14.12	11.89	14.58	13.22	10.59	13.90	12.33	9.33
14	18.64	18.64	18.64	17.76	17.47	16.90	16.90	16.34	15.23	16.06	15.23	13.63	15.23	14.15	12.08	14.42	13.10	10.58	13.63	12.08	9.13	12.84	11.07	7.72
15	18.33	18.33	18.33	17.31	16.97	16.31	16.31	15.67	14.41	15.35	14.41	12.61	14.41	13.20	10.88	13.50	12.02	9.23	12.61	10.88	7.64	11.73	9.77	6.11
16	17.98	17.98	17.98	16.80	16.42	15.68	15.68	14.95	13.54	14.59	13.54	11.54	13.54	12.20	9.65	12.53	10.90	7.86	11.54	9.65	6.15	10.59	8.45	4.51
17	17.60	17.60	17.60	16.26	15.83	15.00	15.00	14.19	12.64	13.79	12.64	10.46	12.64	11.16	8.42	11.53	9.76	6.51	10.46	8.42	4.69	9.42	7.13	2.96
18	17.19	17.19	17.19	15.69	15.21	14.28	14.28	13.39	11.70	12.96	11.70	9.36	11.70	10.12	7.20	10.51	8.62	5.19	9.36	7.20	3.29	8.26	5.84	1.50
19	16.77	16.77	16.77	15.09	14.56	13.55	13.55	12.58	10.77	12.11	10.77	8.28	10.77	9.08	6.02	9.49	7.51	3.92	8.28	6.02	1.96	7.13	4.60	0.11
20	16.33	16.33	16.33	14.47	13.90	12.80	12.80	11.76	9.83	11.26	9.83	7.23	9.83	8.07	4.88	8.50	6.42	2.72	7.23	4.88	0.72	6.03	3.43	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



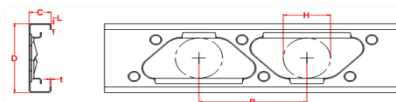
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 3" flange 33 mills - Section Identification: 362D300-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.48	4.48	4.48	3.97	3.81	3.48	3.48	3.16	2.55	3.01	2.55	1.69	2.55	1.97	0.87	2.11	1.41	0.09	1.69	0.87	-	1.27	0.35	-
9	4.35	4.35	4.35	3.71	3.50	3.10	3.10	2.71	1.99	2.53	1.99	0.97	1.99	1.30	0.04	1.47	0.66	-	0.97	0.04	-	0.50	-	-
10	4.21	4.21	4.21	3.41	3.17	2.69	2.69	2.25	1.42	2.03	1.42	0.29	1.42	0.65	-	0.84	-	-	0.29	-	-	-	-	-
11	4.05	4.05	4.05	3.10	2.81	2.28	2.28	1.78	0.87	1.54	0.87	-	0.87	0.05	-	0.25	-	-	-	-	-	-	-	-
12	3.88	3.88	3.88	2.78	2.46	1.87	1.87	1.33	0.37	1.08	0.37	-	0.37	-	-	-	-	-	-	-	-	-	-	-
13	3.70	3.70	3.70	2.45	2.10	1.48	1.48	0.91	-	0.65	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14	3.51	3.51	3.51	2.13	1.77	1.11	1.11	0.53	-	0.27	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.32	3.32	3.32	1.83	1.45	0.79	0.79	0.20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.12	3.12	3.12	1.55	1.16	0.49	0.49	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	2.92	2.92	2.92	1.29	0.90	0.24	0.24	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	2.73	2.73	2.73	1.06	0.67	0.02	0.02	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.56	2.56	2.56	0.85	0.48	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.41	2.41	2.41	0.67	0.30	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



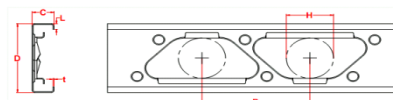
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 3" flange 43 mills - Section Identification: 362D300-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	6.56	6.56	6.56	6.02	5.84	5.50	5.50	5.16	4.51	4.99	4.51	3.57	4.51	3.88	2.68	4.03	3.27	1.83	3.57	2.68	1.03	3.12	2.11	0.24
9	6.38	6.38	6.38	5.69	5.47	5.04	5.04	4.62	3.83	4.42	3.83	2.72	3.83	3.08	1.69	3.26	2.36	0.72	2.72	1.69	-	2.19	1.03	-
10	6.19	6.19	6.19	5.32	5.05	4.54	4.54	4.04	3.12	3.81	3.12	1.87	3.12	2.27	0.73	2.48	1.48	-	1.87	0.73	-	1.29	0.02	-
11	5.97	5.97	5.97	4.92	4.60	4.01	4.01	3.45	2.43	3.18	2.43	1.06	2.43	1.50	-	1.72	0.65	-	1.06	-	-	0.44	-	-
12	5.73	5.73	5.73	4.50	4.14	3.47	3.47	2.86	1.76	2.57	1.76	0.33	1.76	0.79	-	1.02	-	-	0.33	-	-	-	-	-
13	5.48	5.48	5.48	4.06	3.67	2.95	2.95	2.30	1.15	1.99	1.15	-	1.15	0.15	-	0.39	-	-	-	-	-	-	-	-
14	5.21	5.21	5.21	3.63	3.20	2.45	2.45	1.78	0.61	1.47	0.61	-	0.61	-	-	-	-	-	-	-	-	-	-	-
15	4.93	4.93	4.93	3.20	2.76	1.98	1.98	1.30	0.13	0.99	0.13	-	0.13	-	-	-	-	-	-	-	-	-	-	-
16	4.63	4.63	4.63	2.78	2.34	1.56	1.56	0.88	-	0.57	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	4.25	4.25	4.25	2.37	1.93	1.17	1.17	0.51	-	0.21	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.92	3.92	3.92	2.02	1.59	0.84	0.84	0.20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	3.62	3.62	3.62	1.71	1.28	0.56	0.56	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	3.35	3.35	3.35	1.44	1.02	0.32	0.32	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



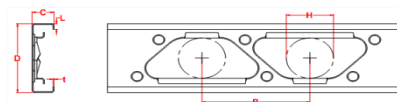
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 3" flange 54 mills - Section Identification: 362D300-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	9.94	9.94	9.94	9.45	9.29	8.97	8.97	8.66	8.05	8.50	8.05	7.18	8.05	7.46	6.34	7.61	6.89	5.53	7.18	6.34	4.76	6.75	5.80	4.01
9	9.52	9.52	9.52	8.89	8.68	8.29	8.29	7.90	7.16	7.71	7.16	6.11	7.16	6.46	5.13	6.63	5.78	4.20	6.11	5.13	3.32	5.62	4.51	2.48
10	9.05	9.05	9.05	8.27	8.02	7.55	7.55	7.09	6.23	6.87	6.23	5.04	6.23	5.42	3.95	5.62	4.67	2.93	5.04	3.95	1.98	4.48	3.26	1.07
11	8.57	8.57	8.57	7.63	7.34	6.79	6.79	6.27	5.31	6.02	5.31	4.01	5.31	4.43	2.84	4.64	3.61	1.77	4.01	2.84	0.78	3.41	2.12	-
12	8.14	8.14	8.14	7.02	6.69	6.08	6.08	5.50	4.46	5.23	4.46	3.08	4.46	3.52	1.86	3.74	2.66	0.76	3.08	1.86	-	2.45	1.12	-
13	7.59	7.59	7.59	6.34	5.98	5.31	5.31	4.71	3.62	4.42	3.62	2.21	3.62	2.66	0.99	2.89	1.79	-	2.21	0.99	-	1.58	0.24	-
14	7.07	7.07	7.07	5.68	5.30	4.60	4.60	3.98	2.87	3.68	2.87	1.46	2.87	1.91	0.24	2.14	1.04	-	1.46	0.24	-	0.83	-	-
15	6.52	6.52	6.52	5.03	4.64	3.93	3.93	3.30	2.21	3.01	2.21	0.82	2.21	1.25	-	1.48	0.41	-	0.82	-	-	0.20	-	-
16	5.93	5.93	5.93	4.40	4.00	3.30	3.30	2.69	1.62	2.40	1.62	0.28	1.62	0.70	-	0.92	-	-	0.28	-	-	-	-	-
17	5.41	5.41	5.41	3.84	3.45	2.76	2.76	2.16	1.13	1.89	1.13	-	1.13	0.24	-	0.45	-	-	-	-	-	-	-	-
18	4.94	4.94	4.94	3.36	2.97	2.30	2.30	1.72	0.72	1.45	0.72	-	0.72	-	-	0.06	-	-	-	-	-	-	-	-
19	4.53	4.53	4.53	2.94	2.56	1.91	1.91	1.34	0.37	1.08	0.37	-	0.37	-	-	-	-	-	-	-	-	-	-	-
20	4.16	4.16	4.16	2.57	2.20	1.57	1.57	1.02	0.08	0.77	0.08	-	0.08	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

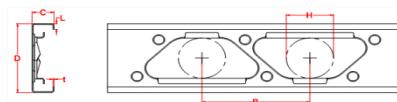
DeltaStud Braced by Bridging Only (Unsheathed) ²

3-5/8" DeltaStud 3" flange 68 mills - Section Identification: 362D300-68

Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	14.04	14.04	14.04	13.51	13.33	12.99	12.99	12.64	11.98	12.48	11.98	11.01	11.98	11.33	10.09	11.49	10.70	9.19	11.01	10.09	8.33	10.54	9.49	7.50
9	13.32	13.32	13.32	12.63	12.41	11.97	11.97	11.55	10.74	11.34	10.74	9.58	10.74	9.96	8.49	10.15	9.21	7.46	9.58	8.49	6.48	9.03	7.80	5.54
10	12.49	12.49	12.49	11.64	11.38	10.86	10.86	10.36	9.41	10.11	9.41	8.10	9.41	8.52	6.89	8.74	7.69	5.77	8.10	6.89	4.72	7.49	6.14	3.72
11	11.60	11.60	11.60	10.60	10.29	9.69	9.69	9.13	8.09	8.86	8.09	6.68	8.09	7.13	5.39	7.37	6.24	4.22	6.68	5.39	3.13	6.02	4.60	2.10
12	10.70	10.70	10.70	9.55	9.20	8.56	8.56	7.95	6.84	7.66	6.84	5.37	6.84	5.84	4.05	6.08	4.92	2.86	5.37	4.05	1.76	4.70	3.25	0.74
13	9.82	9.82	9.82	8.55	8.17	7.48	7.48	6.85	5.70	6.55	5.70	4.21	5.70	4.68	2.89	4.93	3.76	1.71	4.21	2.89	0.63	3.54	2.09	-
14	8.90	8.90	8.90	7.54	7.15	6.44	6.44	5.80	4.66	5.50	4.66	3.18	4.66	3.65	1.90	3.89	2.73	0.74	3.18	1.90	-	2.52	1.12	-
15	8.07	8.07	8.07	6.64	6.25	5.53	5.53	4.89	3.77	4.60	3.77	2.32	3.77	2.78	1.08	3.02	1.89	-	2.32	1.08	-	1.68	0.32	-
16	7.34	7.34	7.34	5.86	5.46	4.75	4.75	4.12	3.01	3.82	3.01	1.61	3.01	2.05	0.41	2.28	1.19	-	1.61	0.41	-	0.99	-	-
17	6.69	6.69	6.69	5.17	4.77	4.07	4.07	3.45	2.37	3.16	2.37	1.01	2.37	1.44	-	1.66	0.61	-	1.01	-	-	0.41	-	-
18	6.11	6.11	6.11	4.57	4.17	3.48	3.48	2.88	1.83	2.60	1.83	0.52	1.83	0.93	-	1.14	0.13	-	0.52	-	-	-	-	-
19	5.60	5.60	5.60	4.04	3.65	2.98	2.98	2.39	1.37	2.12	1.37	0.11	1.37	0.50	-	0.71	-	-	0.11	-	-	-	-	-
20	5.14	5.14	5.14	3.58	3.20	2.54	2.54	1.97	0.98	1.70	0.98	-	0.98	0.15	-	0.35	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



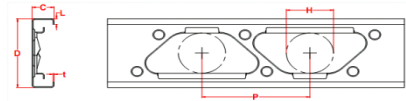
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 3" flange 33 mils - Section Identification: 400D300-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	4.72	4.72	4.72	4.25	4.09	3.79	3.79	3.49	2.90	3.34	2.90	2.06	2.90	2.34	1.27	2.48	1.79	0.49	2.06	1.27	-	1.66	0.75	-
9	4.61	4.61	4.61	4.01	3.81	3.43	3.43	3.06	2.36	2.88	2.36	1.36	2.36	1.68	0.43	1.85	1.04	-	1.36	0.43	-	0.89	-	-
10	4.49	4.49	4.49	3.74	3.51	3.05	3.05	2.61	1.79	2.40	1.79	0.66	1.79	1.03	-	1.21	0.30	-	0.66	-	-	0.13	-	-
11	4.35	4.35	4.35	3.45	3.18	2.65	2.65	2.15	1.23	1.92	1.23	-	1.23	0.39	-	0.60	-	-	-	-	-	-	-	-
12	4.20	4.20	4.20	3.15	2.83	2.24	2.24	1.70	0.70	1.44	0.70	-	0.70	-	-	0.02	-	-	-	-	-	-	-	-
13	4.04	4.04	4.04	2.84	2.49	1.85	1.85	1.26	0.21	0.98	0.21	-	0.21	-	-	-	-	-	-	-	-	-	-	-
14	3.88	3.88	3.88	2.52	2.15	1.46	1.46	0.85	-	0.57	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15	3.70	3.70	3.70	2.21	1.82	1.11	1.11	0.48	-	0.19	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16	3.52	3.52	3.52	1.92	1.51	0.78	0.78	0.15	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17	3.33	3.33	3.33	1.64	1.22	0.49	0.49	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	3.14	3.14	3.14	1.38	0.96	0.23	0.23	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	2.96	2.96	2.96	1.14	0.73	0.01	0.01	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	2.78	2.78	2.78	0.93	0.52	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



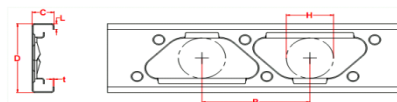
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 3" flange 43 mils - Section Identification: 400D300-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	6.90	6.90	6.90	6.40	6.24	5.92	5.92	5.60	4.98	5.45	4.98	4.09	4.98	4.38	3.23	4.53	3.80	2.40	4.09	3.23	1.61	3.66	2.68	0.84
9	6.75	6.75	6.75	6.11	5.91	5.50	5.50	5.11	4.35	4.91	4.35	3.27	4.35	3.62	2.25	3.80	2.92	1.29	3.27	2.25	0.37	2.75	1.60	-
10	6.59	6.59	6.59	5.79	5.53	5.04	5.04	4.57	3.67	4.34	3.67	2.42	3.67	2.83	1.27	3.03	2.03	0.20	2.42	1.27	-	1.84	0.55	-
11	6.40	6.40	6.40	5.43	5.13	4.55	4.55	4.00	2.98	3.74	2.98	1.60	2.98	2.04	0.35	2.27	1.17	-	1.60	0.35	-	0.96	-	-
12	6.20	6.20	6.20	5.04	4.69	4.03	4.03	3.42	2.30	3.13	2.30	0.82	2.30	1.29	-	1.54	0.36	-	0.82	-	-	0.14	-	-
13	5.98	5.98	5.98	4.63	4.24	3.51	3.51	2.85	1.66	2.54	1.66	0.11	1.66	0.60	-	0.85	-	-	0.11	-	-	-	-	-
14	5.74	5.74	5.74	4.21	3.79	3.00	3.00	2.30	1.06	1.97	1.06	-	1.06	-	-	0.24	-	-	-	-	-	-	-	-
15	5.50	5.50	5.50	3.79	3.34	2.52	2.52	1.79	0.53	1.46	0.53	-	0.53	-	-	-	-	-	-	-	-	-	-	-
16	5.24	5.24	5.24	3.38	2.91	2.06	2.06	1.33	0.05	0.99	0.05	-	0.05	-	-	-	-	-	-	-	-	-	-	-
17	4.98	4.98	4.98	2.98	2.50	1.65	1.65	0.91	-	0.57	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	4.69	4.69	4.69	2.60	2.11	1.26	1.26	0.53	-	0.20	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	4.33	4.33	4.33	2.23	1.75	0.92	0.92	0.21	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.02	4.02	4.02	1.90	1.43	0.62	0.62	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



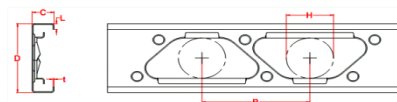
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 3" flange 54 mils - Section Identification: 400D300-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	10.75	10.75	10.75	10.28	10.13	9.83	9.83	9.53	8.95	9.39	8.95	8.10	8.95	8.38	7.28	8.52	7.83	6.49	8.10	7.28	5.72	7.69	6.75	4.97
9	10.39	10.39	10.39	9.79	9.60	9.22	9.22	8.85	8.12	8.66	8.12	7.09	8.12	7.43	6.11	7.60	6.76	5.17	7.09	6.11	4.27	6.59	5.48	3.41
10	9.98	9.98	9.98	9.24	9.00	8.54	8.54	8.09	7.23	7.87	7.23	6.03	7.23	6.42	4.91	6.62	5.65	3.86	6.03	4.91	2.86	5.46	4.20	1.91
11	9.53	9.53	9.53	8.64	8.35	7.81	7.81	7.29	6.31	7.04	6.31	4.97	6.31	5.40	3.75	5.62	4.55	2.61	4.97	3.75	1.55	4.35	2.98	0.55
12	9.05	9.05	9.05	7.99	7.66	7.04	7.04	6.46	5.39	6.18	5.39	3.95	5.39	4.41	2.66	4.65	3.51	1.48	3.95	2.66	0.38	3.29	1.86	-
13	8.59	8.59	8.59	7.36	6.99	6.31	6.31	5.68	4.54	5.38	4.54	3.02	4.54	3.50	1.69	3.75	2.56	0.48	3.02	1.69	-	2.34	0.87	-
14	8.17	8.17	8.17	6.76	6.36	5.62	5.62	4.95	3.75	4.63	3.75	2.19	3.75	2.68	0.83	2.93	1.72	-	2.19	0.83	-	1.49	0.01	-
15	7.63	7.63	7.63	6.09	5.66	4.90	4.90	4.21	2.99	3.88	2.99	1.44	2.99	1.92	0.09	2.18	0.97	-	1.44	0.09	-	0.74	-	-
16	7.13	7.13	7.13	5.47	5.03	4.24	4.24	3.55	2.33	3.22	2.33	0.79	2.33	1.27	-	1.52	0.33	-	0.79	-	-	0.10	-	-
17	6.61	6.61	6.61	4.87	4.42	3.63	3.63	2.94	1.74	2.62	1.74	0.23	1.74	0.71	-	0.95	-	-	0.23	-	-	-	-	-
18	6.09	6.09	6.09	4.30	3.86	3.08	3.08	2.40	1.23	2.09	1.23	-	1.23	0.23	-	0.46	-	-	-	-	-	-	-	-
19	5.63	5.63	5.63	3.81	3.37	2.60	2.60	1.94	0.80	1.63	0.80	-	0.80	-	-	0.05	-	-	-	-	-	-	-	-
20	5.19	5.19	5.19	3.35	2.92	2.17	2.17	1.53	0.43	1.23	0.43	-	0.43	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



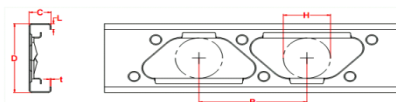
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

4" DeltaStud 3" flange 68 mils - Section Identification: 400D300-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	15.15	15.15	15.15	14.65	14.49	14.16	14.16	13.84	13.21	13.68	13.21	12.29	13.21	12.59	11.39	12.74	11.99	10.52	12.29	11.39	9.68	11.84	10.81	8.86
9	14.69	14.69	14.69	14.04	13.83	13.41	13.41	13.00	12.20	12.80	12.20	11.06	12.20	11.44	9.97	11.63	10.69	8.93	11.06	9.97	7.93	10.51	9.27	6.97
10	14.15	14.15	14.15	13.33	13.06	12.55	12.55	12.04	11.08	11.80	11.08	9.73	11.08	10.17	8.48	10.40	9.31	7.29	9.73	8.48	6.17	9.09	7.68	5.10
11	13.38	13.38	13.38	12.38	12.06	11.46	11.46	10.87	9.78	10.59	9.78	8.28	9.78	8.76	6.90	9.01	7.81	5.63	8.28	6.90	4.44	7.58	6.05	3.31
12	12.58	12.58	12.58	11.40	11.03	10.34	10.34	9.70	8.51	9.39	8.51	6.90	8.51	7.41	5.44	7.68	6.40	4.12	6.90	5.44	2.89	6.16	4.55	1.74
13	11.77	11.77	11.77	10.41	10.02	9.26	9.26	8.56	7.29	8.23	7.29	5.61	7.29	6.15	4.12	6.42	5.10	2.78	5.61	4.12	1.54	4.85	3.21	0.38
14	10.94	10.94	10.94	9.44	9.00	8.20	8.20	7.46	6.15	7.12	6.15	4.45	6.15	4.99	2.96	5.27	3.93	1.62	4.45	2.96	0.39	3.68	2.05	-
15	10.05	10.05	10.05	8.43	7.97	7.15	7.15	6.41	5.09	6.06	5.09	3.41	5.09	3.94	1.94	4.22	2.89	0.63	3.41	1.94	-	2.65	1.05	-
16	9.14	9.14	9.14	7.46	7.00	6.17	6.17	5.44	4.15	5.09	4.15	2.50	4.15	3.02	1.08	3.29	2.01	-	2.50	1.08	-	1.77	0.23	-
17	8.34	8.34	8.34	6.60	6.14	5.32	5.32	4.60	3.34	4.27	3.34	1.74	3.34	2.24	0.38	2.51	1.27	-	1.74	0.38	-	1.04	-	-
18	7.62	7.62	7.62	5.85	5.40	4.59	4.59	3.88	2.66	3.55	2.66	1.11	2.66	1.59	-	1.84	0.65	-	1.11	-	-	0.43	-	-
19	6.98	6.98	6.98	5.19	4.74	3.95	3.95	3.26	2.07	2.94	2.07	0.58	2.07	1.05	-	1.29	0.14	-	0.58	-	-	-	-	-
20	6.42	6.42	6.42	4.61	4.17	3.40	3.40	2.73	1.57	2.42	1.57	0.14	1.57	0.59	-	0.82	-	-	0.14	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



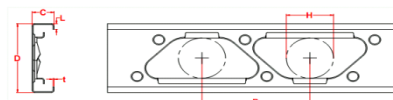
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 3" flange 33 mils - Section Identification: 600D300-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	5.38	5.38	5.38	5.05	4.94	4.72	4.72	4.51	4.07	4.40	4.07	3.43	4.07	3.65	2.80	3.75	3.22	2.18	3.43	2.80	1.57	3.12	2.39	0.96
9	5.34	5.34	5.34	4.92	4.78	4.50	4.50	4.23	3.68	4.09	3.68	2.89	3.68	3.15	2.10	3.28	2.62	1.34	2.89	2.10	0.59	2.49	1.59	-
10	5.28	5.28	5.28	4.76	4.59	4.24	4.24	3.91	3.25	3.74	3.25	2.28	3.25	2.60	1.36	2.76	1.98	0.46	2.28	1.36	-	1.82	0.75	-
11	5.22	5.22	5.22	4.58	4.38	3.97	3.97	3.56	2.78	3.36	2.78	1.66	2.78	2.02	0.58	2.21	1.29	-	1.66	0.58	-	1.11	-	-
12	5.15	5.15	5.15	4.39	4.14	3.66	3.66	3.19	2.29	2.96	2.29	1.01	2.29	1.42	-	1.64	0.60	-	1.01	-	-	0.40	-	-
13	5.07	5.07	5.07	4.18	3.89	3.34	3.34	2.80	1.78	2.54	1.78	0.36	1.78	0.82	-	1.06	-	-	0.36	-	-	-	-	-
14	4.98	4.98	4.98	3.95	3.63	3.00	3.00	2.40	1.27	2.11	1.27	-	1.27	0.22	-	0.48	-	-	-	-	-	-	-	-
15	4.89	4.89	4.89	3.71	3.34	2.65	2.65	1.99	0.77	1.67	0.77	-	0.77	-	-	-	-	-	-	-	-	-	-	-
16	4.78	4.78	4.78	3.46	3.05	2.29	2.29	1.58	0.28	1.25	0.28	-	0.28	-	-	-	-	-	-	-	-	-	-	-
17	4.67	4.67	4.67	3.19	2.76	1.94	1.94	1.19	-	0.83	-	-	-	-	-	-	-	-	-	-	-	-	-	-
18	4.55	4.55	4.55	2.93	2.46	1.60	1.60	0.81	-	0.44	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	4.42	4.42	4.42	2.66	2.17	1.26	1.26	0.45	-	0.06	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20	4.30	4.30	4.30	2.40	1.88	0.95	0.95	0.11	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



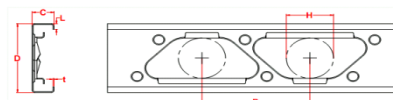
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 3" flange 43 mils - Section Identification: 600D300-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	7.87	7.87	7.87	7.53	7.42	7.19	7.19	6.97	6.53	6.86	6.53	5.87	6.53	6.09	5.22	6.20	5.65	4.58	5.87	5.22	3.94	5.55	4.79	3.31
9	7.81	7.81	7.81	7.37	7.23	6.95	6.95	6.67	6.10	6.53	6.10	5.27	6.10	5.55	4.46	5.68	5.00	3.67	5.27	4.46	2.89	4.86	3.93	2.12
10	7.73	7.73	7.73	7.20	7.02	6.66	6.66	6.31	5.62	6.14	5.62	4.62	5.62	4.95	3.64	5.12	4.29	2.69	4.62	3.64	1.76	4.12	3.00	0.86
11	7.64	7.64	7.64	6.99	6.77	6.34	6.34	5.92	5.10	5.71	5.10	3.91	5.10	4.30	2.77	4.50	3.53	1.67	3.91	2.77	0.61	3.33	2.04	
12	7.55	7.55	7.55	6.75	6.50	5.99	5.99	5.50	4.54	5.25	4.54	3.17	4.54	3.62	1.87	3.85	2.73	0.64	3.17	1.87	-	2.51	1.05	-
13	7.44	7.44	7.44	6.50	6.20	5.61	5.61	5.04	3.95	4.76	3.95	2.41	3.95	2.91	0.98	3.17	1.92	-	2.41	0.98	-	1.68	0.06	-
14	7.31	7.31	7.31	6.22	5.88	5.21	5.21	4.57	3.35	4.25	3.35	1.66	3.35	2.20	0.10	2.48	1.13	-	1.66	0.10	-	0.87	-	-
15	7.18	7.18	7.18	5.93	5.54	4.79	4.79	4.07	2.74	3.73	2.74	0.92	2.74	1.50	-	1.80	0.35	-	0.92	-	-	0.07	-	-
16	7.04	7.04	7.04	5.62	5.18	4.35	4.35	3.57	2.13	3.20	2.13	0.20	2.13	0.83	-	1.14	-	-	0.20	-	-	-	-	-
17	6.89	6.89	6.89	5.29	4.81	3.91	3.91	3.07	1.55	2.68	1.55	-	1.55	0.18	-	0.51	-	-	-	-	-	-	-	-
18	6.73	6.73	6.73	4.95	4.43	3.47	3.47	2.59	0.99	2.17	0.99	-	0.99	-	-	-	-	-	-	-	-	-	-	-
19	6.56	6.56	6.56	4.61	4.06	3.04	3.04	2.11	0.47	1.68	0.47	-	0.47	-	-	-	-	-	-	-	-	-	-	-
20	6.39	6.39	6.39	4.27	3.68	2.62	2.62	1.66	-	1.22	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



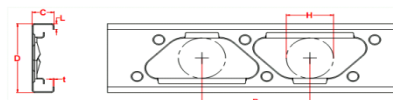
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 3" flange 54 mils - Section Identification: 600D300-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	13.10	13.10	13.10	12.77	12.66	12.45	12.45	12.23	11.80	12.13	11.80	11.16	11.80	11.38	10.52	11.49	10.95	9.89	11.16	10.52	9.26	10.85	10.10	8.65
9	12.94	12.94	12.94	12.52	12.38	12.11	12.11	11.82	11.28	11.69	11.28	10.46	11.28	10.73	9.67	10.87	10.20	8.88	10.46	9.67	8.11	10.06	9.14	7.35
10	12.75	12.75	12.75	12.23	12.05	11.70	11.70	11.36	10.68	11.19	10.68	9.69	10.68	10.02	8.72	10.18	9.37	7.78	9.69	8.72	6.85	9.20	8.09	5.95
11	12.53	12.53	12.53	11.88	11.67	11.25	11.25	10.84	10.03	10.63	10.03	8.85	10.03	9.23	7.70	9.43	8.46	6.60	8.85	7.70	5.53	8.27	6.97	4.49
12	12.28	12.28	12.28	11.50	11.25	10.75	10.75	10.26	9.31	10.02	9.31	7.94	9.31	8.39	6.64	8.62	7.50	5.39	7.94	6.64	4.18	7.28	5.80	3.02
13	12.00	12.00	12.00	11.08	10.78	10.20	10.20	9.64	8.54	9.36	8.54	7.00	8.54	7.50	5.54	7.76	6.50	4.16	7.00	5.54	2.85	6.26	4.62	1.60
14	11.69	11.69	11.69	10.62	10.28	9.61	9.61	8.97	7.75	8.66	7.75	6.04	7.75	6.59	4.46	6.88	5.50	2.97	6.04	4.46	1.57	5.23	3.46	0.24
15	11.36	11.36	11.36	10.13	9.74	9.00	9.00	8.28	6.94	7.93	6.94	5.09	6.94	5.69	3.40	5.99	4.51	1.83	5.09	3.40	0.36	4.23	2.34	-
16	11.01	11.01	11.01	9.62	9.18	8.36	8.36	7.57	6.13	7.20	6.13	4.17	6.13	4.79	2.40	5.12	3.56	0.77	4.17	2.40	-	3.26	1.30	-
17	10.64	10.64	10.64	9.08	8.60	7.70	7.70	6.87	5.33	6.47	5.33	3.29	5.33	3.94	1.46	4.28	2.66	-	3.29	1.46	-	2.35	0.33	-
18	10.26	10.26	10.26	8.52	8.01	7.05	7.05	6.17	4.57	5.75	4.57	2.46	4.57	3.13	0.60	3.48	1.82	-	2.46	0.60	-	1.50	-	-
19	9.86	9.86	9.86	7.96	7.43	6.42	6.42	5.50	3.84	5.06	3.84	1.70	3.84	2.38	-	2.73	1.05	-	1.70	-	-	0.74	-	-
20	9.45	9.45	9.45	7.41	6.84	5.79	5.79	4.85	3.16	4.40	3.16	1.00	3.16	1.69	-	2.04	0.35	-	1.00	-	-	0.04	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



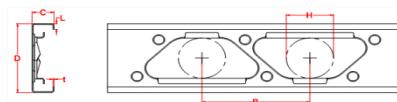
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

6" DeltaStud 3" flange 68 mils - Section Identification: 600D300-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	18.52	18.52	18.52	18.18	18.07	17.84	17.84	17.62	17.16	17.50	17.16	16.48	17.16	16.72	15.81	16.83	16.25	15.14	16.48	15.81	14.48	16.14	15.36	13.83
9	18.28	18.28	18.28	17.85	17.70	17.41	17.41	17.12	16.53	16.96	16.53	15.67	16.53	15.95	14.82	16.10	15.39	13.99	15.67	14.82	13.17	15.25	14.27	12.35
10	18.00	18.00	18.00	17.45	17.26	16.89	16.89	16.53	15.81	16.35	15.81	14.75	15.81	15.10	13.72	15.28	14.40	12.70	14.75	13.72	11.71	14.23	13.04	10.74
11	17.69	17.69	17.69	17.00	16.78	16.33	16.33	15.89	15.02	15.67	15.02	13.75	15.02	14.17	12.52	14.38	13.34	11.33	13.75	12.52	10.17	13.13	11.72	9.04
12	17.36	17.36	17.36	16.53	16.26	15.72	15.72	15.19	14.16	14.93	14.16	12.68	14.16	13.17	11.26	13.41	12.20	9.89	12.68	11.26	8.57	11.96	10.34	7.30
13	17.00	17.00	17.00	16.00	15.68	15.05	15.05	14.43	13.24	14.13	13.24	11.54	13.24	12.09	9.94	12.38	11.00	8.41	11.54	9.94	6.96	10.73	8.91	5.57
14	16.60	16.60	16.60	15.43	15.05	14.32	14.32	13.61	12.26	13.27	12.26	10.36	12.26	10.98	8.60	11.29	9.76	6.94	10.36	8.60	5.38	9.46	7.48	3.89
15	16.18	16.18	16.18	14.81	14.38	13.55	13.55	12.75	11.24	12.36	11.24	9.17	11.24	9.84	7.27	10.18	8.52	5.51	9.17	7.27	3.85	8.20	6.08	2.29
16	15.73	15.73	15.73	14.16	13.67	12.73	12.73	11.85	10.21	11.42	10.21	7.99	10.21	8.70	5.99	9.07	7.30	4.14	7.99	5.99	2.43	6.96	4.74	0.82
17	15.26	15.26	15.26	13.47	12.92	11.89	11.89	10.95	9.19	10.49	9.19	6.85	9.19	7.59	4.76	7.99	6.13	2.87	6.85	4.76	1.12	5.78	3.48	-
18	14.72	14.72	14.72	12.71	12.13	11.03	11.03	10.01	8.17	9.53	8.17	5.74	8.17	6.52	3.61	6.92	5.01	1.69	5.74	3.61	-	4.65	2.31	-
19	14.03	14.03	14.03	11.86	11.23	10.07	10.07	9.02	7.13	8.52	7.13	4.68	7.13	5.46	2.54	5.86	3.94	0.62	4.68	2.54	-	3.58	1.24	-
20	13.35	13.35	13.35	11.01	10.36	9.16	9.16	8.08	6.17	7.58	6.17	3.71	6.17	4.48	1.58	4.89	2.97	-	3.71	1.58	-	2.61	0.28	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



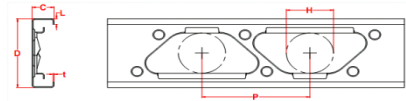
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 3" flange 33 mils - Section Identification: 800D300-33																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	5.63	5.63	5.63	5.38	5.29	5.13	5.13	4.96	4.63	4.88	4.63	4.13	4.63	4.30	3.64	4.38	3.97	3.15	4.13	3.64	2.67	3.89	3.32	2.19
9	5.61	5.61	5.61	5.29	5.18	4.97	4.97	4.76	4.34	4.66	4.34	3.72	4.34	3.93	3.10	4.03	3.51	2.49	3.72	3.10	1.88	3.41	2.69	1.28
10	5.58	5.58	5.58	5.19	5.06	4.79	4.79	4.54	4.02	4.41	4.02	3.26	4.02	3.51	2.50	3.64	3.00	1.76	3.26	2.50	1.03	2.88	2.01	0.30
11	5.55	5.55	5.55	5.07	4.91	4.60	4.60	4.29	3.67	4.13	3.67	2.75	3.67	3.05	1.86	3.21	2.45	0.98	2.75	1.86	0.12	2.30	1.27	-
12	5.51	5.51	5.51	4.94	4.75	4.38	4.38	4.01	3.28	3.83	3.28	2.21	3.28	2.56	1.17	2.74	1.86	0.16	2.21	1.17	-	1.69	0.49	-
13	5.47	5.47	5.47	4.80	4.58	4.14	4.14	3.71	2.87	3.49	2.87	1.64	2.87	2.04	0.45	2.25	1.24	-	1.64	0.45	-	1.04	-	-
14	5.42	5.42	5.42	4.64	4.38	3.88	3.88	3.38	2.42	3.14	2.42	1.04	2.42	1.50	-	1.73	0.60	-	1.04	-	-	0.38	-	-
15	5.37	5.37	5.37	4.47	4.18	3.60	3.60	3.05	1.97	2.77	1.97	0.44	1.97	0.94	-	1.20	-	-	0.44	-	-	-	-	-
16	5.31	5.31	5.31	4.28	3.96	3.32	3.32	2.69	1.51	2.39	1.51	-	1.51	0.38	-	0.66	-	-	-	-	-	-	-	-
17	5.24	5.24	5.24	4.09	3.72	3.01	3.01	2.33	1.04	2.00	1.04	-	1.04	-	-	0.12	-	-	-	-	-	-	-	-
18	5.17	5.17	5.17	3.89	3.48	2.71	2.71	1.97	0.58	1.61	0.58	-	0.58	-	-	-	-	-	-	-	-	-	-	-
19	5.09	5.09	5.09	3.67	3.23	2.39	2.39	1.60	0.12	1.22	0.12	-	0.12	-	-	-	-	-	-	-	-	-	-	-
20	5.01	5.01	5.01	3.46	2.98	2.08	2.08	1.23	-	0.83	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.

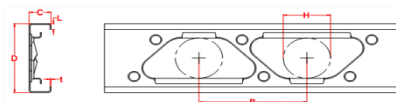


Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 3" flange 43 mils - Section Identification: 800D300-43																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	8.23	8.23	8.23	7.98	7.90	7.73	7.73	7.56	7.22	7.48	7.22	6.72	7.22	6.89	6.22	6.97	6.56	5.73	6.72	6.22	5.23	6.47	5.89	4.74
9	8.21	8.21	8.21	7.88	7.78	7.56	7.56	7.35	6.92	7.24	6.92	6.29	6.92	6.50	5.66	6.61	6.08	5.03	6.29	5.66	4.41	5.97	5.24	3.80
10	8.17	8.17	8.17	7.77	7.64	7.37	7.37	7.11	6.58	6.98	6.58	5.80	6.58	6.06	5.03	6.19	5.55	4.27	5.80	5.03	3.51	5.42	4.52	2.76
11	8.13	8.13	8.13	7.64	7.48	7.16	7.16	6.84	6.21	6.68	6.21	5.27	6.21	5.58	4.34	5.74	4.96	3.43	5.27	4.34	2.54	4.80	3.74	1.66
12	8.08	8.08	8.08	7.50	7.31	6.92	6.92	6.54	5.78	6.35	5.78	4.68	5.78	5.04	3.60	5.23	4.32	2.55	4.68	3.60	1.51	4.14	2.90	0.50
13	8.02	8.02	8.02	7.33	7.11	6.65	6.65	6.21	5.33	5.99	5.33	4.05	5.33	4.47	2.82	4.69	3.64	1.62	4.05	2.82	0.44	3.43	2.01	-
14	7.95	7.95	7.95	7.15	6.88	6.36	6.36	5.85	4.85	5.60	4.85	3.40	4.85	3.87	2.01	4.11	2.93	0.66	3.40	2.01	-	2.70	1.11	-
15	7.88	7.88	7.88	6.95	6.65	6.05	6.05	5.47	4.34	5.18	4.34	2.72	4.34	3.25	1.18	3.52	2.20	-	2.72	1.18	-	1.95	0.19	-
16	7.79	7.79	7.79	6.73	6.39	5.72	5.72	5.07	3.81	4.75	3.81	2.03	3.81	2.61	0.35	2.90	1.46	-	2.03	0.35	-	1.18	-	-
17	7.70	7.70	7.70	6.50	6.12	5.37	5.37	4.65	3.27	4.30	3.27	1.34	3.27	1.96	-	2.28	0.72	-	1.34	-	-	0.42	-	-
18	7.60	7.60	7.60	6.26	5.83	5.01	5.01	4.22	2.72	3.83	2.72	0.65	2.72	1.32	-	1.66	0.00	-	0.65	-	-	-	-	-
19	7.49	7.49	7.49	6.00	5.53	4.63	4.63	3.78	2.17	3.36	2.17	-	2.17	0.69	-	1.05	-	-	-	-	-	-	-	-
20	7.38	7.38	7.38	5.73	5.22	4.25	4.25	3.34	1.63	2.90	1.63	-	1.63	0.07	-	0.45	-	-	-	-	-	-	-	-

- Notes:**
- 1 - A detailed explanation of this table is given in the Design Criteria section.
 - 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
 - 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
 - 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
 - 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



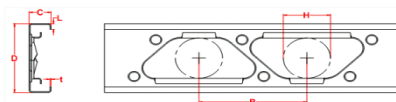
Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 3" flange 54 mils - Section Identification: 800D300-54																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	14.00	14.00	14.00	13.76	13.67	13.51	13.51	13.35	13.02	13.26	13.02	12.53	13.02	12.69	12.04	12.77	12.37	11.55	12.53	12.04	11.07	12.28	11.72	10.59
9	13.93	13.93	13.93	13.62	13.51	13.30	13.30	13.09	12.68	12.99	12.68	12.06	12.68	12.26	11.44	12.37	11.85	10.82	12.06	11.44	10.21	11.75	11.03	9.60
10	13.84	13.84	13.84	13.45	13.32	13.06	13.06	12.80	12.29	12.67	12.29	11.52	12.29	11.78	10.76	11.90	11.27	10.00	11.52	10.76	9.24	11.14	10.25	8.51
11	13.73	13.73	13.73	13.26	13.10	12.78	12.78	12.47	11.84	12.31	11.84	10.90	11.84	11.21	9.98	11.37	10.59	9.08	10.90	9.98	8.19	10.44	9.38	7.31
12	13.60	13.60	13.60	13.03	12.84	12.47	12.47	12.08	11.33	11.89	11.33	10.23	11.33	10.60	9.15	10.78	9.87	8.09	10.23	9.15	7.05	9.69	8.44	6.03
13	13.45	13.45	13.45	12.78	12.54	12.10	12.10	11.65	10.78	11.43	10.78	9.50	10.78	9.93	8.26	10.14	9.09	7.05	9.50	8.26	5.86	8.88	7.45	4.71
14	13.27	13.27	13.27	12.48	12.21	11.70	11.70	11.19	10.18	10.93	10.18	8.73	10.18	9.21	7.32	9.45	8.25	5.96	8.73	7.32	4.63	8.02	6.41	3.35
15	13.08	13.08	13.08	12.16	11.86	11.26	11.26	10.68	9.55	10.39	9.55	7.91	9.55	8.45	6.34	8.72	7.38	4.84	7.91	6.34	3.39	7.12	5.34	2.00
16	12.86	12.86	12.86	11.81	11.47	10.80	10.80	10.14	8.87	9.82	8.87	7.07	8.87	7.66	5.35	7.96	6.49	3.72	7.07	5.35	2.18	6.20	4.26	0.67
17	12.62	12.62	12.62	11.43	11.05	10.30	10.30	9.58	8.18	9.22	8.18	6.21	8.18	6.85	4.36	7.18	5.58	2.62	6.21	4.36	0.97	5.27	3.19	-
18	12.37	12.37	12.37	11.04	10.61	9.78	9.78	8.99	7.47	8.60	7.47	5.35	7.47	6.04	3.39	6.39	4.68	1.56	5.35	3.39	-	4.35	2.15	-
19	12.10	12.10	12.10	10.62	10.15	9.25	9.25	8.38	6.75	7.96	6.75	4.51	6.75	5.23	2.45	5.60	3.80	0.53	4.51	2.45	-	3.46	1.16	-
20	11.82	11.82	11.82	10.19	9.68	8.70	8.70	7.77	6.04	7.32	6.04	3.68	6.04	4.44	1.54	4.83	2.95	-	3.68	1.54	-	2.59	0.21	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
- 2 - Maximum unsupported length of stud about the weak axis is 48 inches.
* The use of one bridging row is at the discretion of the designer for an 8 ft stud height.
- 3 - Sheathing material of adequate strength and rigidity must be provided on both sides of the studs.
- 4 - To prevent failure of the sheathing-to-wall stud connection, the design engineer must provide for adequate connection. For additional design guidance the design engineer is referred to the AISI *Standard for Cold-Formed-Steel Framing: Wall Stud Design* (2007) and the companion commentary document.
- 5 - In addition to other conditions, the maximum single span height must be checked for the given deflection limit using the specified wind load as shown in the Wind Bearing Stud tables.



Maximum Factored Compressive Resistance, in kips per stud

DeltaStud Braced by Bridging Only (Unsheathed) ²

8" DeltaStud 3" flange 68 mils - Section Identification: 800D300-68																								
Stud Height (ft)	Factored Wind Loads (psf)																							
	0			10			20			30			40			50			60			70		
	Stud Spacing (inches)																							
	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24	12	16	24
8*	19.96	19.96	19.96	19.70	19.61	19.44	19.44	19.27	18.93	19.19	18.93	18.42	18.93	18.59	17.91	18.68	18.25	17.41	18.42	17.91	16.90	18.17	17.57	16.40
9	19.85	19.85	19.85	19.52	19.41	19.19	19.19	18.98	18.54	18.87	18.54	17.89	18.54	18.11	17.24	18.22	17.67	16.61	17.89	17.24	15.96	17.57	16.82	15.32
10	19.72	19.72	19.72	19.31	19.17	18.90	18.90	18.63	18.09	18.50	18.09	17.29	18.09	17.55	16.48	17.69	17.01	15.69	17.29	16.48	14.88	16.88	15.95	14.12
11	19.55	19.55	19.55	19.06	18.89	18.56	18.56	18.23	17.56	18.07	17.56	16.57	17.56	16.90	15.61	17.06	16.25	14.65	16.57	15.61	13.70	16.09	14.97	12.77
12	19.36	19.36	19.36	18.76	18.56	18.17	18.17	17.75	16.96	17.55	16.96	15.80	16.96	16.18	14.65	16.38	15.41	13.53	15.80	14.65	12.42	15.22	13.90	11.33
13	19.13	19.13	19.13	18.42	18.17	17.70	17.70	17.23	16.30	17.00	16.30	14.94	16.30	15.39	13.62	15.62	14.50	12.32	14.94	13.62	11.05	14.28	12.75	9.80
14	18.87	18.87	18.87	18.02	17.74	17.19	17.19	16.65	15.58	16.38	15.58	14.02	15.58	14.53	12.51	14.79	13.51	11.04	14.02	12.51	9.61	13.26	11.52	8.22
15	18.57	18.57	18.57	17.59	17.27	16.64	16.64	16.02	14.80	15.71	14.80	13.04	14.80	13.62	11.35	13.91	12.47	9.72	13.04	11.35	8.14	12.18	10.25	6.62
16	18.24	18.24	18.24	17.12	16.76	16.04	16.04	15.33	13.97	14.99	13.97	12.01	13.97	12.65	10.15	12.98	11.38	8.37	12.01	10.15	6.66	11.07	8.95	5.02
17	17.93	17.93	17.93	16.65	16.24	15.43	15.43	14.64	13.12	14.25	13.12	10.97	13.12	11.67	8.95	12.03	10.29	7.04	10.97	8.95	5.21	9.95	7.66	3.47
18	17.60	17.60	17.60	16.15	15.69	14.79	14.79	13.92	12.25	13.49	12.25	9.92	12.25	10.68	7.76	11.07	9.19	5.72	9.92	7.76	3.80	8.82	6.39	1.98
19	17.25	17.25	17.25	15.63	15.12	14.12	14.12	13.17	11.37	12.71	11.37	8.87	11.37	9.68	6.58	10.09	8.09	4.45	8.87	6.58	2.44	7.71	5.14	0.55
20	16.89	16.89	16.89	15.08	14.52	13.43	13.43	12.40	10.47	11.90	10.47	7.84	10.47	8.69	5.44	9.12	7.01	3.22	7.84	5.44	1.16	6.61	3.95	-

Notes:

- 1 - A detailed explanation of this table is given in the Design Criteria section.
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